HYDROGEOLOGIC ASSESSMENT REPORT (rev. 1)

PLANT YATES R6 CCR LANDFILL AND ASH MANAGEMENT AREA (AMA) COWETA COUNTY, GEORGIA

FOR



MAY 2021



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CERTIFICATION STATEMENT

This Hydrogeologic Assessment Report, Georgia Power Company - Plant Yates R6-AMA has been prepared to comply with the Georgia Environmental Protection Division Rules for Solid Waste Management 391-3-4.10 Coal Combustion Residuals, revised June 18, 2018; which are also in compliance with United States Environmental Protection Agency (USEPA) coal combustion residual (CCR) rule 40 Code of Federal Regulations [CFR] 257 Subpart D, published in 80 FR 21302-21501, April 17, 2015, by a licensed professional geologist with:

ATLANTIC COAST CONSIDER

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1.0 INTRODUCTION

In support of Georgia Department of Natural Resources Environmental Protection Division (GA EPD) Regulation 391-3-4-.10 (6) "Groundwater Monitoring and Corrective Action" as referenced in the Code of Federal Regulations (CFR) 40 CFR 257.90 – 40 CFR 257.98, Atlantic Coast Consulting, Inc (ACC) has prepared for Georgia Power Company (Georgia Power) this Hydrogeologic Assessment Report, Georgia Power Company Plant Yates R6 - AMA. Georgia Power is proposing to close several CCR surface impoundments and landfills with "closure by removal" and "closure in-place". This report is a comprehensive summary of the hydrogeologic characteristics for Plant Yates and focuses on multi-units; R6 CCR Landfill (R6) and the Ash Management Area (AMA).

This report was originally submitted as part of the November 2018 permit application package. The document has been expanded to include additional data (*requested by GA EPD*) supporting the permit application package and is being resubmitted in its entirety. The additional data includes a mapping and lineament study report, hydraulic conductivity testing data, geochemical characterization data, additional potentiometric data, and groundwater modeling.

1.1 Site Location & Topographic Setting

Plant Yates is located at 708 Dyer Road, on the east bank of the Chattahoochee River in Coweta County, Georgia near the Coweta and Carroll County line, approximately eight miles northwest of the city of Newnan and 13 miles southeast of the city of Carrollton. Plant Yates occupies approximately 2,400 acres. Figure 1, Site Location Map depicts the site location relative to the surrounding area. Plant Yates, once a coal fired, power generation facility, was converted to was converted to natural-gas-combustion turbines. Plant Yates is located within the northeast quarter of the Whitesburg, GA United States Geological Survey (USGS) 7.5-minute topographic quadrangle.

1.2 Coal Combustion Residuals

Plant Yates has multiple Coal Combustion Residual (CCR) units which will be permitted separately according to Chapter 391-3-4-.10, Coal Combustion Residuals of the Georgia Rules for Solid Waste Management (State CCR Rule). The CCR units at Plant Yates consist of the Gypsum Stack, R6/AP-C CCR Landfill, AP-1, Ash Pond 2 (AP-2) and the AMA.

1.3 R6 CCR Landfill

The Plant Yates R6, Solid Waste Handling Permit 038-011D(LI) was issued by GA EPD on May 17, 1985. R6 is an Inactive CCR landfill as defined in the Georgia Rules for Solid Waste Management, 391-3-4-.10(2)(a)3. in that it no longer received CCR on or after October 19, 2015. R6 will be closed in place in accordance with the solid waste permit and the CCR Rules. Figure 2, R6-AMA Monitoring Well Network illustrates the monitoring well network and Table 1, R6-AMA Well Construction Detail Summary provides the monitoring well construction details. Boring and well construction logs for the monitoring wells are provided in Appendix A, Boring and Well Construction Diagrams.

1.4 AMA

The AMA is located within the former footprints of AP-3 and AP-B'. This multi-pond area is currently being consolidated and closed in place and contains CCR from AP-1 and AP-2 (permitted separately), and AP-A and AP-B (included as part of the AMA permit application) which have been or are being closed by removal. AP-A is an inactive 8.9-acre CCR surface impoundment

that completed closure by removal in June 2017. AP-B is an inactive 6.3-acre CCR surface impoundment currently undergoing closure by removal. AP-B' is a 29.8-acre CCR surface impoundment that is being closed-in-place. AP-3 is a 55-acre surface impoundment that is being closed-in-place. Figure 2 illustrates the monitoring well network and Table 1 provides the monitoring well construction details. Boring and well construction logs for the monitoring wells are provided in Appendix A.

2.0 SURFACE AND SUBSURFACE INVESTIGATIONS

A geologic investigation was conducted by Golder Associates (Golder) in 2015 to evaluate the subsurface conditions at Plant Yates. Golder relied on published literature, topographic maps and local geologic experts in addition to on-site investigations. The field investigation included strike and dip measurements (orientation) of foliations, lineaments and fractures of outcropping bedrock across the site and remote sensing data. Rock types were identified from previous bedrock core drilling logs and rock/soil weathering profiles from the parent bedrock. Golder's report is provided as Appendix B, Geologic Mapping and Lineament Analysis Report.

2.1 Regional Geology

The Piedmont/Blue Ridge geologic province contains some of the oldest rocks in the Southeastern United States. Since their origin, approximately 276 million to 1,100 million years ago (Ma), these late Precambrian (Neoproterozoic) to late Paleozoic (Permian) rocks have undergone repeated cycles of igneous intrusions and extrusions, metamorphism, folding, faulting, shearing, and silicification. The latest regional metamorphism and associated deformation has been attributed to the collision of the North America plate with the Eurasian plate approximately 200 Ma to 230 Ma. More recent deformation and emplacement of mafic dikes is associated with the rifting of the North American craton during the Mesozoic and Cenozoic Eras.

The metamorphic and igneous rocks that underlie the area have been subjected to physical and chemical weathering which has created a landscape dissected by creeks and streams forming a dendritic drainage pattern. These rocks are deeply weathered due to the humid climate and bedrock is typically overlain by a variably thick blanket of residual soils and saprolite. The overall depth of weathering in the Piedmont/Blue Ridge is generally about 20 to 60 feet; however, the depth of weathering along discontinuities and/or very feldspathic rock units may extend to depths greater than 100 feet. Because of such variations in rock types and structure, the depth of weathering can vary significantly over short horizontal distances. Locally, significant accumulations of alluvial soils are also present in the area. Sand and gravel alluvial terraces related to changing stream base grade elevations occur above residual soils in areas near larger rivers (i.e. Chattahoochee River).

2.2 Site Geology

The site geology is typical of Piedmont weathering of igneous and metamorphic bedrock. In addition to the physical and chemical weathering soil profile that includes residuum and saprolitic soils; alluvial deposits from the Chattahoochee River were noted in the soils and sediments on the west side of the site.

2.1.1 Alluvial Deposits

Alluvial soils primarily consisted of courser material that may be related to a former Chattahoochee River terrace, consisting of silty-sand, clayey silt, and silty clay with well-rounded gravel and cobbles. The terrace was observed primarily within point bars of the Chattahoochee River with up to 20 feet of relief. Overbank, floodplain-type deposits are anticipated to underlie the Plant, as well as former river channels related to previous migration of the Chattahoochee River.

2.1.2 Residuum and Saprolite

Residual soils were also observed at ground surface. These soils were derived from physical and chemical weathering of the underlying bedrock, and consist primarily of silty sand, sandy silt, and

silty clay, with local gravel-sized angular fragments of weathered bedrock. A variably-thick blanket of residual soils is anticipated to overlie bedrock across most of the site.

Based on the detailed geologic mapping, rock types present at the site includes granitic and migmatitic gneiss, biotite gneiss and amphibolite, all of which have highly variable mineralogy, texture and chemistry, and a muscovite schist that serves as a structural marker horizon. Residual soils developed from weathering of these rock types may have variable geochemical characteristics. A thin layer of soil from approximately one feet to ten feet thick overlies a thick layer of saprolite. The saprolite, which extends to typical depths of 20-40 feet below ground surface, was formed from the physical and chemical weathering of the underlying metamorphic rocks. The saprolite generally maintains the texture of the parent bedrock but is friable with unconsolidated hydraulic permeability that is typically greater than the overlying residuum.

2.1.3 Bedrock

Plant Yates lies within the Dadeville Complex of the Inner Piedmont of western Georgia, immediately southeast of the regional zone of deformation referred to as the Brevard Zone. Dadeville Complex rock units are interlayered gneiss and schists as shown in Figure 3, Plant Yates Geologic Map. At ground surface the units have weathered in place to form a layer of saprolite. A regional thrust fault, the Katy Creek Fault, forms boundary between the Brevard Zone, and the Dadeville Complex. The Dadeville Complex is considered to represent an Ordovician-age Island Arc. The rocks in the area have been subjected to several episodes of metamorphism and intrusion by igneous bodies. Extensive jointing occurs in the area. Surface expressions of the joints are observed on topographic maps and aerial photos of the Plant Yates area.

Rocks within the Dadeville complex are generally more mafic near the Brevard Zone, becoming more felsic in areas farther southeast of the Brevard Zone. The mafic portion of the Dadeville Complex observed northwest of Plant Yates primarily consists of biotite gneiss and layers of amphibolite/hornblende gneiss. The continuous and discontinuous amphibolite layers and lenses weather more deeply but less uniformly than the surrounding biotite gneiss in this area. Units underlying that area around Plant Yates and further southeast are more felsic, being comprised of a mixture of granitic intrusives, migamitic and biotite gneiss, and alumnosilicate schists. Mappable lithologic units across the site described in Section 3.3 of Appendix B, include: massive granite (OZgr), migmatitic gneiss (OZmgn), feldsphatic gneiss (OZggn), porphyroblastic schist (OZgss), biotite gneiss (OZog), and amphibolite (OZa). A geologic map prepared by Golder Associates as part of the Geologic Mapping And Lineament Analysis in October 2017 (Golder 2017) is provided on Figure 3.

Geologic cross sections for Plant Yates R6-AMA was prepared along an alignment presented on Figure 4, Cross Section Location Map. Cross section A-A' (Figure 5) is oriented northwest to southeast, B-B' (Figure 6) approximately southwest to northeast, and C-C' (Figure 7) approximately southwest to northeast.

2.2 Fractures and Structure

Based on detailed geologic mapping conducted by Golder, folding and faulting have been observed at and near the site (Appendix B). A regional fault that occurs northwest of the site is a thrust fault that is part of the Brevard Fault Zone. There has been no active displacement of the rocks during the Holocene Epoch. The following description is summarized from the Golder's report.

Migmatitic gneiss occurs both north and south of the Chattahoochee River and this unit is

truncated northwest and southwest of the site by an unnamed, strike-slip fault, referred to as the Katy Creek Fault. The Katy Creek Fault occurs subparallel to the Chattahoochee River near the site and turns southward in the southwest corner of the site. There is likely to be a rheologic contrast between the migmatitic gneiss and the mass of granite, with the granite being much harder than the surrounding gneiss. This hard mass likely forced the fault to deflect around the mass in this area. Rocks that were being transported in the southeastern block of the fault appear to have been "piled" up, creating an upright, slightly overturned, west verging antiformal structure and is presented on the Geologic Map (Figure 3).

The core of the antiformal structure is comprised of feldspathic gneiss, which weathers more deeply than other lithologic units observed on site. Rimming this unit is the porphyroblastic schist, which serves as a distinctive marker horizon for delineating structure on site. Lithologies related to the OZa Amphibolite unit which is observed in the mafic portion of the Dadeville Complex are also present adjacent to the fault on the southeastern block.

Foliation measured during geologic mapping near the site reflects the dominantly north-south trending structures in this area. Mapping further northwest of the fault reflects the more regional orientation of foliation characteristic of the Brevard Zone, which is generally oriented about N40-45E (Golder 2017).

Because the evaluation of joints is visual and judgmental, an effort is made for consistency in describing the relative frequency of occurrence using the following designations: Abundant (A); Common (C); and Scarce (S). These designations are relative to one another but are used consistently in descriptions made throughout the study area. An effort is made to record all of the different joint sets and, if an exposure is large, several same (or similar) joints may be recorded at the same Map Station. This deliberate method of visual evaluation in the field is more scientifically relevant and efficient than saturation-measurement of joints.

Four major joint sets were recorded during the detailed geologic mapping. The four major joint sets are (quadrant and azimuth, right hand rule) (Golder 2017):

- J1: N48E 61NW (228/61) regional strike joint
- J2: N50E 72SE (050/72) regional strike joint
- J3: N44W 72SW (136/72) regional dip joint
- J4: N3W 87SW (177/87) local strike joint

Joint sets 1 and 2 are oriented subparallel to regional foliation, the dip of which varies in response to the dip of foliation. These joints are referred to above as regional strike joints. Joint set 3 is oriented perpendicular to regional foliation and is referred to as a regional dip joint. Joint 4 appears to have formed locally in response to the north-south trending structures mapped on the site and southwest of the site. This joint set is referred to as the local strike joint

Two major structural/stratigraphic packages occur within the Piedmont/Blue Ridge around the site. These packages are separated by The Katy Creek Fault, a regional fault that occurs northwest of the Plant property and traverses northeast-southwest through this area. This Fault is a thrust fault, dipping at a lower angle than the strike-slip faults present in the region. This thrust fault developed at a relatively shallower depth in the crust than the strike-slip faults, resulting in less-pronounced ductile structural fabrics. Uplift associated with building of the Appalachian Mountains and subsequent erosion has allowed modern exposure of these structural features (Golder 2017).

The Katy Creek Fault forms boundary between a regional zone of deformation, referred to as the

Brevard Zone, and the Dadeville Complex. Lithologic contacts and major structural features in the Brevard Zone generally trend northeast-southwest. In addition to strike-slip and thrust faults, structural features within this shear zone consist of northwest-verging, doubly-plunging, overturned folds that have been overprinted by a shear-induced foliation (Golder 2017).

The Brevard Zone consists, but is not limited to fine-grained and porphyroblastic, phyllonitic, biotite and button schists, and schists locally interlayered with amphibolite/hornblende gneiss, and ultamafic bodies. Other igneous and metamorphic bedrock types include; mylonites, ultramylonites, and flinty crush rock; metagraywacke and feldspathic quartzite; and granitic gneiss. Rocks within these various lithologic units have been intensely deformed, sheared, chemically altered, silicified and are generally repeated because of movement along faults both within and outside of the Brevard Zone (Golder 2017).

The Dadeville Complex occurs southeast of the Brevard Zone and is considered to represent an Ordovician-age Island Arc. Rocks within this complex are generally more mafic near the Brevard Zone, becoming more felsic in areas further southeast of the Brevard. The mafic portion of the Dadeville Complex primarily consists of biotite gneiss and thick, mappable layers of amphibolite/hornblende gneiss. The biotite gneiss is also interlayered with thin, discontinuous layers, lenses and pods of amphibolite. The continuous and discontinuous amphibolite layers and lenses weather more deeply but less uniformly than the surrounding biotite gneiss in this area. These mafic units are observed northwest of Plant Yates. Units underlying that area around Plant Yates and further southeast are more felsic, being comprised of a mixture of granitic intrusives, migamitic and biotite gneiss, and alumnosilicate schists.

Typically, up to four different joint sets formed in this area due to tectonic stresses imposed upon the bedrock. Dip joints form parallel to the regional dip direction of foliation/compositional layering and are typically perpendicular to fold limbs, representing extension perpendicular to the maximum principal stress direction or direction of compression. These joints are commonly near vertical. Strike joints develop parallel to the strike of foliation/compositional layering and fold limbs, typically forming from tension during relaxation of the maximum principal stress. The dip direction and angle of these joints is orthogonal to the dip direction and angle of compositional layering. Oblique joints develop diagonal (\pm 30°) to the principal stress direction and represent conjugate sets formed from shear along the intermediate principal stress. (Golder 2017).

3.0 SITE HYDROGEOLOGIC CONDITIONS

The composition of the unsaturated zone varies across the area and includes soils and saprolite. Sieve analyses show an upper zone in most of the area to be clays, silty sands, and clayey sands. Clays may act as a confining or semi-confining material in soils, but at this location, they are interbedded with other materials or absent, and they are not considered a true confining unit. Atterberg limit tests, visual inspection, and grain analyses, indicate the presence of significant fines. The soil/rock interface has been identified as the shallowest water-bearing unit in the area. Groundwater monitoring wells have been installed in shallow bedrock at depths that will allow for consistent sample collection in the event of future drought conditions. Grain size analyses of the soils and saprolite indicate that this unit is predominantly fine to medium silty sand.

At Plant Yates, shallow groundwater is typically encountered near the saprolite/weathered rock interface. Rock becomes increasingly more competent with depth and movement of groundwater occurs only in fractures (i.e. secondary porosity). Recharge to the water-bearing zones in fractured bedrock takes place by seepage through the overlying mantle of residuum/saprolite, or by direct entrance through openings in outcrops. The ponds were established along a topographically low area formed by an unnamed tributary. Monitored aquifer zones in the subsurface at Plant Yates includes the shallow unconsolidated residuum/saprolite and the upper bedrock. Typically, in the Piedmont, fracture frequency and permeability in the igneous and metamorphic bedrock decrease with depth.

3.1 R6-AMA Site Hydrogeology

Groundwater is encountered across R6-AMA in five hydrogeologic units. The uppermost hydrogeologic layer is the residuum/saprolite layer. The residuum layer is located from the ground surface and characteristically has more fines and typically is classified as a sandy silt or clayey silt. The saprolite layer has the texture of the underlying bedrock but is friable and has less fines. Typically, the saprolite layer has nearly an order of magnitude greater hydraulic conductivity compared to the overlying residuum.

The transition zone, located below the saprolite, is hydraulically connected to the residuum/saprolite and has the highest frequency of weathered fractures in the bedrock. The bedrock varies across the site from metamorphosed granitic gneiss to amphibolites to schist. Three zones or layers of the competent bedrock (upper, middle and lower) are monitored zones. Typically, in the Piedmont, the transition zone and upper bedrock have the greatest hydraulic conductivity of the five monitoring zones. The middle and lower bedrock generally have less fractures and thus lower hydraulic conductivity.

Groundwater flow across the site is generally from the southeast to the northwest. Groundwater flow across the R6-AMA area is from three directions; south to north, southeast to northwest and from northeast to southwest as illustrated in the September 2020 potentiometric contour map illustrated on Figure 8. Groundwater elevations for the monitoring wells and piezometers measured during the September 2020 sampling event are presented on Table 1. The monitoring wells are either screened in the residuum/saprolite or the deeper igneous and metamorphic bedrock. Upgradient monitoring well(s) for Plant Yates include:

YGWA-1I	•	YGWA-5I	•	YGWA-21I
YGWA-1D	•	YGWA-5D	•	YGWA-30I
 GWA-2 	•	YGWA-14S	•	YGWA-39
YGWA-2I	•	YGWA-17S	•	YGWA-40

•	YGWA-3I	•	YGWA-18S	•	YGWA-47
•	YGWA-3D	•	YGWA-18I		
•	YGWA-4I	•	YGWA-20S		

Downgradient monitoring wells for the R6-AMA include:

•	YGWC-23S	•	YGWC-38	•	YGWC-43
•	YGWC-24SA	•	YGWC-41	•	YGWC-49
•	YGWC-36A	•	YGWC-42	•	YGWC-50
					(Future)

Hydraulic conductivity (K) is defined as the rate at which water can move through a permeable medium. In situ rising head and falling slug tests were performed at multiple locations on the site. The range in k values at these locations was small, indicating a fairly uniform hydrogeologic layers across the saprolite and weathered rock horizon (typically range from 10⁻³ cm/sec to 10⁻⁴ cm/sec). Table 2A, Horizontal Hydraulic Conductivity Data Summary, presents a summary of the K values calculated from variable head testing from 2014 and 2017 from wells in CCR unit AP-1. The values from the field tests are consistent with other values measured at Plant Yates. Vertical K values for locations throughout Plant Yates were determined by laboratory testing of undisturbed overburden samples (Shelby Tubes) collected at multiple Plant Yates locations (Cardno ATC, 2015) and are summarized in Table 2B, Sitewide Vertical Hydraulic Conductivity Data Summary. Test data and graphs are presented in Appendix C, Hydraulic Conductivity Testing and Results. The average depth of the water table at the R6-AMA during the September 2020 sampling event varies with topography, ranging from approximately 5 feet below ground surface (bgs) to 28 feet bgs. The water table occurs in the saprolite ranging from ten feet above the top of competent bedrock to within the upper bedrock

3.2 Potentiometric Data

Groundwater monitoring data have been collected since 2016 and multiple potentiometric maps of the uppermost aquifer are presented in Appendix D, 2016 - 2020 Historical Potentiometric Maps and Water Level Data. Groundwater monitoring wells and non-network wells were resurveyed in June 2020. Data depicted on the historical potentiometric surface maps reference slightly different top of casing elevations than those referenced in the remainder of the report. Groundwater flow across the AMA -R6 is generally from the east northeast to the west southwest. The horizontal gradients were consistent until CCR excavations in the AMA began in 2017.

The horizontal hydraulic gradient (i) across the R6-AMA was measured during the September 2020 groundwater monitoring event from YGWA-40 to YGWC-2R and YGWC-49 to PZ-24I resulting in an estimated average horizontal gradient of 0.022 ft/ft (Figure 8).

Average groundwater flow velocity in the R6-AMA area is based on K, lateral gradient (i) and effective porosity (P_e). The average K for the site is 505 feet/year. The effective porosity (n_e) was estimated at 0.20. The average groundwater velocity is calculated as:

 $V_{gw} = (K)(i)/n_e = ((505 \text{ ft/year}) (0.022 \text{ ft/ft})/0.20) = 56 \text{ feet/year}.$

3.3 Groundwater Modeling

A numerical groundwater flow model (Groundwater Vistas®) for the AMA-R6 area, was prepared to achieve the following objectives;

• Improve the conceptualization of the hydro-stratigraphy, hydrogeology, and groundwater/surface water dynamics at the site,

- Construct a numerical groundwater flow model that accurately depicts pre-closure groundwater flow conditions, and
- Use the numerical groundwater flow model as a tool to predict groundwater flow conditions from the implementation of closure activities.

The numerical groundwater model was set up using six layers to depict the five subsurface hydrogeologic units underlying the ash layer. The model layers are:

- Layer 1 surficial ash layer
- Layer 2 unconsolidated residuum/saprolite
- Layer 3 transition zone, highly weathered upper bedrock
- Layer 4 moderately weathered, highly fractured competent bedrock
- Layer 5 less weathered, less fractured competent bedrock
- Layer 6 lower bedrock with decreased fracture frequency

The model was calibrated using an industry standard software, Parameter Estimation (PEST®), which adjusts various parameters until the groundwater elevations are nearly matched. Groundwater elevation measurements collected from 2014 through 2016 and prior to closure activities were used in the model. A comprehensive groundwater modeling report prepared by TRC is provided in Appendix E, Numerical Modeling Report.

3.4 Conceptual Site Model

The hydrogeologic Conceptual Site Model (CSM) for Plant Yates and R6-AMA consists of geologic and hydrologic elements.

Geologic model elements include:

- Plant Yates is located in the Ordovician-age Dadeville Complex of the Inner Piedmont
 Physiographic Province of western Georgia. The rocks have been subject to several
 episodes for metamorphism and intrusion by igneous bodies (granitic units). Rock types
 present at AP-1 include sillimanite-staurolite-garnet schist, granitic gneiss, and orange
 gneiss.
- Rocks have been weathered in place to form soil (silty sand residuum), weathered bedrock (saprolite) and partially weathered bedrock. These materials constitute the unconsolidated uppermost aquifer at the site.
- The facility lies to the southeast of the Yates fault, a regional thrust fault that is the boundary between the Dadeville Complex and the Brevard Zone.
- Folding and faulting of rock units has been observed at the site. Joints sets are observable both parallel and perpendicular to foliation.

Hydrologic elements include:

- The facility lies within the Middle Chattahoochee River basin of the Piedmont Physiographic Province, which characteristically has moderate rolling hills that are steeply cut with surface water drainages.
- The depth to groundwater is variable depending on the topography; the depth to groundwater is deeper at topographic highs and shallower at topographic lows.
- Groundwater flow in the upper aquifer is under unconfined conditions and the water table is typically noted in the saprolite near the bedrock interface. Deeper groundwater flow is within the fractured bedrock and along discontinuities.

- Groundwater flow direction in the upper aquifer is controlled by topography and by drainage features such as creeks, man-made ash impoundments, and the Chattahoochee River. The general site-wide groundwater flow direction is from the eastnortheast to west-southwest. Groundwater flow at R6-AMA is from three directions; south to north, southeast to northwest and from northeast to southwest.
- Surface water bodies, surface water drainage, and flow across the facility are noted on Figure 2. Surface water flows onto the property in the northeast corner and is routed around the AMA and north and east of R6 CCR Landfill emptying into impoundment AP-2.
- Surface water discharge to the Chattahoochee River is regulated by a Georgia NPDES permit No. GA0001473.

The hydrogeologic framework consists of a residuum overlying fractured bedrock, separated by a relatively-thin soil zone (approximately 10 feet thick) underlain by an approximately 20-40 feet thick saprolite-zone that includes partially-weathered rock and then deeper to a highly-fractured upper bedrock. The thickness of saprolite is variable depending on the lithologic variations across the site. Groundwater level and geochemical data from the site supports topography-controlled groundwater flow with a downward hydraulic gradient from the residuum material and upward hydraulic gradient near discharge areas. Based on extensive data collection from the site, the residuum, saprolite and the upper bedrock together constitutes the uppermost aquifer. Therefore, the monitoring well network is designed to capture groundwater flow from wells screened in the upper bedrock. This well network serves as the detection monitoring network for the CCR monitoring program.

4.0 GROUNDWATER MONITORING NETWORK

An adjacent drainage channel separates R6 and the AMA. Both the CCR units share similar lithologies and hydrogeologic framework, and the groundwater flow is predominantly towards the drainage channel, following the topography, and is directed to the downgradient AP-2. For these reasons, groundwater flow from both the CCR units are expected to mix along preferential flow paths in the regolith-bedrock aquifer system. Therefore, a multi-unit groundwater monitoring network has been designed and installed to monitor groundwater flow from the R6 and the AMA. This multi-unit groundwater monitoring network consists of 19 upgradient background monitoring wells located throughout Plant Yates, 9 existing downgradient groundwater monitoring wells, and 1 proposed groundwater monitoring well located around the intersection of the perimeter of R6, and the adjacent AMA. Figure 2 illustrates the multi-unit monitoring well network and Table 1 provides the monitoring well construction details.

The monitoring system is designed to monitor groundwater at AMA-R6 within the uppermost aquifer. Wells designated with "GWA" are considered upgradient and with a designation of "GWC" are considered downgradient. Wells were located to serve as upgradient and downgradient monitoring points based on groundwater flow direction (Table 1). In accordance with §257.91, a groundwater monitoring system was installed that provides the following; (1) consists of a sufficient number of wells, (2) installed at appropriate locations and depths to yield groundwater samples from the uppermost aquifer, and (3) meets the performance standards of §257.91(a). In summary, groundwater monitoring activities included the installation of the following:

- Upgradient groundwater monitoring network wells throughout Plant Yates (YGWA-1I, YGWA-1D, GWA-2, YGWA-2I, YGWA-3I, YGWA-3D, YGWA-4I, YGWA-5I, YGWA-5D, YGWA-14S, YGWA-17S, YGWA-18S, YGWA-18I, YGWA-20S, YGWA-21I, YGWA-30I, YGWA-39, YGWA-40, and YGWA-47)
- Ten (10) downgradient groundwater monitoring network wells; (YGWC-23S, YGWC-24SA, YGWC-33S, YGWC-36A, YGWC-38, YGWC-41, YGWC-42, YGWC-43, YGWC-49 and proposed well YGWC-50)

Well YGWC-24SA was installed to replace well YGWC-24S in June 2020 to accommodate planned construction work along Dyer Road.. Closure by removal activities in the R6 ditch necessitated abandonment of YGWC-36 on July 20, 2020 and replacement well YGWC-36A was installed on September 22, 2020. Monitoring well locations are provided on Figure 2.

The monitoring wells are screened in shallow bedrock. The historical water levels for the R6-AMA are tabulated and are found in Appendix D.

4.1 Groundwater Monitoring and Constituents of Concern

In accordance with 40 CFR 257.91, a groundwater monitoring system was installed at the site, based on the characterization of site-specific hydrogeologic conditions. The well network was certified by a P.E. for the AMA (for AP-A, AP-B, AP-B' and AP-3) on October 17, 2017 (amended to include AP-A on April 17, 2019); the certification is maintained in the Operating Record. The certified compliance monitoring well network for the original group of AMA CCR units (AP-A, AP-B, AP-B' and AP-3) consisted of a total of 13 monitoring wells: eight (8) upgradient wells and five (5) downgradient wells (YGWA-4I, YGWA-5I, YGWA-5D, YGWA-17S, YGWA-18S, YGWA-18I, YGWA-20S, YGWA-21I, YGWC-23S, YGWC-24S, YGWC-33S, YGWC-36, and YGWC-49).

The original monitoring well network for R6 included a total of two upgradient wells (YGWA-39 and YGWA-40) and four downgradient wells (YGWC-38, YGWC-41, YGWC-42 and YGWC-43). The well network has been updated in include all 19 upgradient background monitoring wells

installed throughout Plant Yates. Downgradient monitoring wells YGWC-24S and YGWC-36 have been replaced by YGWC-24SA and YGWC-36A, respectively. One location, YGWC-33S has been abandoned. The combined monitoring well networks for AMA and R6 provides for 19 upgradient monitoring wells and 8 downgradient monitoring wells. GPC is also proposing to install one additional downgradient monitoring well YGWC-50, to be located between monitoring wells YGWC-36 and YGWC-43. Additionally, there are 5 downgradient characterization wells (YAMW-1 through YAMW-5) and 10 piezometers used for water level measurements, vertical/horizontals delineation, or non-routine sample collection. Locations are shown on Figure 2; well construction and water level elevation details for monitoring network wells are listed in Table 1.

Statistical analysis of the June and October 2018 analytical data identified a statistically significant level (SSL) for beryllium in one well, YGWC-33S. In accordance with 40 CFR 257.95(g), a notification identifying the SSL was prepared and placed in the Operating Record on November 14, 2018. Pursuant to 40 CFR 257.96, an assessment of corrective measures (ACM) was initiated on January 13, 2019.

To assess the extent of groundwater protection standard (GWPS) exceedances identified in 2018, two additional groundwater monitoring wells, PZ-35 and YAMW-1, were sampled in 2018 to provide additional data for characterizing groundwater quality downgradient of this well.

The facility has completed the actions included in 40 CFR 257.95(g)(1)(i – iv), including the installation of horizontal and vertical extent assessment wells (PZ-35 and YAMW-1). These well locations are downgradient from former well YGWC-33S between YGWC-36A and YGWC-24SA. The location of YGWC-33S was narrowly constrained to an area between R6 and the former footprint of AP-B'. Therefore, delineation wells YAMW-1 and PZ-35 provide appropriate representations with the spatial and vertical extent of beryllium in groundwater near well YGWC-33S.

R6 ceased accepting CCR prior to October 19, 2015 and is therefore not subject to Federal monitoring requirements. A CCR Unit Solid Waste Handling Permit application for R6 was submitted to GA EPD in November 2018 and is currently under review. In accordance with § 257.94(b), a detection monitoring program for R6 was implemented by collecting eight (8) background samples during 2016 – 2018. Groundwater monitoring has been initiated in order to meet GA EPD requirements. The initial detection monitoring data for R6 was collected in March 2019. Assessment monitoring was initiated at the R6 monitoring wells in September 2019. Statistical comparisons of AIV parameters to relevant GWPS indicated SSLs for beryllium at YGWC-38 and selenium at YGWC-38 and YGWC-41. An exceedance for selenium was also identified in delineation well PZ-37. R6 was added to the ACM on January 30, 2020.

The SSL of selenium in well YGWC-41 is spatially delineated to below GWPS at wells YAMW-2 and YAMW-3, and vertically at YAMW-4. The SSL of beryllium and selenium in well YGWC-38 is vertically delineated to below GWPS at YAMW-5, and spatially delineated by downgradient well YGWC-36A. PZ-37 is in a groundwater flow path that leads into the waste boundary. Delineation of selenium at that location is maintained by compliance wells YGWC-23S and YGWC-36A.

4.2 Groundwater Monitoring Status

AMA-R6 is monitored as one multi-unit groundwater monitoring network. In accordance with State CCR rule 391-3-4.10(6), a multi-unit groundwater monitoring network has been designed and installed to monitor groundwater at R6 and the AMA. The combined network reflects a well-defined upgradient and downgradient locations to the combined CCR unit and captures the topographic variations and hydrogeologic units, providing a suitable network for compliance monitoring at the site. The monitoring status of the combined AMA-R6 CCR unit will follow the

GEORGIA POWER PLANT YATES
R6-AMA HYDROGEOLOGICAL ASSESSMENT REPORT (rev. 1)

schedule established for the AMA unit (i.e., relevant SSLs identified for R6 will be addressed in conjunction with AMA exceedances).

5.0 REFERENCES

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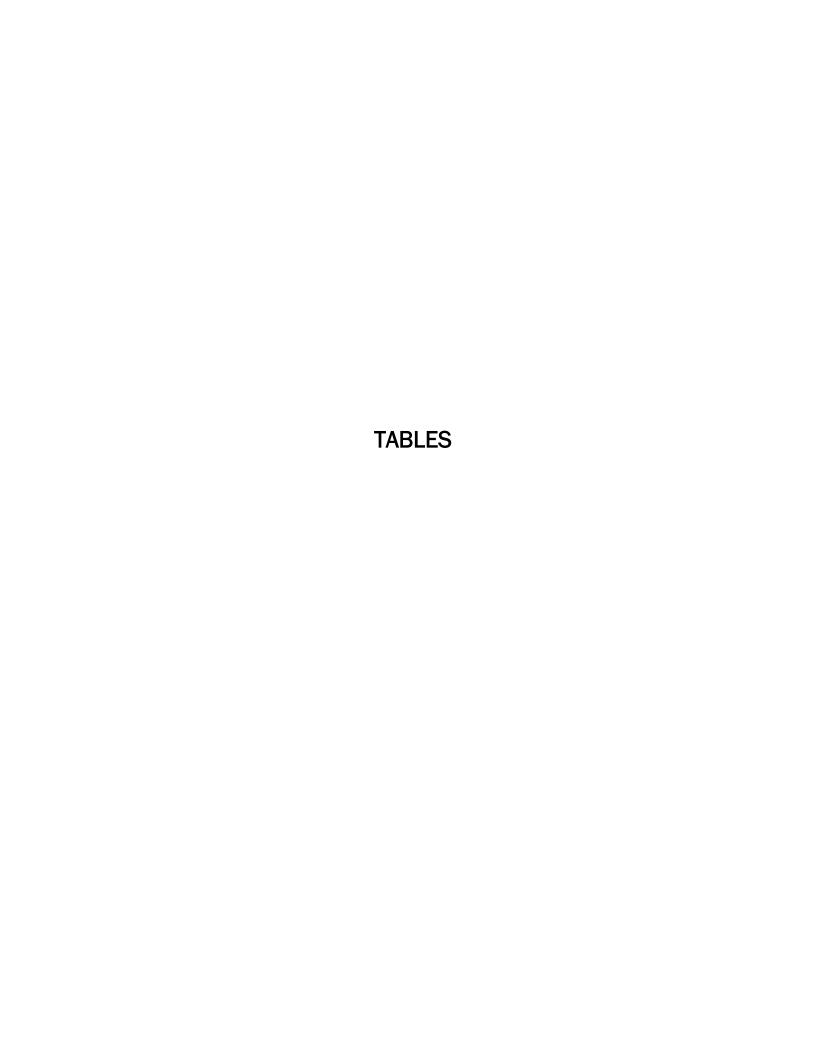


TABLE 1 R6-AMA Monitoring Well Construction Detail Summary Plant Yates

Well ID	Hydraulic Location	Easting	Northing	Ground Surface Elevation	Top of Casing Elevation	Total Well Depth (ft btoc)	Top of Screen Elevation	Bottom of Screen Elevation	Screen Length (ft)	Depth to Bedrock (ft bgs)	Geologic Description of Bedrock	Sep. 2020 Depth to Groundwater	Sep. 2020 Groundwater Elevation	Date Completed
YGWA-1I	Upgradient	2070097.9	1256876.1	834.3	836.60	53.60	793.30	783.30	10	35.0	Biotite Gneiss/Mica Schist	36.71	799.89	5/20/2014
YGWA-1D	Upgradient	2070104.6	1256867.3	834.9	837.25	128.85	759.20	709.20	50	34.0	Biotite Gneiss/Mica Schist	48.22	789.03	5/20/2014
GWA-2	Upgradient	2073510.0	1261383.1	803.1	805.62	52.02	763.80	753.80	10	32.0	Gneiss/Schist	34.98	770.64	4/17/2007
YGWA-2I	Upgradient	2070790.5	1256144.1	864.0	866.25	63.75	812.80	802.80	10	40.0	Biotite Gneiss/Granitic Gneiss	44.18	822.07	5/20/2014
YGWA-3I	Upgradient	2072024.2	1256405.2	794.0	796.55	59.05	747.70	737.70	10	40.0	Biotite Gneiss/Granitic Gneiss	53.32	743.23	5/20/2014
YGWA-3D	Upgradient	2072026.2	1256399.9	794.1	796.78	134.18	712.90	662.90	50	40.0	Biotite Gneiss/Granitic Gneiss	23.41	773.37	5/20/2014
YGWA-4I	Upgradient	2075455.6	1254436.7	781.9	784.21	48.81	745.70	735.70	10	30.5	Interlayered Grantic Gneiss/Biotite Gneiss	23.45	760.76	5/21/2014
YGWA-5I	Upgradient	2076218.9	1254400.0	782.1	784.54	58.94	735.90	725.90	10	41.0	Biotite Gneiss	19.82	764.72	5/21/2014
YGWA-5D	Upgradient	2076223.6	1254396.7	781.9	784.53	129.13	706.00	656.00	50	40.0	Biotite Gneiss	22.51	762.02	5/21/2014
YGWA-14S	Upgradient	2072537.2	1257828.6	746.8	748.76	34.96	724.10	714.10	10	n/a	n/a	17.37	731.39	5/20/2014
YGWA-17S	Upgradient	2076758.3	1257602.8	780.2	783.05	39.85	753.20	743.20	10	n/a	n/a	12.62	770.43	9/10/2015
YGWA-18S	Upgradient	2077015.3	1257116.1	787.6	790.57	39.97	760.90	750.90	10	n/a	n/a	20.39	770.18	9/8/2015
YGWA-18I	Upgradient	2077015.8	1257090.1	787.9	790.57	79.97	720.90	710.90	10	50.0	Granitic Gneiss	23.59	766.98	9/8/2015
YGWA-20S	Upgradient	2077410.4	1255531.6	764.6	767.12	29.52	747.90	737.90	10	n/a	n/a	11.44	755.68	9/29/2015
YGWA-21I	Upgradient	2076768.1	1255538.3	780.8	783.70	79.90	714.10	704.10	10	38.0	Granitic Gneiss	31.29	756.10	9/28/2015
YGWA-30I	Upgradient	2071107.1	1258421.9	760.1	762.58	59.48	713.40	703.40	10	31.0	Granitic Gneiss	48.47	714.11	9/23/2015
YGWA-39	Upgradient	2073865.6	1255717.1	815.6	818.19	68.59	760.10	750.10	10	33.0	Biotite Gneiss	21.81	796.38	7/7/2016
YGWA-40	Upgradient	2073431.3	1255792.0	813.5	815.73	48.23	778.00	768.00	10	36.0	Biotite Gneiss	25.44	790.29	7/7/2016
YGWA-47	Upgradient	2071818.1	1262411.8	755.6	758.22	59.19	709.60	699.60	10	36.0	Amphibolite Schist	33.38	724.84	7/11/2016
YGWC-23S	Downgradient	2074734.07	1256366.93	762.00	764.91	38.91	736.30	726.30	10	n/a	n/a	17.61	747.44	9/21/2015
YGWC-24SA	Downgradient	2073924.81	1258907.98	762.00	765.00	57.00	718.00	708.00	10	n/a	n/a	28.77	736.23	6/4/2020
YGWC-36A	Downgradient	2073748.73	1258547.74	737.70	740.88	51.20	699.70	689.70	10	n/a	n/a	7.30	733.58	9/22/2020
YGWC-38	Downgradient	2074446.80	1256108.38	797.10	799.69	49.59	760.10	750.10	10	37	Muscovite/Biotite gneiss	29.82	768.78	7/23/2016
YGWC-41	Downgradient	2073274.41	1256510.62	801.10	803.92	66.82	747.10	737.10	10	39	Granitic Gneiss	26.91	777.01	7/8/2016
YGWC-42	Downgradient	2073326.52	1257882.87	795.10	797.86	59.76	748.50	738.50	10	30	Granitic Gneiss	27.48	770.38	7/8/2016
YGWC-43	Downgradient	2073199.65	1257547.41	742.30	744.96	79.66	675.80	665.80	10	28	Granitic gneiss	15.11	729.85	7/9/2016
YGWC-49	Downgradient	2074337.51	1259375.23	780.10	782.73	78.53	715.10	705.10	10	n/a	n/a	31.00	751.73	7/13/2016
	_				1			ork Locations	•					
YGWA-6S	Water Level Only	2074786.49	1260484.87	779.80	782.47	39.87	752.90	742.90	10	n/a	n/a	18.22	764.25	5/19/2014
YGWA-6I	Water Level Only	2074790.49	1260490.02	780.20	782.73	69.03	724.00	714.00	10	48.0	Granitic Gneiss/Mica Schist	18.48	764.25	5/19/2014
PZ-04S	Water Level Only	2075454.20	1254442.86	781.80	784.25	32.75	761.80	751.80	10	n/a	n/a	24.95	759.30	5/21/2014
PZ-05S	Water Level Only	2076211.43	1254404.42	782.20	784.64	41.94	753.00	743.00	10	n/a	n/a	19.81	764.83	5/21/2014
PZ-06D	Water Level Only	2074782.68	1260480.15	779.50	782.02	134.02	698.30	688.30	10	46.5	Granitic and Biotite Gneiss	21.43	760.59	5/19/2014
PZ-24IA	Water Level Only	2073930.07	1258910.76	761.80	764.65	89.53	684.80	674.80	10	59.0	Granitic Gneiss	29.13	735.20	6/3/2020
PZ-35	Water Level Only	2073805.60	1258593.16	740.90	743.81	50.01	704.90	694.90	10	42.0	Biotite Gneiss	15.44	728.37	7/20/2016
PZ-37	Water Level Only	2074699.59	1256471.14	758.00	760.78	49.78	721.50	711.50	10	37.0	Biotite Gneiss	13.79	746.40	7/6/2016
PZ-48	Water Level Only	2074528.00	1259868.04	777.20	779.83	58.73	731.40	721.40	10	36.0	Biotite Gneiss	19.94	779.89	7/12/2016
PZ-51	Water Level Only	2073182.55	1257595.80	741.30	744.30	36.32	717.98	707.98	10	4.0	Gneiss/Schist	6.94	737.36	11/8/2019
YAMW-1	Water Level Only	2073814.55	1258602.12	740.90	743.83	69.93	683.90	673.90	10	44.0	Biotite Gneiss	15.14	728.69	9/19/2018
YAMW-2	Water Level Only	2072924.89	1256780.59	777.90	781.04	46.48	744.56	734.56	10	39.0	Gneiss/Schist	22.18	758.86	11/12/2019
YAMW-3	Water Level Only	2073345.21	1256915.25	793.20	796.05	91.44	714.61	704.61	10	29.0	Gneiss/Schist	35.28	760.77	11/6/2019
YAMW-4	Water Level Only	2073280.71	1256532.64	802.60	805.59	96.55	719.04	709.04	10	45.0	Gneiss/Schist	30.61	774.98	11/7/2019
YAMW-5	Water Level Only	2074486.69	1256140.21	785.90	788.90	90.34	708.56	698.56	10	29.0	Gneiss/Schist	12.84	776.06	11/13/2019

Notes:

- 1. ft BTOC indicates feet below top of casing
- 2. ft bgs indicates feet below ground surface
- 3. ft indicates feet
- 4. Northings and Eastings are GA State Plane West (NAD83)

TABLE 2A HORIZONTAL HYDRAULIC CONDUCTIVITY DATA SUMMARY

		Hydraulic	Hydraulic	Hydraulic
		Conductivity	Conductivity	Conductivity
Location	Test	(cm/sec)	(ft/day)	(ft/yr)
PZ-4S	Slug-In Test	7.41E-04	2.10	767
FZ-43	Slug-Out Test	1.52E-04	0.43	157
YGWA-4I	Slug-In Test	9.17E-05	0.26	95
TGWA-41	Slug-Out Test	8.11E-05	0.23	84
PZ-5S	Slug-In Test	N/A	N/A	N/A
PZ-55	Slug-Out Test	4.34E-04	1.23	449
YGWA-5I	Slug-In Test	N/A	N/A	N/A
TGWA-SI	Slug-Out Test	3.77E-04	1.07	391
YGWA-5D	Slug-In Test	1.16E-04	0.33	121
TGWA-SD	Slug-Out Test	1.06E-04	0.30	110
YGWA-17S	Slug-In Test	3.42E-04	0.97	354
1GWA-175	Slug-Out Test	3.46E-04	0.98	358
YGWA-18S	Slug-In Test	9.52E-05	0.27	99
1GWA-185	Slug-Out Test	8.47E-05	0.24	88
YGWA-18I	Slug-In Test	7.76E-04	2.20	804
YGWA-181	Slug-Out Test	5.12E-04	1.45	530
VCWC 100	Slug-In Test	3.39E-04	0.96	351
YGWC-19S	Slug-Out Test	N/A	N/A	N/A
YGWA-20S	Slug-In Test	3.63E-04	1.03	376
1GWA-203	Slug-Out Test	2.22E-04	0.63	230
YGWC-22S	Slug-In Test	3.14E-04	0.89	325
1GWC-225	Slug-Out Test	2.93E-04	0.83	303
VOWO 000	Slug-In Test	2.15E-04	0.61	223
YGWC-23S	Slug-Out Test	2.40E-04	0.68	248
VOWO 040	Slug-In Test	2.79E-04	0.79	289
YGWC-24S	Slug-Out Test	2.93E-04	0.83	303
PZ-35	Slug-In Test	3.12E-03	8.87	3228
PZ-30	Slug-Out Test	2.05E-03	5.82	2117
YGWC-32S	Slug-In Test	1.45E-03	4.11	1498
1GW0-323	Slug-Out Test	2.39E-03	6.80	2474
VOMO 201	Slug-In Test	1.88E-03	5.33	1942
YGWC-32I	Slug-Out Test	2.39E-03	6.79	2471
	Slug-In Test	2.37E-03	6.73	2451
YGWC-33S	Slug-Out Test	1.58E-03	4.50	1639
	Slug-In Test	1.99E-03	5.64	2054
YGWC-34I	Slug-Out Test	1.78E-03	5.07	1846
	Slug-In Test	2.34E-04	0.67	243
YGWC-36	Slug-Out Test	2.46E-04	0.70	255
	Slug-In Test	2.11E-03	6.01	2187
PZ-37	Slug-Out Test	1.65E-03	4.70	1711
	Slug-In Test	2.08E-04	0.59	215
YGWC-38 (PZ-38)	Slug-Out Test	1.73E-04	0.49	179
	_			
YGWA-39 (PZ-39)	Slug-In Test	1.81E-03	5.15	1873
	Slug-Out Test	1.89E-03	5.36	1951
YGWA-40 (PZ-40)	Slug-In Test	5.79E-04	1.65	599
	Slug-Out Test	7.20E-04	2.05	745
YGWC-42	Slug-In Test	2.20E-04	0.63	228
	Slug-Out Test	1.47E-04	0.42	152
YGWC-43	Slug-In Test	3.66E-03	10.42	3791
	Slug-Out Test	2.61E-03	7.43	2705
PZ-48	Slug-In Test	6.54E-04	1.86	677
-	Slug-Out Test	5.58E-04	1.59	577
YGWC-49	Slug-In Test	6.92E-04	1.97	716
	Slug-Out Test	8.00E-04	2.27	828
Slug Test Geometric Mean		5.12E-04	1.45	530

Notes:

- 1. Slug Test on locations -1 through -16 completed in 2014, -17 through -30 in 2015, -31 and higher performed by ACC, Inc. personnel 3/2-3/9, 2017.
- 2. N/A = Result not available.
- 3. cm/sec = centimeters per second; ft/day = feet per day; ft/yr = feet per year

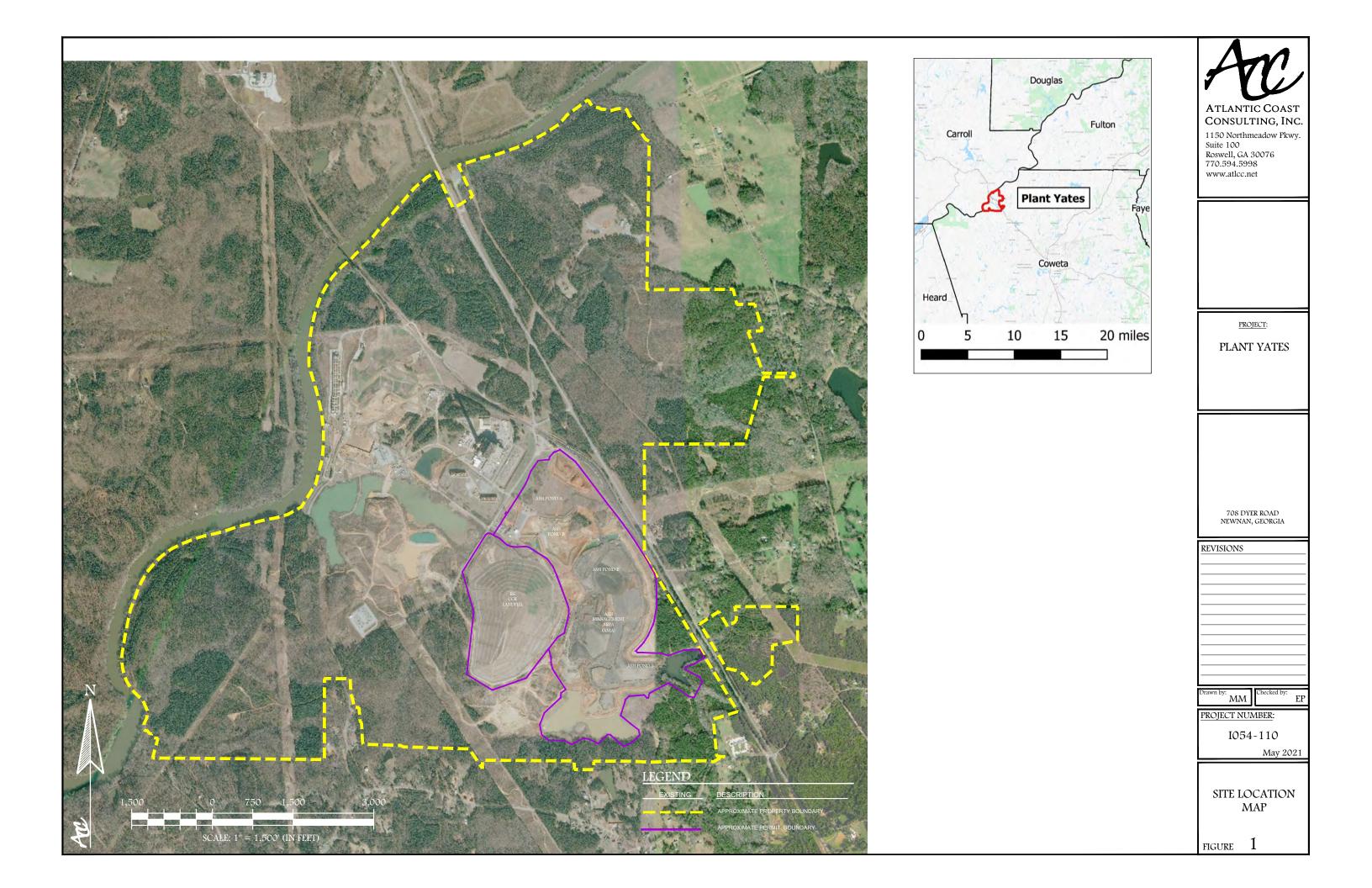
TABLE 2B
Sitewide Vertical Hydraulic Conductivity Data Summary

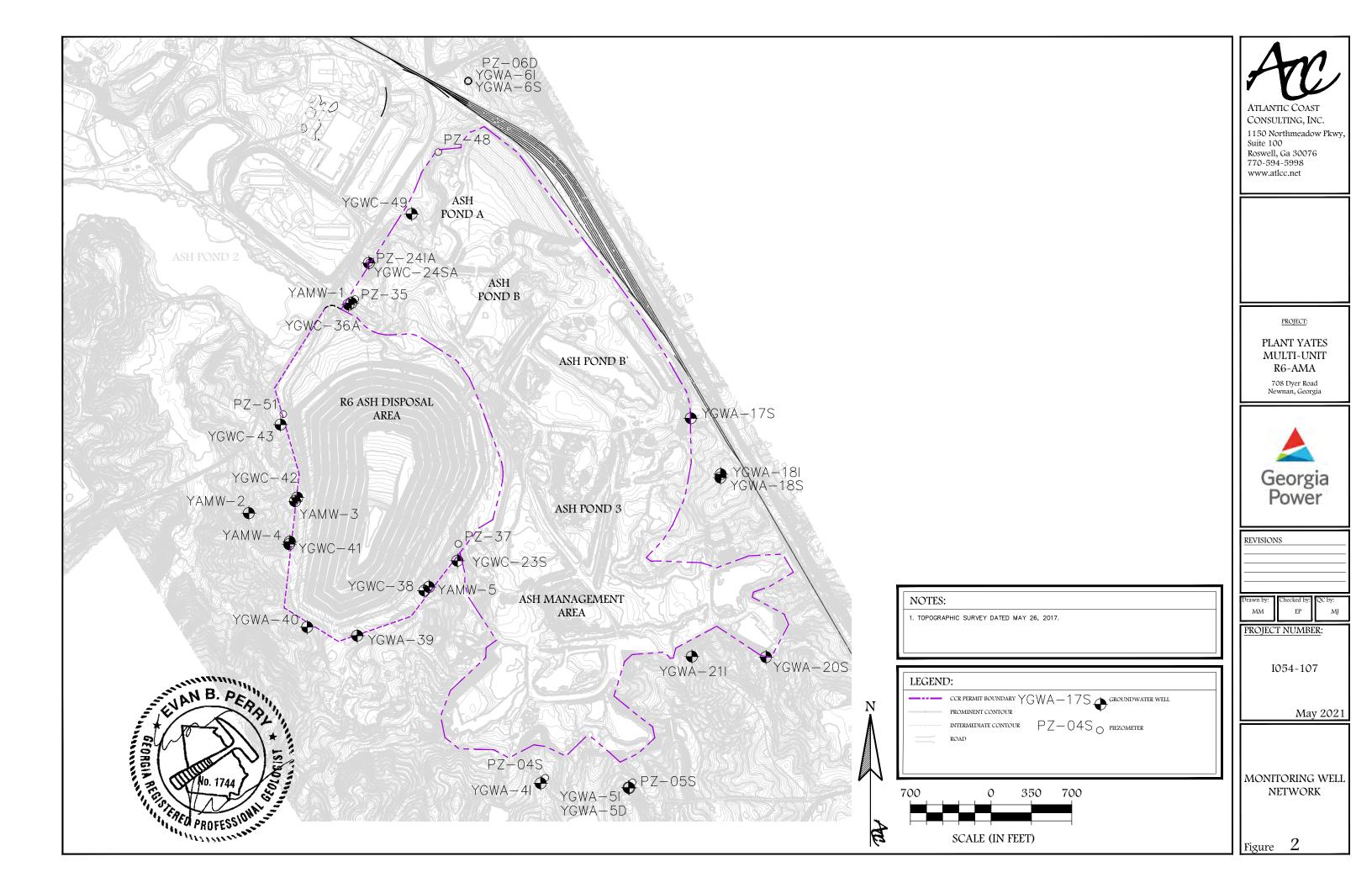
Location	Depth (ft bgs)	Hydraulic Conductivity (cm/sec)	Hydraulic Conductivity (ft/day)	Hydraulic Conductivity (ft/yr)
YGWA-17S	17 - 19	6.91E-04	1.96	715
YGWC-19S	17 - 19	1.78E-04	0.50	184
YGWA-20S	17 - 19	9.72E-05	0.28	101
YGWC-22S	7 – 9	1.63E-03	4.62	1688
YGWC-22S	17 - 19	4.66E-04	1.32	485
YGWC-24S	17 - 19	2.51E-03	7.11	2599
YGWC-24S	37 - 39	2.50E-05	0.07	26
PZ-25S	33 - 35	4.13E-05	0.12	43
PZ-25S	44 - 46	2.00E-04	0.57	207
YGWC-26S	17 - 19	1.79E-06	0.01	1.9
YGWC-26S	27 - 29	3.36E-05	0.10	35
YGWC-27S	17 - 19	4.58E-07	0.00	0.5
YGWC-27S	27 - 29	3.56E-06	0.01	3.7
YGWC-28S	17 - 19	2.08E-07	0.00	0.2
PZ-30S	27 - 29	1.38E-05	0.04	14
PZ-31S	44 - 46	7.85E-04	2.23	813
Geometric Mean		4.88E-05	0.14	51

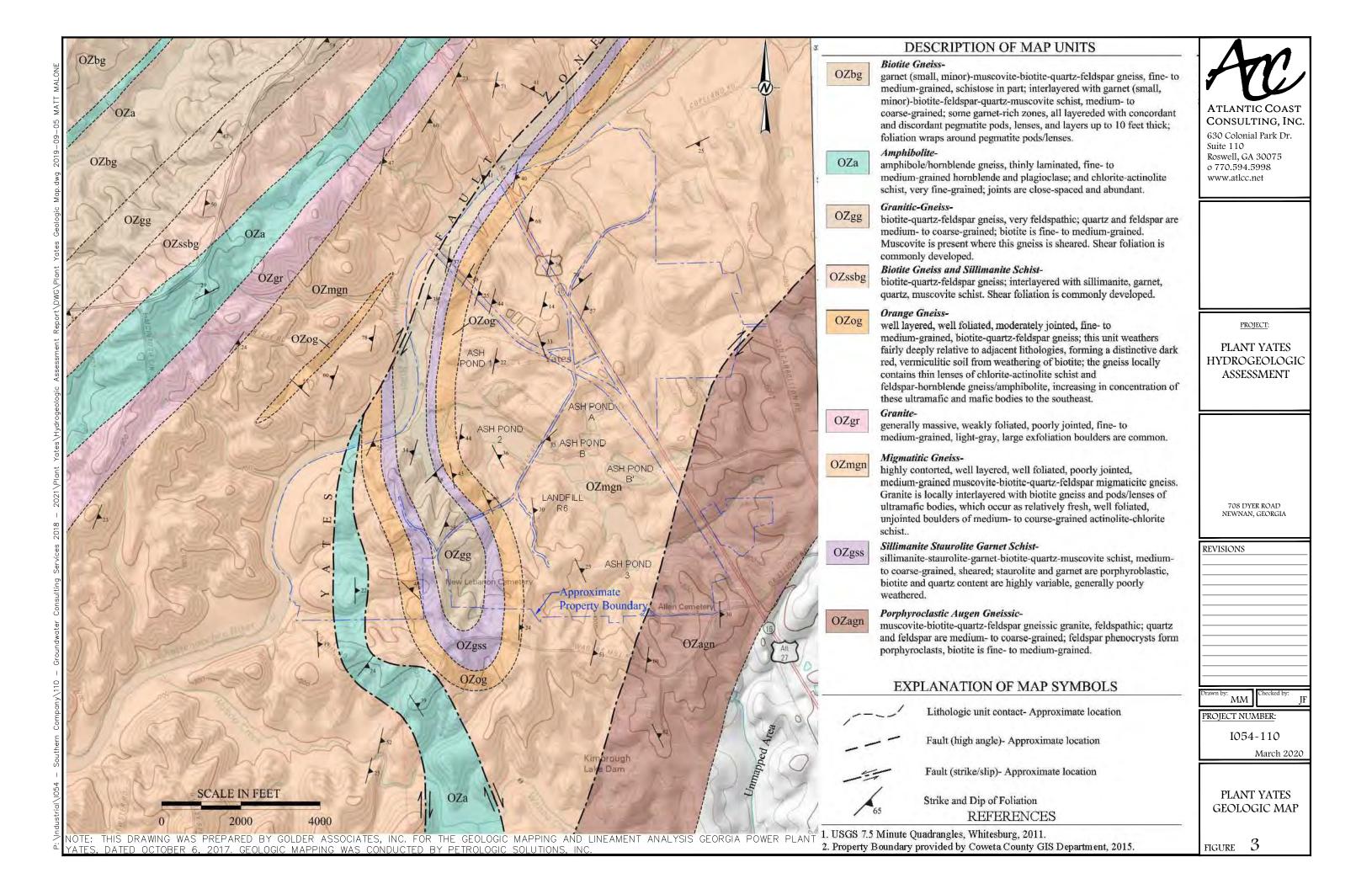
Notes:

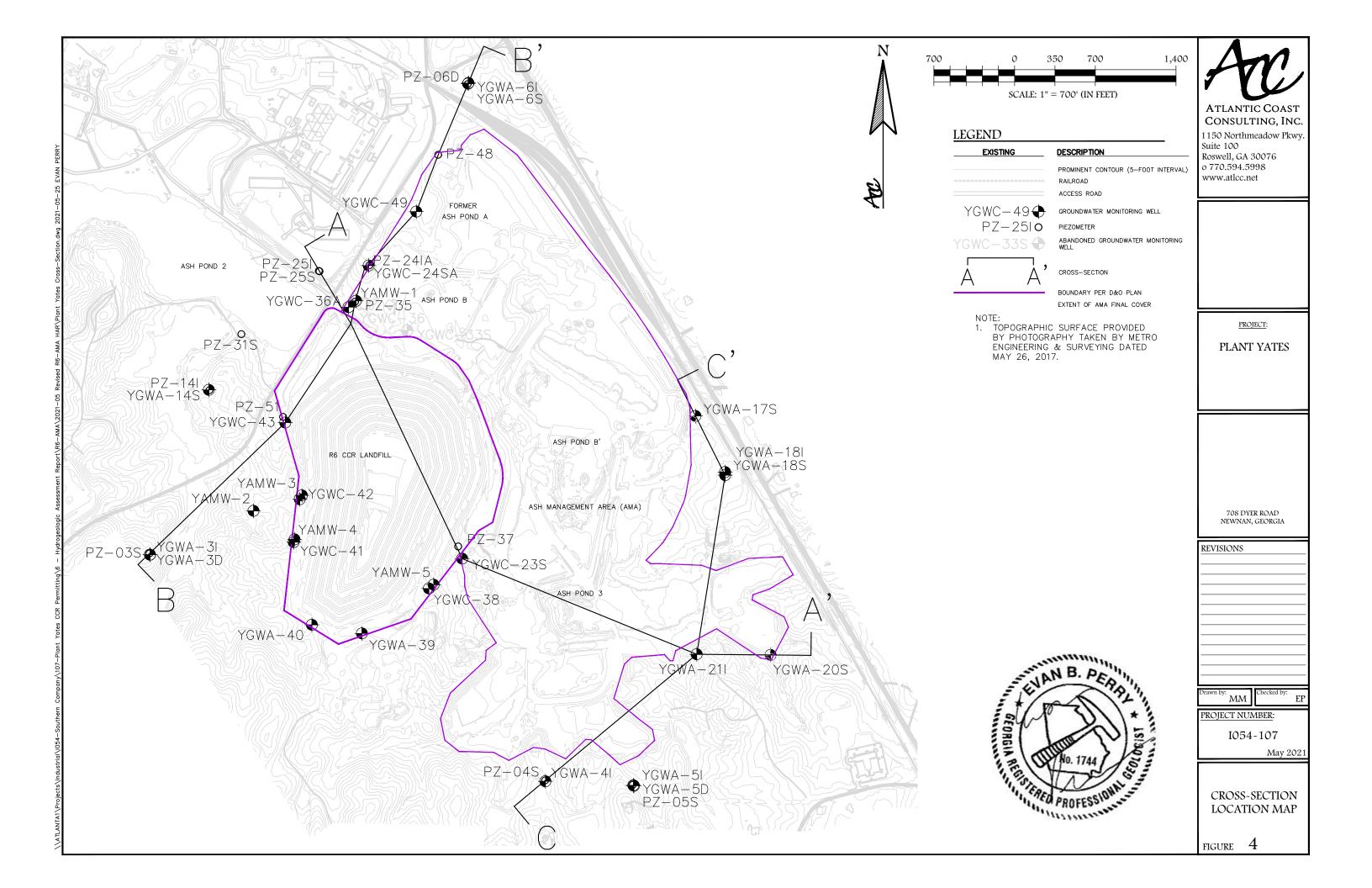
- 1. Data from Shelby Tube sample analysis completed by Cardno ATC, 2015.
- 2. All locations original IDs were originally pre-fixed with "PZ"; pre-fixes of locations incorporated into a groundwater monitoring network were changed to "YGWA" or "YGWC" as appropriate.
- 3. ft bgs = feet below ground surface
- 4. cm/sec = centimeters per second; ft/day = feet per day; ft/yr = feet per year

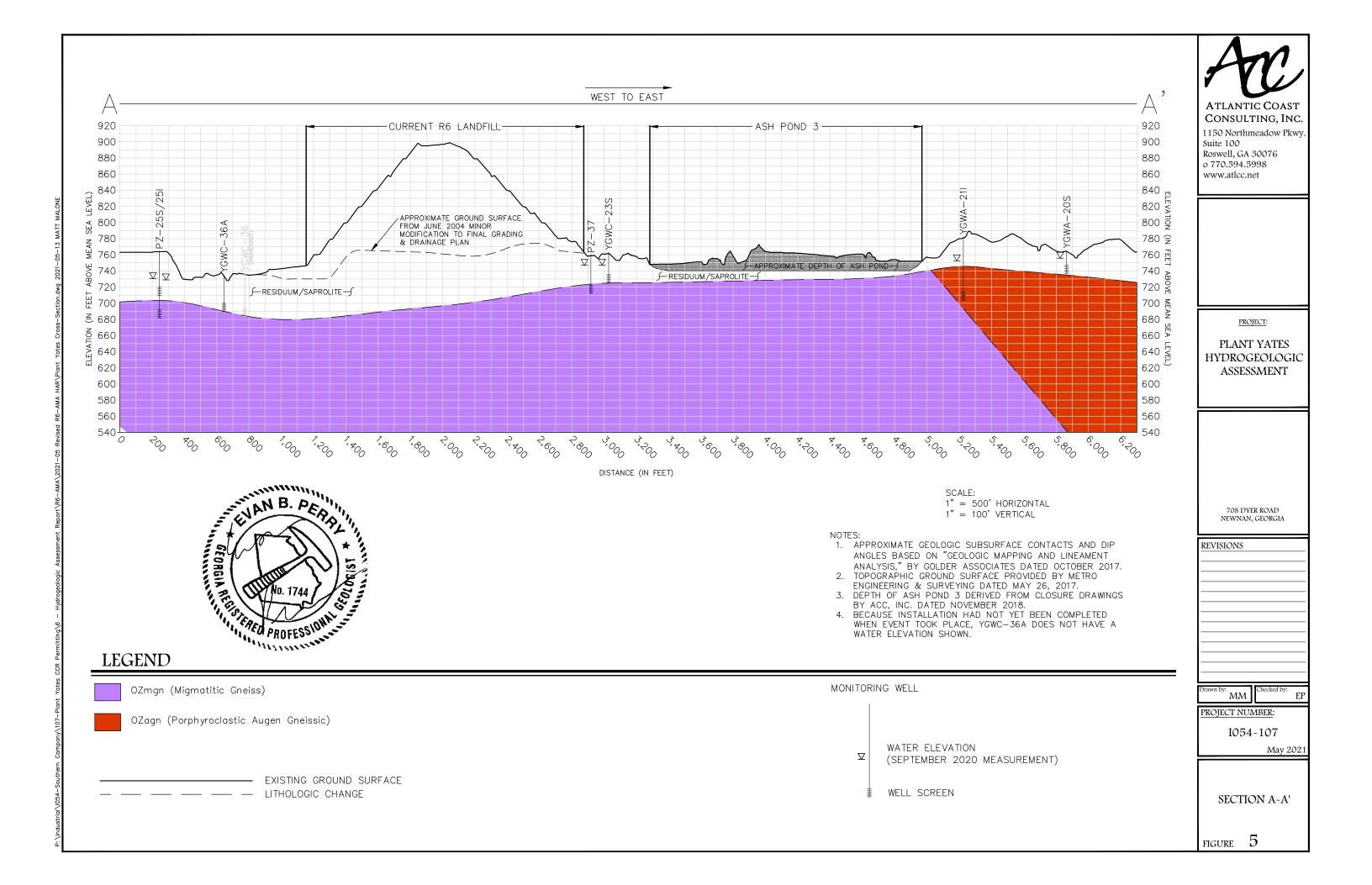


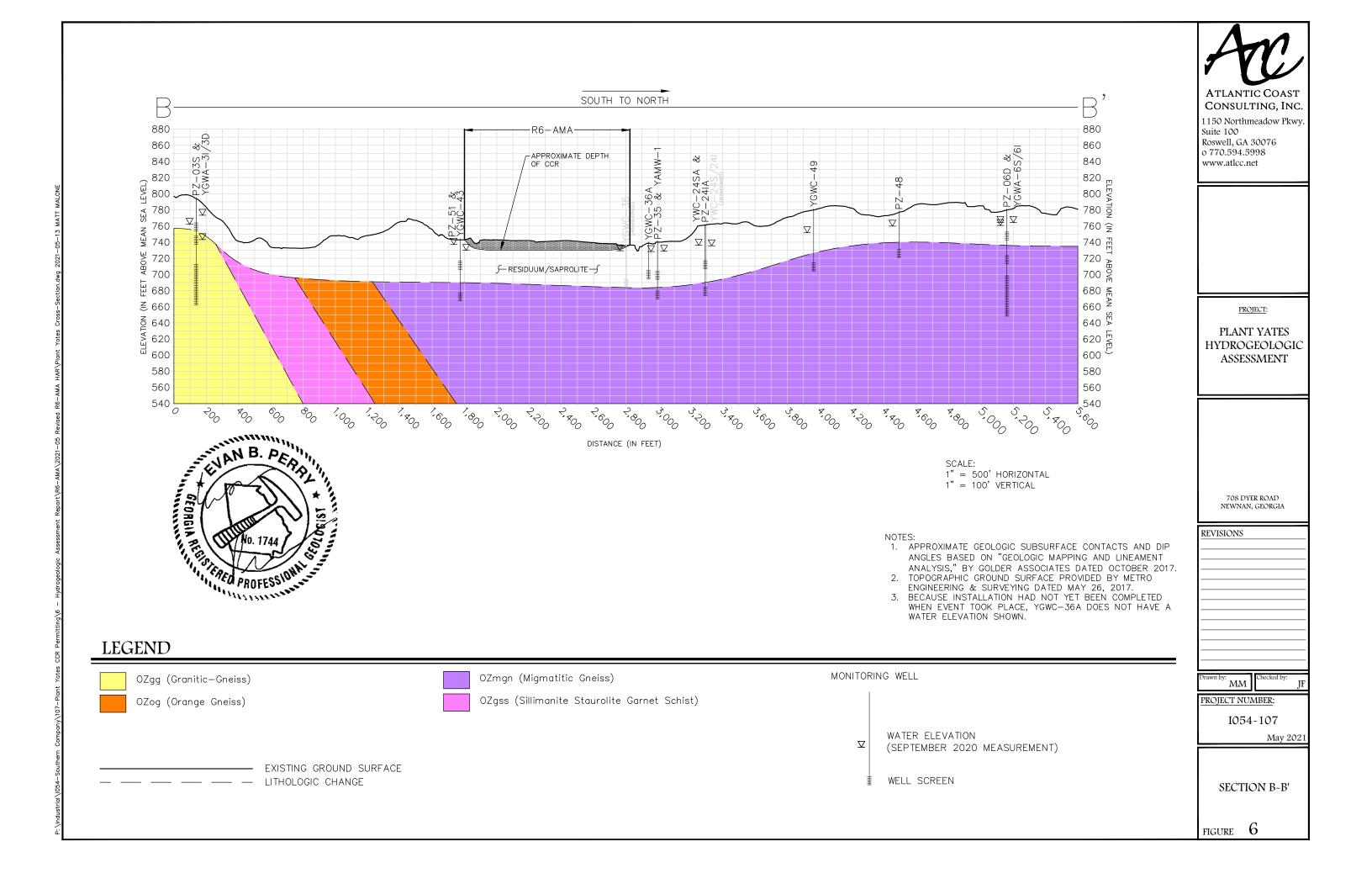


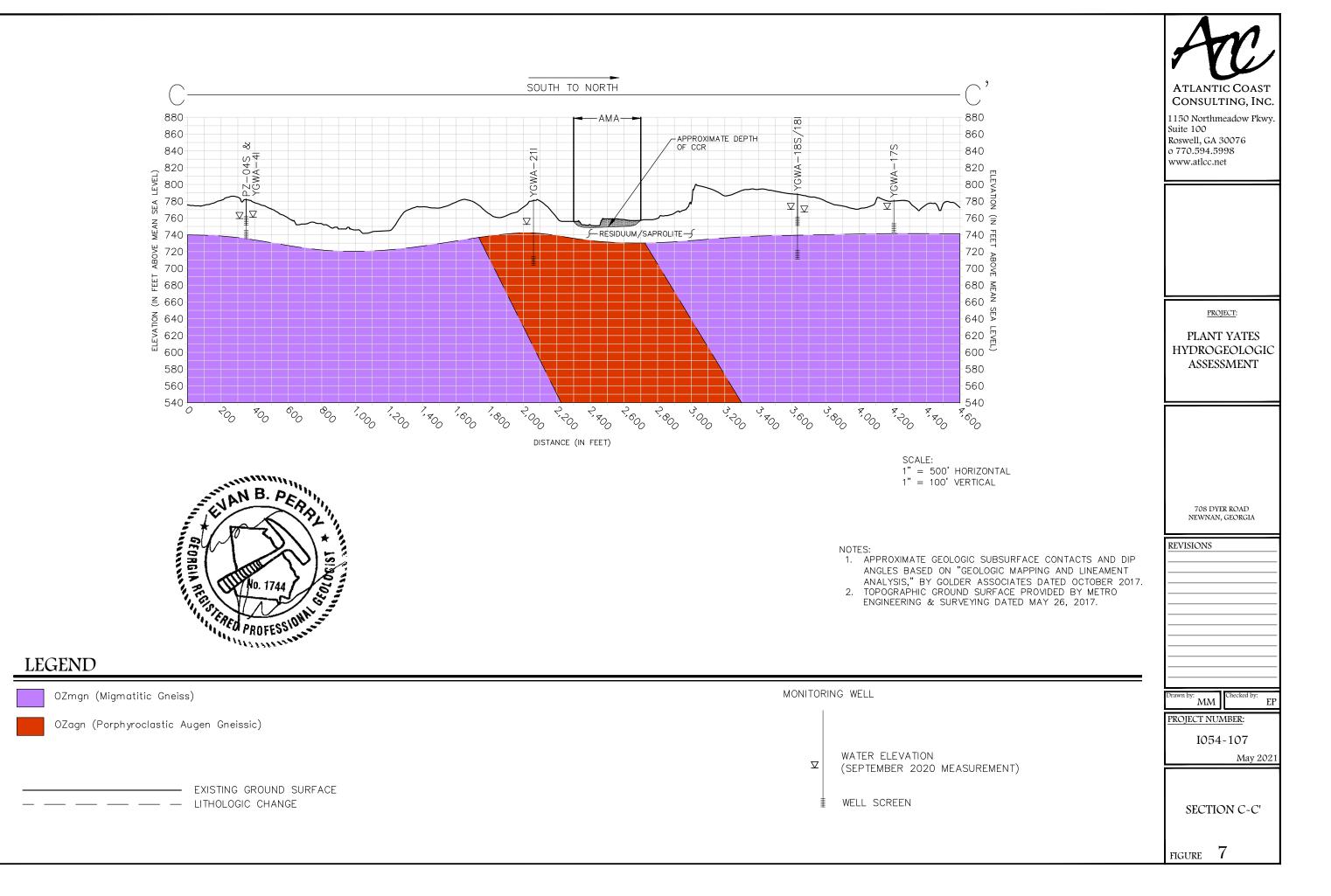




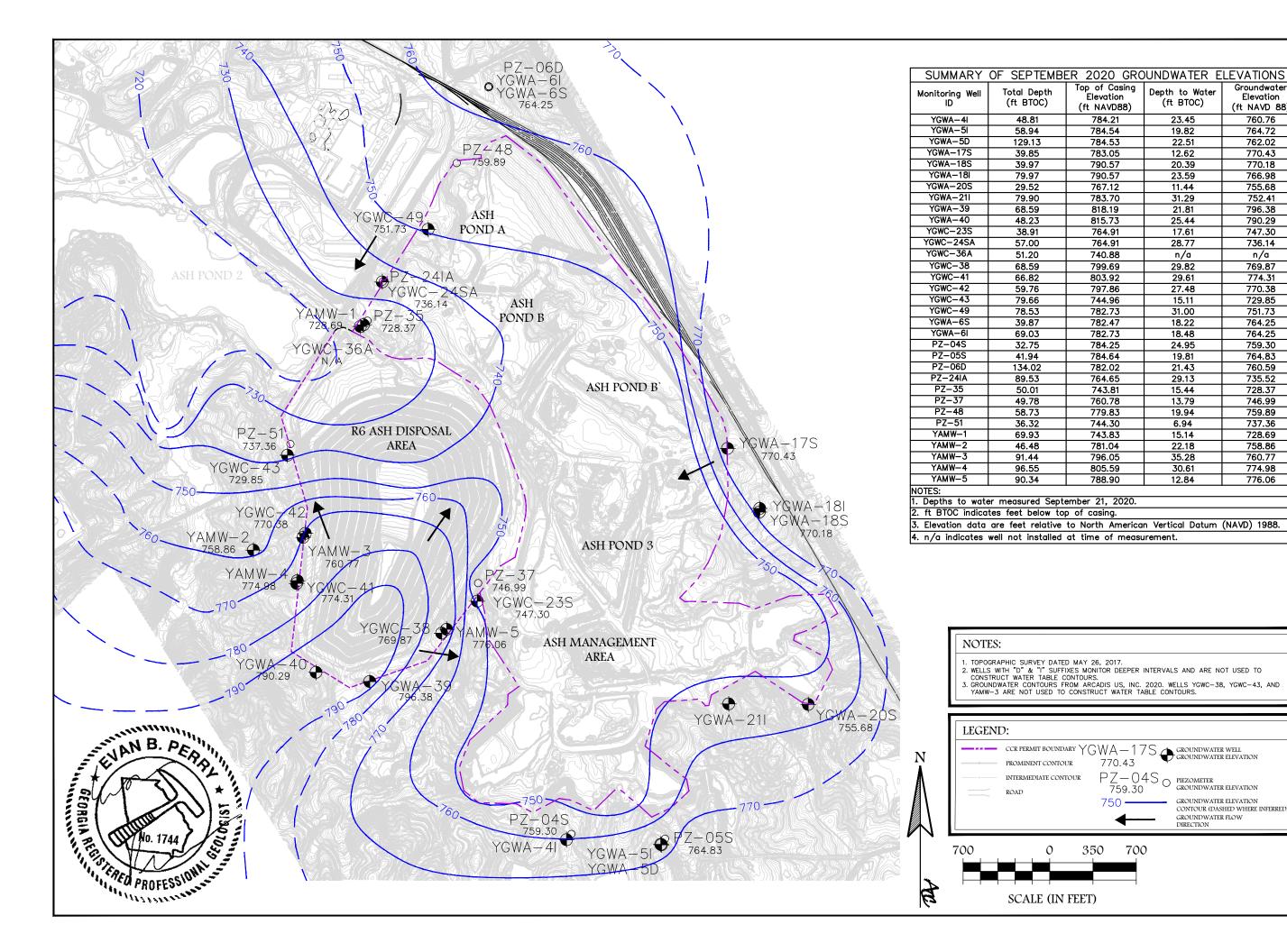








P:\Industrial\1054—Southern Company\107—Plant Yates CCR Permitting\6 — Hydrogeologi



ATLANTIC COAST CONSULTING, INC. 1150 Northmeadow Pkwy, Suite 100 Roswell, Ga 30076 770~594~5998 www.atlcc.net

Groundwater

Elevation

(ft NAVD 88)

760.76

764.72

762.02

770.43

770.18

766.98

755.68

752.41

796.38

790.29

747.30

736.14

n/a

769.87

774.31

770.38

729.85

751.73

764.25

764.25

759.30

764.83

760.59

735.52

728.37

746.99

759.89

737.36

728.69

758.86

760.77

774.98

776.06

PROJECT:

PLANT YATES MULTI~UNIT R6~AMA

708 Dyer Road Newnan, Georgia



REVISIONS

MM

PROJECT NUMBER:

EP

I054~107

May 2021

SEPTEMBER 2020 **POTENTIOMETRIC** SURFACE CONTOUR MAP

Figure



APPENDIX A BORING AND WELL CONSTRUCTIONLOGS



STUDY.GP.

GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 19:25 - \\ALTRCFS02\X2DBSMEL\$\GINT\PLANT YATES HYDRO-GEOLOGICAL

LOG OF TEST BORING AND WELL INSTALLATION

BORING YGWA-4I
PAGE 1 OF 1
ECS37976

PROJECT Plant Yates Hydro-Geological Study SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING LOCATION Newnan, GA DATE STARTED 4/10/2014 COMPLETED 5/21/2014 SURF. ELEV. 781.9 COORDINATES: N:1,254,436.58 E:2,075,455.62 EQUIPMENT PS-150 METHOD Rotosonic CONTRACTOR Cascade Drilling ANGLE BEARING DRILLED BY D. Wilcox LOGGED BY B. Smelser CHECKED BY BORING DEPTH 46.5 ft. GROUND WATER DEPTH: DURING COMP. 17.72 ft. DELAYED **NOTES** Top of Casing Elevation = 784.21 STRATA DESCRIPTION **WELL DATA** Œ GRAPHIC DEPTH Surface: protective aluminum cover with bollards; 4-foot square concrete pad ELEV. (DEPTH) FLEV Sandy Silt (ML) -Surface Seal: concrete - rusty red, damp, medium stiff, low plasticity, very fine grain, 779.9 cohesive; micaceous; trace organics 779.4 (2.0)Silty Sand (SM) - reddish brown to light brown, dry, medium dense to loose, no plasticity, lower fine to upper medium grain, some to trace clay decreasing with depth; trace mica; trace organics - SM: medium to light brown to tan, dry, loose, no plasticity, lower fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; noncohesive; trace rock fragments (brittle); trace - SM: light brown to tan grading to reddish brown @ 15', damp, loose, no plasticity, lower fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; noncohesive; trace rock fragments (brittle); trace mica 15 Annular Fill: 90/10 Portland - SM: reddish brown to tan to white with a greenish tan zone @ Cement/Bentonite Powder ▼ approx. 18-20', moist, medium dense, no plasticity, lower fine to upper medium grain, saprolite, visible zones where saprolite has not completely broken down to residual soil and remnant rock fabric visible; zone of more competent saprolite observed; gravel sized rock fragments included; muscovite, biotite, chlorite phyllosilicates visible - SM: orangish brown to light gray to white, moist, medium dense, no plasticity, lower fine to upper medium grain, saprolite, increasing rock fragment size and abundance with depth; rock fragments range from coarse gravel to cobble size; angular fragments 30 751.4 Interlayered Granitic Gneiss and Biotite Gneiss (30.6)- light gray to white with rusty red to orangish brown staining, lower -Annular Seal: 3/8 Hole Plug fine to lower medium grain, medium hard, moderately weathered, trace banding, quartz, biotite, muscovite, plagioclase, hornblende; low 35 747.4 to moderate angle fractures visible; slight schistose foliation visible in -Filter: 20/30 Silica Sand some weathered zones; some zones highly weathered (34.5)745.7 (36.2)Well: 2" OD PVC (SCH 40) 40 - Interlayered Granitic Gneiss and Biotite Gneiss: light gray to white with zones of dark gray to black, upper fine to lower medium grain, hard, slightly weathered, trace banding, quartz, biotite, muscovite, Screen: 10 ft; pre-pack plagioclase, hornblende, trace garnet; thin zones (1-2 mm to 2-4 mm) of mica schist interlayered with the gneiss; low to moderate angled fractures; total healing observed with quartz fracture fill 735.7 735.4 =Sump:0.30 ft.



2012 GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 18:41 - \\ALTRCFS02X2DBSMEL\$\G\NT\PLANT YATES HYDRO-GEOLOGICAL STUDY.GPJ

LOG OF TEST BORING AND WELL INSTALLATION

BORING YGWA-5I
PAGE 1 OF 2
ECS37976

PROJECT Plant Yates Hydro-Geological Study SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING LOCATION Newnan, GA DATE STARTED 4/9/2014 **COMPLETED** 5/21/2014 **SURF. ELEV.** 782.1 **COORDINATES:** N:1,254,399.95 E: 2,076,218.86 **EQUIPMENT** PS-150 METHOD Rotosonic CONTRACTOR Cascade Drilling CHECKED BY _____ ANGLE _____ BEARING ____ DRILLED BY D. Wilcox LOGGED BY B. Smelser BORING DEPTH 56.5 ft. GROUND WATER DEPTH: DURING COMP. 13.66 ft. DELAYED **NOTES** Top of Casing Elevation = 784.54 STRATA DESCRIPTION **WELL DATA** Œ GRAPHIC DEPTH Surface: protective aluminum cover with bollards; 4-foot square concrete pad ELEV. (DEPTH) ELEV. Silty Sand (SM) Surface Seal: concrete - brown, dry, medium dense, no plasticity, upper fine to lower medium 780.1 grain, noncohesive; trace organics; angular to sub angular grains - SM: brown, dry, medium dense, no plasticity, upper fine to lower 144 (2.0)medium grain, slight increase in clay content with depth; cohesive (slight); trace mica; trace coarse grains Poorly-graded Sand (SP) - light gray to off white, dry, very loose, no plasticity, upper fine to upper medium grain, noncohesive; upper coarse to coarse gravel sized weathered rock fragments; angular to subangular grains; angular rock fragments - SP: med gray to tan to light gray /white with orangish brown to greenish gray mottling, damp, medium dense, no plasticity, upper fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; zones of more competent (completely ▼ weathered) rock increasing with depth; trace lenses of silt/clay interbedded within the sand/saprolite; brittle upper coarse to lower gravel sized rock fragments included; micaceous - SP: light gray/white grading to med gray with orangish brown to dark gray mottling, damp, medium dense, no plasticity, upper fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; angular to subangular grains 20 Annular Fill: 90/10 Portland Cement/Bentonite Powder 25 - SP: light gray to med gray, damp, loose, no plasticity, upper fine to lower medium grain, saprolite, increasing in gravel sized rock fragments (completely weathered, very brittle) 8 - SP: medium gray to light gray, damp, loose, no plasticity, upper fine to upper coarse grain, saprolite, noncohesive; completely weathered to residual soil with zones of more competent but brittle rock fragments; angular grains; micaceous



SOUTHERN.

LOG OF TEST BORING AND WELL INSTALLATION

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING PROJECT Plant Yates Hydro-Geological Study LOCATION Newnan, GA

DEPTH (ft) GRAPHIC LOG STRATA DESCRIPTION **WELL DATA** Surface: protective aluminum cover with bollards; 4-foot square concrete pad ELEV (DEPTH ELEV Partially Weathered Rock 741.1 741.8 - light gray, Pulverized Rock (powder) due to sonic drilling; no (40.3)describable sample Annular Seal: 3/8 Hole Plug (medium bentonite chips) **Biotite Gneiss** - light brown to light gray to white, upper fine to upper medium grain, 737.6 medium hard to soft, moderately weathered, banded, quartz, biotite, muscovite, plagioclase, hornblende, trace chlorite; low to moderate Filter: 20/30 Silica Sand (44.5)angled fractures; no visible healing/fracture fill 735.9 (46.2)Well: 2" OD PVC (SCH 40) - Biotite Gneiss: light gray to medium gray, upper fine to upper medium grain, medium hard to hard, moderately weathered, banded, quartz, biotite, muscovite, plagioclase, hornblende, trace chlorite; Screen: 10 ft; pre-pack increase in mafic minerals; orangish brown staining visible in zones; low to moderate angled fractures visible; no visible healing/fracture fill; slight schistose foliation observed in zones 725.9 725.6 Sump:0.30 ft 725.6

Bottom of borehole at 56.5 feet.



2012 GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 18:56 - \\alphaLTRCFS02X2DBSMEL\$\GINT\PLANT YATES HYDRO-GEOLOGICAL STUDY.GPJ

LOG OF TEST BORING AND WELL INSTALLATION

BORING YGWA-5D PAGE 1 OF 3 ECS37976

		COMPANY	AND WELL	. INSTA	LLA	MOLLA			
	SOLITI	HERN COMPANY SEI	RVICES INC	PROJECT	Plant	Yates Hydro	o-Geological Study		
			/IRONMENTAL ENGINEERING	LOCATION	New	nan, GA			
D	ATE ST	TARTED 4/11/2014	_ COMPLETED _5/21/2014 SURF	. ELEV 78	1.9	COOR	DINATES: N:1,25	4,396.67 E:2,076,223	,63
С	ONTRA	ACTOR Cascade Drillin	ng EQUIPMENT PS	150 ME	THOD	Rotosonic			
D	RILLED	D. Wilcox	LOGGED BY B. Smelser C	HECKED BY	<i></i>		ANGLE	BEARING	
В	ORING	DEPTH 126.5 ft.	$_{_}$ Ground water depth: During $_{_}$		COMF	9. <u>6.84 ft.</u>	DELAYED		
N	OTES	Top of Casing Elevation	on = 784.53						
						ı			
£		S.	TRATA DESCRIPTION				WELL I	ΣΑΤΑ	
프 ፲	GRAPHIC LOG	J	mann begon mon			Curtoso	******		
DEPTH (ft)	LC &					Surface: protective	aluminum cover with	bollards; 4-foot squa	re
						concrete p	ad		ELEV.
		Silty Sand (SM)			ELEV.				(DEPTH)
		- brown, dry, medium	n dense, no plasticity, upper fine to low				⊢Surface Seal: con	crete	770.0
		- SM: brown, dry, me	trace organics; angular to sub angular q edium dense, no plasticity, upper fine to	lower					(2.0)
		medium grain, slight (slight); trace mica; t	increase in clay content with depth; co	hesive					(2.0)
2					776.9				
		Poorly-graded Sand - light gray to off whit	ו (SP) te, dry, very loose, no plasticity, upper f	ine to					
		upper medium grain,	, noncohesive; upper coarse to coarse	gravel					
		sized weathered rock angular rock fragmer	k fragments; angular to subangular grai nts	ns;					
10		o o							
		- SP: med gray to tai	n to light gray /white with orangish brow	n to					
		fine to lower medium	ng, damp, medium dense, no plasticity, ngrain, saprolite, noncohesive; complet	ely					
			al soil; zones of more competent (comp reasing with depth; trace lenses of silt/c						
15		interbedded within th	ne sand/saprolite; brittle upper coarse to						
		gravei sized rock frag	gments included; micaceous						
		- SP: light gray/white	e grading to med gray with orangish bro	wn to dark					
		gray mottling, damp,	, medium dense, no plasticity, upper fin	e to lower					
 Q			olite, noncohesive; completely weathered to subangular grains	a to					
Ñ		-					Annular Fill: 90/10	Dortland	
							Cement/Bentonite		
25									
		- SP: light gray to me	ed gray, damp, loose, no plasticity, upp saprolite, increasing in gravel sized roo	er fine to					
		fragments (complete	ely weathered, very brittle)	·K					
8									
32		00 "		,					
.,		 SP: medium gray to to upper coarse grain 	o light gray, damp, loose, no plasticity, in, saprolite, noncohesive; completely w	upper fine reathered			<u> </u>		
		to residual soil with z	zones of more competent but brittle rocl						
		fragments; angular g	grains, micaceous						
6					742.1	K// K//			



BORING YGWA-5D PAGE 2 OF 3 ECS37976

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

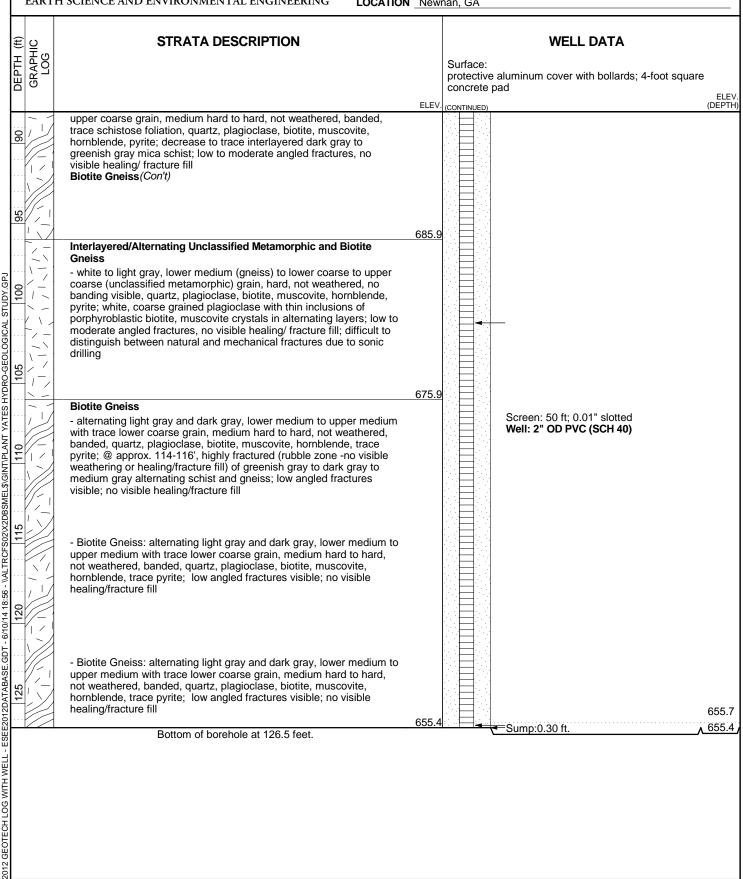
PROJECT Plant Yates Hydro-Geological Study

의유. <u>.</u>	STRATA DESCRIPTION		WELL DATA	
GRAPHIC LOG			Surface: protective aluminum cover with bollards; 4-foot square concrete pad	
1		ELEV.	' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' ' '	EL DEP
	Biotite Gneiss			
64	 light gray to white with light brown to tan staining, lower fine to upper fine grain, medium hard to soft, slightly to moderately weathered, banded, quartz, biotite, plagioclase, muscovite, hornblende, trace chlorite; low angle fracturing visible; 1-2 mm to 6-8 mm thick quartz fracture fill; moderate to partial healing; visible weathering characteristics include staining/discoloration and some mineral decomposition 			
	- Biotite Gneiss: light gray to white with light brown to tan staining, lower fine to upper fine grain, medium hard, moderately weathered, banded, quartz, biotite, plagioclase, muscovite, hornblende, trace chlorite; low to moderate angle fracturing visible; some quartz fracture fill visible; partial to no visible healing		Annular Fill: 90/10 Portland	
	 Interlayered/Alternating Biotite Gneiss and Mica Schist: alternating dark gray and white bands (Gneiss) interlayered with thin (1-2 mm to 5-6 mm) dark gray to greenish gray (Schist), upper fine to lower medium grain, medium hard to hard, not weathered, banded, slight schistose foliation associated with the interlayered mica schist, quartz, plagioclase, biotite, muscovite, hornblende, trace pyrite, trace chlorite; primarily low to moderate angled fracturing observed, difficult to distinguish between natural and mechanical fractures; no fracture healing visible; fracturing tends to occur along thinly interlayered zones of schist 		Cement/Bentonite Powder	
	- Interlayered/Alternating Biotite Gneiss and Mica Schist: alternating dark gray and white bands (Gneiss) interlayered with thin (1-2 mm to 5-6 mm) dark gray to greenish gray (Schist), upper fine to lower medium grain, medium hard to hard, not weathered, banded, slight schistose foliation associated with the interlayered mica schist, quartz, plagioclase, biotite, muscovite, hornblende, trace pyrite, trace chlorite; primarily low to moderate angled fracturing observed; no to partial healing visible; fracturing tends to occur along thinly interlayered zones of schist; @ approx. 66' and 74', 90-120 mm thick zones of white, localized, coarse grained plagioclase feldspar and quartz (Granulite? unclassified metamorphic) with thinly interlayered mica schist; no banding visible in the plagioclase/quartz zone; trace			71 (6
1/1/	augen plagioclase surrounded by flaky/bladed habit biotite and		<u>-</u>	70
<u> </u>	muscovite, around the zones associated with the coarse grain plagioclase		그 사람의 그림을 하는 것이 되었다.	(74
1/1/				
	 Interlayered/Alternating Biotite Gneiss and Mica Schist: alternating dark gray and white bands (Gneiss) interlayered with thin (1-2 mm to 5-6 mm) dark gray to greenish gray (Schist), upper fine to lower medium grain, medium hard to hard, not to slightly weathered, banded, slight schistose foliation associated with the interlayered mica schist, quartz, plagioclase, biotite, muscovite, hornblende, trace pyrite, trace chlorite; primarily low to moderate angled fracturing observed; no to partial healing visible; highly fractured (rubble zone) approx. 84-86' with some discoloration/staining 			70 (7)



BORING YGV

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING PROJECT Plant Yates Hydro-Geological Study





BORING YGWA-17S PAGE 1 OF 1 ECS37967

CONTRACT DRILLED BY BORING DE	TED 9/10/2015 COMPLETED 9/10/2015 SURF. OR Cascade EQUIPMENT L. Yancey LOGGED BY W. Shaughnessy C PTH 37 ft. GROUND WATER DEPTH: DURING 0 of casing elev: 783.05	METHOD	Rotosonic	
DEPTH (ft) GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	Weak HCL Moderate REACTION Strong GROUNDWATER OBSERVATIONS	WELL DATA Completion: protective aluminum cover with bollard 4-foot square concrete pad
10 15 20 25 30 35	sitty Sand (SM) - very pale brown (10YR 7/3) dry, fine to coarse-grained - mottled very pale brown (10YR 7/3) and white (10YR - pale brown (10YR 6/3) ✓ - brownish yellow (10YR 6/8) - light reddish brown (2.5YR 7/3) thin banding ✓ - white (10YR 8/1) wet, massive feldspar and quartz seelight reddish brown (2.5YR 7/3) - light reddish brown (2.5YR 7/4) wet ✓ - mottled pale yellow (2.5Y 8/3) and white / yellowish generated pale yellow (2.5Y 7/3) an	eam		Annular Fill: cement-bentonite grout Annular Seal: bentonite pellets Filter: silica filter sand Standpipe: 2" OD PVC (SCH 40) Screen: 10 ft; pre-pack

SOUTHERN COMPANY

SIMPLE GEOLOGY WITH WELL - ESEE DATABASE. GDT - 1/1/5/15 12:03 - NALTRCFP01/WSHAUGNE\$/DESKTOP/PLANTS PROJECTS/GEORGIA POWER/YATES/2015 PIEZOMETERS/YATES 2015 PZS.GPJ

LOG OF TEST BORING

BORING YGWA-18S PAGE 1 OF 1 ECS37967

		COMPA	IN Y								
			NY SERVICES, IN	NC. NTAL ENGINEERING		CT CCR					
EAF	анз	SCIENCE AN	DENVIRONME	NIAL ENGINEERING	LOCATI	ON Plan	t Yates				
DATE	STAF	RTED 9/4/20	15 COMPLE	ETED <u>9/8/2015</u> SI	URF. ELEV.	787.6	С	OORDIN	IATES: N	l:1.257.116.05 E:	2.077.015.25
			e								
DRILL	ED B	Y L. Yancey	LOGGED	BY W. Shaughnessy	CHECKED) BY			ANGLE	BEA	RING
BORII	NG DE	EPTH 37 ft.	GROUND	WATER DEPTH: DURIN							
NOTE	s To	p of casing ele	ev: 790.57								
DEPTH (ft)	GRAPHIC LOG		MATE	ERIAL DESCRIPTION		ELEVATION	Weak Moderate Strong	GROUNDWATER OBSERVATIONS	Comple protecti 4-foot s	WELL DATA etion: ve aluminum covo quare concrete pa	er with bollards; ad
	//	Clayey S								_ Surface Seal:	
	//	- yellowis	h red (5YR 5/8) d	dry, no, fine to medium-g	jrained					concrete	
	//	- with mid	ca								
5	//	2									
				7.5YR 6/8) and very paled, with mica	e brown (10Yi	R 8/4)					
10		- mottled (10YR 8/		7 7/4) and very pale bro	wn / very pale	e orange					
15		- moist								▲ Annular Fill: cement-bentor	nite grout
20		coarse-g coarse-g repaired coarse-g coarse-g coarse-g coarse-g coarse-g coarse-g coarse-g	light yellowish bro rained ve (5Y 6/3) very m light yellowish bro	own (2.5Y 6/3) and whit moist, fine to medium-gra own (2.5Y 6/3) and dark 3), dark olive brown (2.5	ained k olive brown	(2.5Y					
25		- wet								Annular Seal: bentonite pelle	ets
		4								Filter:	d
		- wet Clayey S	ilty Sand (SC-SM))					旧目	Smod mor odn	-
30		1	, ,	(5Y 6/4) saturated, fine t	o coarse-grai	ned, with					
		- mottled	olive / moderate o	olive brown (5Y 4/4) and	white (N9)					Standpipe:	O.I. 40\
		- pale ye	llow (5Y 8/2)							Screen: 10 ft; pre-pack	,
35		- banded	olive brown (2.5)	Y 4/4) and white (N9)							
		- regolith			-> / 0/5				H II		
		- mottled		7/3) and pale yellow (2.5 m of borehole at 37.0 fee						—Şump:0.29999	999999999999999 ft.
			DUILON	in or porenote at 37.0 let	,t.						
40											



SIMPLE GEOLOGY WITH WELL - ESEE DATABASE. GDT - 11/15/15 12:03 - NALTRCFP01/WSHAUGNE\$/DESKTOP/PLANTS PROJECTS/GEORGIA POWERNYATES/2015 PIEZOMETERS/YATES 2015 PZS.GPJ

LOG OF TEST BORING

BORING YGWA-18I PAGE 1 OF 2 ECS37967

	COMPANY					
	IERN COMPANY SE	ERVICES, INC.	PROJECT CCR	Piezomete	rs	
EARTH	SCIENCE AND EN	VIRONMENTAL ENGINEERING	LOCATION Plan	t Yates		
		COMPLETED 9/8/2015 SURF.			_	N:1,257,090.05 E: 2,077,015.82
	CTOR Cascade					
		LOGGED BY _W. Shaughnessy CI				
ORING I	DEPTH <u>77 ft.</u> Fop of casing elev: 79	_ GROUND WATER DEPTH: DURING _	COMI	P. <u>19 ft.</u>	DELAY	ED 18.5 ft. after 24 hrs.
OTES _	rop or casing cicv. 75					
(f) (R) GRAPHIC		MATERIAL DESCRIPTION	ELEVATION	Weak Moderate REACTION Strong GROUNDWATER	protect	WELL DATA letion: ctive aluminum cover with bollards; square concrete pad
- /	Clayey Sand (S	SC)		5 2 0 C		
	- olive / light oli	ive brown (5Y 5/6) moist, fine to coarse-g	rained			Surface Seal: concrete
/ /						
_	Silty Sand (SM	n.		1 : :		
5 1	-1.]	l) g brown(7.5YR 5/6) and very pale browl	n / gravish			
		7/4) dry, fine to coarse-grained, with mica				
	- pale yellow (2	2.5Y 8/3) dry, some residual quartz grave	d .			
0						
[]	- pale yellow (2	2.5Y 7/3)				
	년 11					
15	- mottled pale y	yellow (2.5Y 7/3) and yellow (2.5Y 7/6)				
	- mottled light b	prownish gray (2.5Y 6/2) and light gray (2.5Y 7/1) damp			
1.1						
	Clayey Silty Sa		rov (EV 9/1) wet			
20	fine to coarse-o	yellow(2.5Y 7/4) and white / yellowish g grained, massive white quartz+feldspar (d	completely			Annular Fill: cement-bentonite grout
	weathered), wit	th mica yellow (2.5Y 7/3) and white (2.5Y 8/1)				y comem comem groun
	- mottied paid y	7010W (2.51 775) and write (2.51 6/1)				
25	- white / yellow	ish gray (5Y 8/1) fine to coarse-grained, r r (completely weathered), with mica	nassive white			
	- pale olive (5)	(6/3)				
	Ä					
	- banded light o	blive gray (5Y 6/2) and white (2.5Y 8/1)	wet, fine to			
30	coarse-grained		•			
	- banded light y	vellowish brown (2.5Y 6/3) and white (2.5	5Y 8/1)			
35						
	- saprolite					×
	1.1	gray (2.5Y 7/2) and white (2.5Y 8/1) wet,	, fine to coarse-			
· · · · · / / :	grained	, , , , , , , , , , , , , , , , ,			\sim	%



LOG OF TEST BORING



GRAPHIC LOG

DEPTH (ft)

45

50

55

60

65

70

75

80

85

SIMPLE GEOLOGY WITH WELL - ESEE DATABASE GDT - 11/15/15 12:03 - NALTRCFP01WSHAUGNE\$/DESKTOP/PLANTS PROJECTS/GEORGIA POWER/YATES/2015 PIEZOMETERS/YATES 2015 PZS, GP,

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

Clayey Silty Sand (SC-SM) (Con't)

massive quartz+feldspar

3/1) weathered schist seam

silty sand (weathered zones)

planes, feldspar, quartz, mica, water stained

- brownish yellow (10YR 6/8) and white (10R 8/1)

Bottom of borehole at 77.0 feet.

- highly weathered, water stained

Silty Sand (SM)

Grantic gneiss

weathered

weathered

sub-horizontal

- thinly foliated

competent rock

seam

- mottled light gray (2.5Y 7/2) and white (2.5Y 8/1)

MATERIAL DESCRIPTION

PAGE 2 OF 2 **PROJECT** CCR Piezometers **LOCATION** Plant Yates HCL REACTION 3ROUNDWATER 3BSERVATIONS **WELL DATA** ELEVATION protective aluminum cover with bollards: **Joderate** 4-foot square concrete pad - mottled light gray (2.5Y 7/2) and white / yellowish gray (5Y 8/1) - mottled dark yellowish brown (10YR 4/6) and very dark gray (10YR - brown (10YR 5/3) wet, cohesive, fine to coarse-grained Annular Fill: - transition zone, quartz, interbedded with mica schist cement-bentonite grout - pale yellow (2.5Y 7/3) slightly to completely weathered, with gravelly - dark yellowish brown (10YR 4/6) and pale yellow (2.5Y 7/3) fine to coarse grain, medium hard, slightly to completely weathered - yellowish brown (10YR 5/6) and very dark greenish gray (10G 3/1) coarse grain, soft to medium hard, highly weathered, thinly foliated, moderately fractured, fractures sub-horizontal, separates at foliation - dark greenish gray (10BG 4/1) and light bluish gray (5PB 7/1) slightly - yellowish brown (10YR 5/6) and very dark gray (10YR 3/1) highly Annular Seal: bentonite pellets Filter: silica filter sand - grayish brown (10YR 5/2) and black (5Y 2.5/1) coarse grain, moderately weathered, thinly foliated, moderately fractured, fractures Standpipe: 2" OD PVC (SCH 40) - white (10R 8/1) massive feldspar and quartz seam Screen: 10 ft; pre-pack - grayish brown (10YR 5/2) and white (10R 8/1) massive quartzite - bluish gray (5PB 6/1) and white (10R 8/1) not weathered, fresh Sump:0.299999999999997 ft.



BORING YGWA-20S

ECS3/9	=		ING	TEST BOR	LOG OF	MPANY			-
		eters	Piezom	PROJECT CCR F	RVICES, INC.	COMPANY SE			SOI
			t Yates	LOCATION Plant	TRONMENTAL ENGINEERING				
410.27	ATEC: N:1 255 521 55 E: 2 077 /	OODDIN	C	E ELEV 7646	COMPLETED 9/29/2015 SUR	0/29/2015	TEN	CTAD	^TE
,410.37	ATES: N. 1,255,531.55 E. 2,077,4			METHOD					
	ANGLE BEARING				LOGGED BY W. Shaughnessy				
					GROUND WATER DEPTH: DURING				
	<u> </u>	<u></u>		<u> </u>	7.12	casing elev: 76	p of c	To	OTE:
bollards	WELL DATA Completion: protective aluminum cover with the definition of the desired concrete pad	GROUNDWATER OBSERVATIONS	Weak Moderate HCL Strong	ELEVATION	MATERIAL DESCRIPTION			GRAPHIC LOG	(ft)
		00	≤ ≥ ω	:	nd (SC-SM)	Clayey Silty Sa	(
	Surface Seal: concrete			e gravel	rown (2.5Y 4/2) wet, fine grained on (10YR 7/3) and yellowish brown / r 4) fine to coarse-grained, with quartzit	- dark grayish b - very pale brow brown (10YR 5	- - k		
out	Annular Fill: cement-bentonite grou			, ,	ale brown (10YR 7/3) and reddish ye	moist - moist	r ▼ -		5
				2)	ellowish brown (2.5Y 6/3) and pale yo to coarse grained rown / dark yellowish brown (10YR 4/2 ish yellow / dark yellowish orange (10°	very moist, fine - dark grayish b	\ - -		10
	Annular Seal:bentonite pelletsFilter:silica filter sand					Silty Sand (SM)			15
ı	Standpipe: 2" OD PVC (SCH 40) Screen:			,	ish yellow / dark yellowish orange (10° olite wet, fine to coarse-grained brown (2.5Y 6/3), pale yellow (2.5Y 8	- mottled brown (10YR 8/1) sap	(20
	10 ft; pre-pack			2) moist, massive	nd (SC-SM) (2.5Y 8/1) and pinkish white (5YR 8/2 par and quartzite	Clayey Silty Sa - mottled white weathered felds	-		25
00001 ft	Sump:0.300000000000		<u> </u>		Bottom of borehole at 27.0 feet.		1	<u> Zdd</u>	
<u> </u>	Sump:0.300000000000			2) moist, massive	(2.5Y 8/1) and pinkish white (5YR 8/2 par and quartzite	- mottled white	-		25 30



SIMPLE GEOLOGY WITH WELL - ESEE DATABASE. GDT - 11/15/15 12:04 - NALTRCFP01\WSHAUGNE\$\DESKTOP\PLANTS PROJECTS\GEORGIA POWERIYATES\2015 PIEZOMETERS\YATES 2015 PZS. GPJ

LOG OF TEST BORING

BORING YGWA-21I PAGE 1 OF 2 ECS37967

	C	OMPANY				
SOI	UTHERI	N COMPANY SERVICES, INC.	PROJECT CCR	Piezometers	i	
		· · · · · · · · · · · · · · · · · · ·	OCATION Plan	t Yates		
DATE	START	ED <u>9/23/2015</u> COMPLETED <u>9/28/2015</u> SURF.	ELEV. 780.8	COOR	DINATES: N	:1,255,538.27 E: 2,076,768.14
		R Cascade EQUIPMENT				
		L. Yancey LOGGED BY W. Shaughnessy CH				
BORI	NG DEP	TH _77 ft GROUND WATER DEPTH: DURING	COMF	P. 24 ft.	DELAYE	D 24 ft. after 48 hrs.
NOTE	S	p of casing elev: 783.7				
DEPTH (ft)	GRAPHIC LOG	MATERIAL DESCRIPTION	ELEVATION	Weak Moderate HCL Strong GROUNDWATER	Comple protection 4-foot see	WELL DATA tion: /e aluminum cover with bollards; quare concrete pad
	/	Clayey Sand (SC)		5 2 0 0 0		
		- mottled strong brown (7.5YR 5/6) and yellowish red / I 5/6) dry, fine to coarse-grained, mica Silty Sand (SM) - reddish yellow (7.5YR 6/8) soil fine to coarse-grained	` _			_Surface Seal: concrete
5		Well-graded Sand (SW) - very pale brown (10YR 7/3) fine to coarse-grained, mic (residual rock)	ca, gravel			
		Poorly-graded Sand with Silt (SP-SM) - pale yellow (2.5Y 8/3) and pale yellow (2.5Y 8/2) dry				
10		- fine to medium-grained				
		- yellow (2.5Y 7/6)				
15		Silty Sand (SM)				
		- mottled yellow (2.5Y 7/6), white (2.5Y 8/1) and olive br saprolite weathered schist, feldsapr, quartz, fine to coars				
		Poorly-graded Sand with Silt (SP-SM)				
20		 mottled white (2.5Y 8/1) and yellowish brown / modera brown (10YR 5/4) dry, fine to medium-grained highly decomposed granitic gneiss interbedded with bio 				Annular Fill: cement-bentonite grout
		- mottled olive brown (2.5Y 4/3) and white (2.5Y 8/1)				
25		- yellowish brown / moderate yellowish brown (10YR 5/4))			
		- mottled light olive brown $$ (2.5Y 5/4) and $$ pale yellow (2 highly decomposed mica scist	.5Y 8/3) moist,			
30		- mottled white (2.5Y 8/1) and pale brown (10YR 6/3) didecomposed granitic gneiss, feldspar quartz, mica	ry, highly			-
	•••••	Well-graded Sand (SW)				
35		- mottled brown (10YR 4/3) and pale yellow (2.5Y 8/2) r coarse grained, mica, quartz	moist, fine to			
		- Granitic gneiss interbedded with biotite gneiss: mottled 7/2) and white (2.5Y 8/1) - Bedrock transition zone	light gray (2.5Y			
	1/ '/1	- Degreen transition zone		1 1 1 1	KY/I KY/I	



LOG OF TEST BORING

BORING YGWA-21I PAGE 2 OF 2 ECS37967

PROJECT CCR Piezometers SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING **LOCATION** Plant Yates HCL REACTION 3ROUNDWATER 3BSERVATIONS **WELL DATA** ELEVATION GRAPHIC LOG DEPTH (ft) MATERIAL DESCRIPTION Completion: protective aluminum cover with bollards: **Aoderate** 4-foot square concrete pad (Con't) SIMPLE GEOLOGY WITH WELL - ESEE DATABASE.GDT - 11/15/15 12:04 - \\ALTRCFP01\WSHAUGNESKTOP\PLANTS PROJECTS\GEORGIA POWER\YATES\2015 PIEZOMETERS\YATES 2015 PZS.GP. (2.5Y 8/1) and dark grayish brown (2.5Y 4/2) coarse grain, soft - white to hard, slightly to moderately weathered, medium to thick foliation, banded, moderately fractured (veritcal to sub-vertical) 45 - white (2.5Y 8/1), dark grayish brown (2.5Y 4/2) and pale yellow (2.5Y 7/3) coarse grain, soft to hard, not to highly weathered, medium to thick 50 foliation, banded, noderately fractured (near vertical), biotite gneiss Annular Fill: cement-bentonite grout 55 - gray (2.5Y 6/1), dark gray (2.5Y 4/1) and white (2.5Y 8/1) coarse grain, not to highly weathered, thin to medium foliation, moderately fractured (vertical to sub-vertical), pyrite, biotite, feldspar, quartz 60 - pale yellow / grayish yellow (5Ý 8/4) Annular Seal: bentonite pellets 65 Filter: silica filter sand - very dark gray (2.5Y 3/1) and white (2.5Y 8/1) coarse grain, not to slightly weathered, thin to medium foliation, moderately fractured (vertical to sub-vertical) Standpipe: 2" OD PVC (SCH 40) Screen: 10 ft; pre-pack 75 Sump:0.299999999999997 ft. Bottom of borehole at 77.0 feet. 80 85

AF	CADI	S for rists	S Compliancy rail and sets					Boring	g No.: <u>YGWC-24</u>	SA	
Borin	a Lo	a/W	ell Co	nstruction	Loa				Sheet: 1 of	2	
Project I	Name:	<u>Plant</u>	Yates				Date Started: <u>06/03/2020</u>	_Logger: <u>Clem</u>	ent Papafio		
Project I						Da	ate Completed: <u>06/04/2020</u>	_ Editor: <u>Gran</u>			
Project I	_ocation:	Newr	nan, GA				Weather C	onditions: <u>-</u>			
Depth (feet)	Sample Interval		Recovery (in.)	Photo Log	PID (ppm)	Graphic Log	Description		Construction Details	w	/ell
0							(0.0-13.0') No recovery. Hydrovac to borehole clearance. (13.0-16.0') Silty sand (SM); pale bromottled with white (10YR 8/1); fine gradium grained sand; very loose; mi weathered quartz vein present; sapro (16.0-19.0') Silty sand (SM); brownish 6/8); trace clay; loose; slight plasticity moist. (19.0-29.0') Silty sand (SM); brownish 6/8); fine grained to medium grained present; weathered quartz vein present; saprolitic; dry.	wn (10YR 8/3) and to cs present; litic; moist. Tyellow (10YR 7; mica present; Tyellow (10YR 8) and the control of the contro	Surface completion consists of a locking monument 3.00' above ground surface with a weep hole, vent hole in well casing, pea gravel between well monument and well casing, 4'x4'x4" concrete pad, and four concrete bollards. Portland Cement with 6% Bentonite. 2 inch diameter schedule 40 PVC riser.		
29	1 /										
Drilling (Co.:	Casc	ade Drilli	ng			Sampling Method <u>:Core B</u>	arrel		/ /	
Driller:	•			-ig							
Drilling I	/lethod:		-								
Drilling F		Wate					Water Level Finish (ft. btd	•			
Remark				= inch; bgs = below	ground	surface;	` —	Yes	No		
NA = not							Surface Elev.:762.00				
							North Coor: 1258907.98	3			
							East Coor: 2073924.8				

AR	CAD	IS for rate	S Comultancy rai and					Boring	g No.: <u>YGWC-248</u>	SA
1	ig Lo Name:	g/W Plant	ell Co	nstruction			Date Started: 06/03/2020 ate Completed: 06/04/2020	• • • • • • • • • • • • • • • • • • • •	ent Papafio	2
1 -								Conditions: -		
Depth (feet)	Sample Interval	Blow Counts	Recovery (in.)	Photo Log	PID (ppm)	Graphic Log	Description		Construction Details	Well
							(29.0-39.0') Silty sand (SM); yellowing 5/8); fine grained to medium grained medium dense; mica present; felds (34.0-36.0') Color change to dark by medium dense; dry. (36.0-39.0') Color change to white (oxidised iron present. (37.0-37.5') Weathered quartz vein. (39.0-49.0') Silty sand (SM); mottled (10YR 6/3) with yellow (10YR 8/8/); (41.0-49.0') Color change to yellowing 5/8); quartz seam present; moist.	d sand; loose to pars present. rown (10YR 3/3); 10YR 8/1); d pale brown mica present; wet. sh brown (10YR	Portland Cement with 6% Bentonite. 2 inch diameter schedule 40 PVC riser. Bentonite seal. Filter pack: #1A (12-40) sand. 2 inch diameter Sch. 40 PVC U-Pack dual wall 0.010-inch slotted screen	
53 54										
55 56 57 58 59 60 61 62 62							End of boring 54.0 ft bgs.			
Remark	s:									
	-	-			-	-				

Soil Boring Log	AR	CADI	S Design & for nature built ass	Consultancy al and ets					Boring No.	: YGWC-36A
Project Location Newman Georgia Date Completed: 09/22/2020	Soil E	3orin	a Lo	q					Sheet:	1 of 2
Project Location: Newman Georgia							ate Started: <u>09/21/2020</u>	Logg	ger: <u>Becky Steever</u>	· · · · ·
Depth Sample Blow Recovery (Rest) Infanced Courts (in.) Photo Log PiD Graphic (ppm) Grap	1					Date	•			
Toc Elev. Toc	Project L	ocation.	: <u>Newr</u>	ian, Geor	gıa	-	Weather C	onditio	ns: <u>Sunny & Warn</u>	1
1	Depth (feet)	Sample Interval	Blow Counts	Recovery (in.)	Photo Log		•			ヿ
Driller: Sampling Interval: Cont. Drilling Method: Sonic Water Level Start: 14 Drilling Fluid: None Water Level Finish: 12.1 Remarks: Converted to Well: Yes No Surface Elev.: 737.7 North Coor.: 1258547.74	3						tan to orange-red; fine grained to medium grained sand; no staining/odor; wet (8.0-10'); Sand; tan to medium grained sand; no staining/odor; some silt; moist (10-14'); Silty Sandy Clay; mottled gray; orange, tan, and brown; saprolite, relict structure; fine grains sand; dry (14-35); Silty Sand (SM); medium brown; fine to medium grained sand staining/odor; wet	ray; d ined	30% Bentonite Solids 3/8" Bentonite —	Stick-up with 4x4 Pad
Drilling Method: Sonic Water Level Start: 14 Drilling Fluid: None Water Level Finish: 12.1 Remarks: Converted to Well: X Yes No Surface Elev.: 737.7 North Coor.: 1258547.74	:I -	Co.:	Casc	ade						
Drilling Fluid: None Water Level Finish: 12.1 Remarks: Converted to Well:	•	Nathad:	Sonia	<u> </u>						
Remarks: Converted to Well: X Yes No Surface Elev.: 737.7 North Coor.: 1258547.74										
Surface Elev.:_737.7 North Coor.:_1258547.74			_					Yes	No	
North Coor.: 1258547.74						 				
						 		47.74		

AR	CAD	S Designate for natural hullt as	& Consultancy ral and						Boring No.: YGWC-36A
Soil E	Vame.	Yate:	<u>y</u>			D:	ate Started: 09/21/2020	I oager:	Sheet: 2 of 2 Becky Steever
Project I							Completed: <u>09/22/2020</u>		-
			<u>nan, Geor</u>	gia		-			Sunny & Warm
Depth (feet)	Sample Interval	Blow Counts	Recovery (in.)	Photo Log	PID (ppm)	Graphic Log	Description		Well Construction
36							(35-38'); Silty Sand (SM); brown t gray; fine to medium grained san	o d: no	
37				臺州東到			staining/odor; slightly moist	5	So. Products Filter Pack
38						77777	 		#1 (16-40) Top of
39							Same as above, some clay		Screen
40	-			1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1					
41									
42									
43				CANADA MANAGA MA			Pulverized rock sand; pale gray;		2" PVC Screen
44							some fine plagioclase gravel; fine	,	
45				Z m			grained sand		
46									
47				SEC.					
48				MAN CONTRACT					Bottom of Screen
49									
50									HOLEPLUG Bentonite —
51									Backfill
52 53	-			AND THE REAL PROPERTY.					
54						1787778			
55									
56									
57									
58									
59									
60									
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62_									
63_	1								
64	1								
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67_									
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69									
70_									
71 72									
8									
Remark	ъ.								
<u> </u>									

RECORD OF BOREHOLE YGWC-38/PZ-38

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 67.00 ft LOCATION: Newnan, GA

DRILL RIG: Sonic PS-150 DATE STARTED: 7/20/16 DATE COMPLETED: 7/23/16

NORTHING: 1,256,108.38 EASTING: 2,074,446.80 GS ELEVATION: 797.1 TOC ELEVATION: 799.69 ft

SHEET 1 of 2 DEPTH W.L.: 26.35 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/23/2016 TIME W.L.: N/A

		CON DEOCH C			TOC EL				9 π	
_	Z	SOIL PROFILE	T				AMPLE	:5		
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
5 —	- 795 	0.00 - 1.00 sitty SAND, goethite, loose, moist 1.00 - 8.00 tan to brown, mottled, loose, dry	SM		796.10	1		<u>5.00</u> 7.00		WELL CASING Interval: 0.0'-37.0' Material: Schedule 40 PV: Diameter: 2" Joint Type: Threaded WELL SCREEN Interval: 37.0'-47.0' Material: U-Pack Schedule PVC Diameter: 2"
10 —	790 785 	8.00 - 9.00 white to tan, plagioclase, loose, dry 9.00 - 27.00 tan to brown, mottled, micaceous, loose, dry			789.10 8.00 788.10 9.00	2		10.00	Portland _ Type 1	Slot Size: 0.010' End Cap: Schedule 40 PN FILTER PACK Interval: 35.0'-48.0' Type: #1 Type Sand FILTER PACK SEAL Interval: 30.0'-35.0' Type: Bentonite Pellets ar Chips ANNULUS SEAL Interval: 0.0'-30.0' Type: Portland Type 1 WELL COMPLETION Pad: 4'X4''A4"
20 —	780 775 				770.10	3		<u>10.00</u> 10.00	Portland Type 1	Protective Casing: Alumin DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
30 —	— 770 - - - - - 765 -	27.00 - 34.00 brown to tan, relict structure, moist to wet, loose (saprolite)			27.00	4		<u>7.00</u> 10.00	1/2" Bentonite – Pellets	
35 —	- - - 760 -	34.00 - 37.00 transitionally weathered rock - weathered GNEISS to high grade SCHIST, garnet, muscovite, biotite, recrytallization, fractured, friable 37.00 - 39.00 bedrock - muscovite/biotite GNEISS, homblende, fresh	PWR	2000 00 00 00 00 00 00 00 00 00 00 00 00	34.00					
40 —	- - - 755 - -	39.00 - 40.00 muscovite/biotite GNEISS, hornblende, iron staining 40.00 - 56.00 muscovite/biotite GNEISS, hornblende, fresh			758.10 39.00 757.10 40.00	5		<u>7.00</u> 10.00	0.010" Slotted - Screen #1 Type _ Sand - Sand	
50 —	- 750 - - -	Log continued on next page				6		<u>6.00</u> 10.00	Sump –	
DRII	LLING	LE: 1 in = 6.5 ft COMPANY: Cascade Drilling Tom Ardito		CHEC	SPECT KED B\ : 9/29/1	r: Ra			/ nan, PG	Golder



RECORD OF BOREHOLE YGWC-38/PZ-38 DRILL RIG: Sonic PS-150 DATE STARTED: 7/20/16 DATE COMPLETED: 7/23/16 NORTHING: 1,256,108.38 EASTING: 2,074,446.80 GS EL EVATION: 707.1

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 67.00 ft LOCATION: Newnan, GA

GS ELEVATION: 797.1 TOC ELEVATION: 799.69 ft

SHEET 2 of 2 DEPTH W.L.: 26.35 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/23/2016 TIME W.L.: N/A

				-	TOC ELE				9 ft T	<u> </u>
	z	SOIL PROFILE				S	AMPLE			
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	ELEV. DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
50 —	- 745	40.00 - 56.00 muscovite/biotite GNEISS, homblende, fresh (Continued)			(11)	o o			Bentonite Pellet Backfill	WELL CASING Interval: 0.0'-37.0' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded
55 —	- -	56.00 - 58.00			741.10	6		6.00 10.00	- - -	WELL SCREEN Interval: 37.0'-47.0' Material: U-Pack Schedule PVC Diameter: 2" Slot Size: 0.010'
60 —	740 	muscovite/biotite GNEISS, hornblende, iron staining 58.00 - 67.00 muscovite/biotite GNEISS, hornblende, fresh			739.10				- - - -	End Cap: Schedule 40 PVC FILTER PACK Interval: 35.0'-48.0' Type: #1 Type Sand
	- 735 -					7		7.00 10.00	- - -	FILTER PACK SEAL Interval: 30.0'-35.0' Type: Bentonite Pellets and Chips ANNULUS SEAL
65 —	- - - - 730				730.10				- - -	Interval: 0.0'-30.0' Type: Portland Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Aluminu
70 —	- - -	Boring completed at 67.00 ft								DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
-	- 725 -								- - -	
75 —	- - - - 720								- - -	
80 —	- -								- -	
-	- 715 -								- - -	
85 —	- - -								- - -	
90 —	 710 - -								- - -	
90 -	- 705 -								- - -	
95 —	- - - - 700								- - - -	
100 —	- - 								<u>-</u>	
DRII	LLING	LE: 1 in = 6.5 ft COMPANY: Cascade Drilling Tom Ardito	(CHEC	SPECT KED BY 9/29/1	r: Ra			y nan, PG	Golder



RECORD OF BOREHOLE YGWA-39/PZ-39

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 66.00 ft LOCATION: Newnan, GA

DRILL RIG: Sonic PS-150 DATE STARTED: 7/6/16 DATE COMPLETED: 7/7/16

NORTHING: 1,255,717.13 EASTING: 2,073,865.58 GS ELEVATION: 815.6 TOC ELEVATION: 818.19 ft

SHEET 1 of 2 DEPTH W.L.: 19.15 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/8/2016 TIME W.L.: N/A

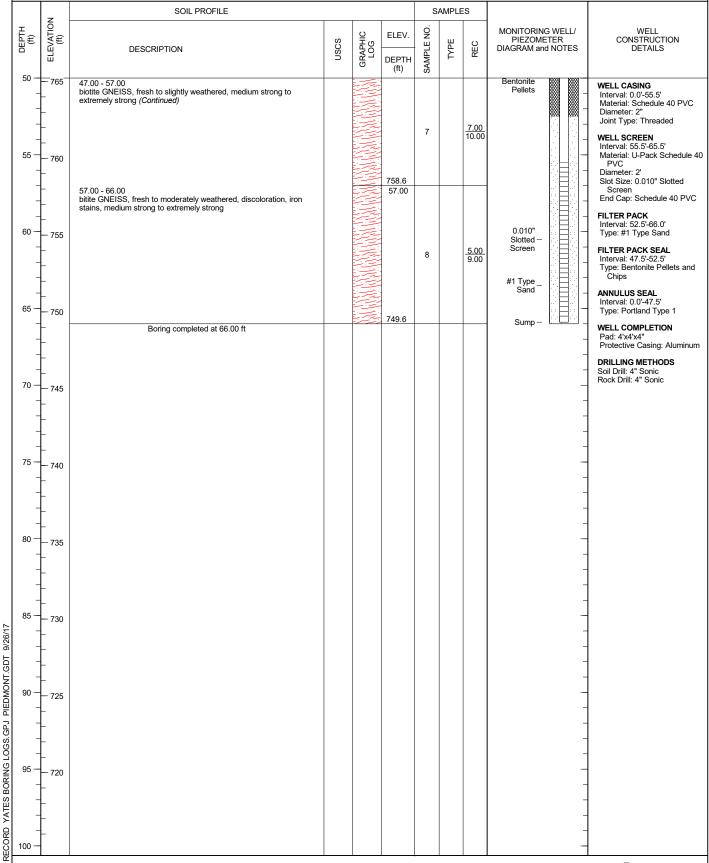
	z	SOIL PROFILE				S	AMPLE	S		
(#)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	ELEV. DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
0	815	0.00 - 0.40	TOPSOI		0.40	S			9000 P5000 P	WELL CASING
5 —	- - - - - 810	topsoil 0.40 - 7.00 poorly sorted SAND with SILT, trace gravel, tan, mica fragments, dry, firm	SP-SM		0.40	1		7.00 7.00	Portland	Interval: 0.0'-55.5' Material: Schedule 40 PV(Diameter: 2" Joint Type: Threaded WELL SCREEN Interval: 55.5'-65.5' Material: U-Pack Schedule
-	- - -	7.00 - 17.00 some silt, tan, dry, firm			808.6					PVC Diameter: 2' Slot Size: 0.010" Slotted Screen End Cap: Schedule 40 PV FILTER PACK Interval: 52.5'-66.0'
10 -	805 					2		10.00 10.00		Type: #1 Type Sand FILTER PACK SEAL Interval: 47.5-52.5' Type: Bentonite Pellets an Chips ANNULUS SEAL
15 — - -	— 800 - -	17.00 - 19.00			798.6					Interval: 0.0'-47.5' Type: Portland Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Alumin
20 —	- - 795	silty SAND, non-plastic fines, orange tan, micaceous, cohesive, firm 19.00 - 21.00 poorly graded SAND with SILT, non-plastic fines, moist, firm	SM SP-SM		796.6 19.00 794.6	3		4.00 4.00		DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
-	- - - -	21.00 - 24.00 silty SAND, 15-20% fines, orange tan with iron staining, wet (saprolite) 24.00 - 29.00	SM		791.6 24.00	4		6.00 6.00	Portland	
25 - -	— 790 – –	SAND to silty SAND, some fines, mica, orange tan to tan, severely weathered fragements, dry to moist (saprolite)	SP-SM							
30 —	- - 785 -	29.00 - 32.00 transitionally weathered rock - sand, some gravel, tan, rock seams, iron staining			783.6	5		3.80 6.00		
35 —	- - - - 780	32.00 - 33.00 pulverized GNEISS, tan 33.00 - 37.00 bedrock - biotite GNEISS, fresh to medium weathered, medium strong to extremely strong, iron stains and deposits	GNEISS	D 4 0 0	32.00 782.6 33.00	6		4.00 4.00	00001 00001	
-	- - -	37.00 - 39.00 biotite GNEISS, severely weathered, iron staining and deposits			778.6 37.00					
40 -	- 775 - -	39.00 - 43.00 biotite GNEISS, severely weathered, sand layers noted iron staining and deposits			39.00					
- - 45 —	- - - 770	43.00 - 47.00 biotite GNEISS, severely weathered, iron staining and deposits			772.6 43.00					
50 —	- - -	47.00 - 57.00 biotite GNEISS, fresh to slightly weathered, medium strong to extremely strong Log continued on next page			768.6 47.00	7		7.00 10.00	1/2"	
		LE: 1 in = 6.5 ft COMPANY: Cascade Drilling			SPECTO KED BY			-	/issman	Golder



RECORD OF BOREHOLE YGWA-39/PZ-39

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 66.00 ft LOCATION: Newnan, GA DRILL RIG: Sonic PS-150 DATE STARTED: 7/6/16 DATE COMPLETED: 7/7/16

NORTHING: 1,255,791.95 EASTING: 2,073,431.34 GS ELEVATION: 815.6 TOC ELEVATION: 818.19 ft SHEET 2 of 2
DEPTH W.L.: 19.15 ft (bgs)
ELEVATION W.L.: (amsl)
DATE W.L.: 7/8/2016
TIME W.L.: N/A



LOG SCALE: 1 in = 6.5 ft

DRILLING COMPANY: Cascade Drilling

DRILLER: Tom Ardito

GA INSPECTOR: Courtney Vissman CHECKED BY: Rachel Kirkman, PG



RECORD OF BOREHOLE YGWA-40/PZ-40

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 46.00 ft LOCATION: Newnan, GA

DRILL RIG: Sonic PS-150 DATE STARTED: 7/6/16 DATE COMPLETED: 7/7/16

NORTHING: 1,255,791.95 EASTING: 2,073,431.34 GS ELEVATION: 813.5 TOC ELEVATION: 815.73 ft

SHEET 1 of 1 DEPTH W.L.: 23.1 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/8/2016 TIME W.L.: N/A

					TOC ELE				3 π
_	Z -	SOIL PROFILE					AMPLE	S	
Œ.	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	DEPTH	SAMPLE NO	TYPE	REC	MONITORING WELL/ PIEZOMETER CONSTRUCTION DIAGRAM and NOTES DETAILS
o <u> </u>	- -	0.00 - 2.00 sandy SILT, fine to medium sand, reddish brown, low plastic	SM		(ft) 811.5	Ø			WELL CASING Interval: 0,0°-35.5' Material: Schedule 40 F Diameter: 2"
-	- 810 	2.00 - 6.00 fine to medium sand, light orange brown, micaceous, dry, loose			2.00	1		6.00 6.00	Diameter: 2" Joint Type: Threaded WELL SCREEN
5 —	-	6.00 - 16.00 fine to coarse sand, low plastic silt, some gravel, brown grey to			807.5				Interval: 35.5'-45.5' Material: U-Pack Sched PVC Diameter: 2' Slot Size: 0.010" Slotte
-	- 805 -	grey, corasening downward, relict laminations, more dense with depth, saprolitic, dry							Screen End Cap: Schedule 40 FILTER PACK Interval: 32.5'-46.0'
10 —	-					2		12.00 10.00	Type: #1 Type Sand FILTER PACK SEAL Interval: 27.0-32.5
-	800 								Portland _ Type 1 Type: Bentonite Pellets Chips ANNULUS SEAL Interval: 0.0'-27.0'
5 —		16.00 - 17.00 coarse, competent		D	797.5 16.00 796.5			3.00	Type: Portland Type 1 WELL COMPLETION Pad: 4'x4'x4"
-	- 795 -	17.00 - 19.00 transitionally weathered rock - highly weathered GNEISS, red, white, dark brown 19.00 - 36.00	PWR	AAAA	794.5 19.00	3		3.00	Protective Casing: Alur DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
20 —	- - -	highly weathered biotite GNEISS, oxidized staining		4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4 4				7.00	
- - -	— 790 –					4		7.00 7.00	
25 —	- - -								
-	 785 			20 20 20 20 20 20 20 20 20 20 20 20 20 2					Bentonite Pellets and —
30 —	- - -					5		10.00 10.00	Chips
- - 35 -	— 780 –								
-	- -	36.00 - 46.00 bedrock - biotite GNEISS, some weathering, trace pyrite		A A A A A A A A A A A A A A A A A A A	777.5				0.010"
10 —	— 775 - -								Slotted - Screen
-	-		GNEIS			6		9.00 10.00	#1 Type
15 —	— 770 — —								
-	- - - 765	Boring completed at 46.00 ft		E	767.5				Sump - [:
_ 50 —	_								
DRII	LLING	LE: 1 in = 6.5 ft COMPANY: Cascade Drilling Adam M.	(CHEC	SPECTO KED BY 9/29/1	′: Ra			es nan, PG Golden Associa



RECORD OF BOREHOLE YGWC-41/PZ-41

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 64.50 ft LOCATION: Newnan, GA

DRILL RIG: Sonic PS-150 DATE STARTED: 7/7/16 DATE COMPLETED: 7/8/16

NORTHING: 1,256,510.62 EASTING: 2,073,274.41 GS ELEVATION: 801.1 TOC ELEVATION: 803.92 ft

SHEET 1 of 2 DEPTH W.L.: 22.1 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/8/2016 TIME W.L.: 07:30

		SOIL PROFILE			TOC EL	1	SAMPLE		2 11	
_	NO	SOIL PROFILE	1				AWIFLE			
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC	DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
0 -	- 800	0.00 - 2.00 sandy SILT, dark reddish brown, severely weathered gneiss, dry	ML		799.10			4.00		WELL CASING Interval: 0.0'-54.0' Material: Schedule 40 PVC
-	-	2.00 - 4.00 silty SAND, light brown, dry, non-cohesive	SM		2.00 797.10	1		4.00		Diameter: 2" Joint Type: Threaded WELL SCREEN
5 —	- 795 	4.00 - 14.00 light brown, quartz sand at ~5', dry			4.00					Interval: 54.0'-64.0' Material: U-Pack Schedule PVC Diameter: 2' Slot Size: 0.010" End Cap: Schedule 40 PV
10 —	- - - - 790					2		10.00 10.00		FILTER PACK Interval: 51.0'-64.5' Type: #1 Type Sand FILTER PACK SEAL Interval: 45.5'-51.0'
15	- - -	14.00 - 24.00 light brown, dry			787.10 14.00					Type: Bentonite Pellets an Chips ANNULUS SEAL Interval: 0.0'-45.5' Type: Portland Type 1
2 -	785 					3		<u>9.00</u> 10.00		WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Alumin DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
20 —	- - 780 - -				5 777 40			.5.00	Portland Type 1	
25 —	- - - 775 - -	24.00 - 29.00 trace gravel, sand coarsening, light brown, dry			777.10 24.00	4		<u>5.00</u> 5.00		
30 —	- - - 770 - -	29.00 - 38.00 transitionally weathered rock - highly weathered biotite/muscovite GNEISS	PWR		DD 0	5		<u>5.00</u> 5.00	Portland Type 1	
35 —	- - 765 - -	38.00 - 39.00		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	763.10	6		<u>5.00</u> 5.00		
40	- 760 - -	more competent 39.00 - 44.00 bedrock - biotite GNEISS, red to orange staining	GNEIS:		39.00	7		<u>4.00</u> 5.00		
45	- - 755 - - -	44.00 - 54.00 transitionally weathered rock - highly weathered biotite GNEISS, red oxide staining	PWR	DO AD DO	44.00 PDA 44	8		<u>5.00</u> 10.00	Bentonite Pellets and — Chips	
50 -		Log continued on next page						<u> </u>		
DRIL		LE: 1 in = 6.5 ft COMPANY: Cascade Drilling		CHE	NSPECT CKED B' E: 9/29/	Y: R			es nan, PG	Golder



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 64.50 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE YGWC-41/PZ-41

DRILL RIG: Sonic PS-150
DATE STARTED: 7/7/16
DATE COMPLETED: 7/8/16

NORTHING: 1,256,510.62
EASTING: 2,073,274.41
GS FI FVATION: 801 1 GS ELEVATION: 801.1 TOC ELEVATION: 803.92 ft

SHEET 2 of 2 DEPTH W.L.: 22.1 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/8/2016 TIME W.L.: 07:30

		SOIL PROFILE			OC ELE		AMPLE		Tt.	
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	ELEV. DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
50 — - - -	— 750 —	44.00 - 54.00 transitionally weathered rock - highly weathered biotite GNEISS, red oxide staining (Continued)	PWR		747.40	8		<u>5.00</u> 10.00		WELL CASING Interval: 0.0'-54.0' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded
55 — - -	- - 745 - -	54.00 - 64.00 bedrock - biotite GNEISS, vertical oxide staining along fractures			747.10 54.00			0.00	0.010" - Soreen	WELL SCREEN Interval: 54.0'-64.0' Material: U-Pack Schedule 4 PVC Diameter: 2' Slot Size: 0.010" End Cap: Schedule 40 PVC FILTER PACK
60 —	- - 740 -		GNEISS		727 10	9		8.00 10.00	#1 Type	Interval: 51.0'-64.5' Type: #1 Type Sand FILTER PACK SEAL Interval: 45.5'-51.0' Type: Bentonite Pellets and Chips ANNULUS SEAL
65 -	- - 735 -	Boring completed at 64.50 ft			737.10 64.00				Sump –	Interval: 0.0'-45.5' Type: Portland Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Aluminum DRILLING METHODS
70 — - - -	- - 730 - -								- - - - -	Soil Drill: 4" Sonic Rock Drill: 4" Sonic
75 	- - 725 - -								- - - -	
80 — - - -	- 720 - -								- - - -	
85 — - - -	- 715 - -								- - - -	
90 —	- 710 - -								- - - -	
95 — — —	- 705 - - -								- - - -	
DRI		LE: 1 in = 6.5 ft COMPANY: Cascade Drilling	(CHEC	SPECT KED BY 9/29/1	r: Ra			es nan, PG	Golder



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 57.00 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE

DRILL RIG: Sonic PS-150

DATE STARTED: 7/7/16

DATE COMPLETED: 7/8/16

DATE COMPLETED: 7/8/16

PGWC-42/ PZ-42

NORTHING: 1,256,882.87

EASTING: 2,073,326.52

GS EL EVALUON: 795.1

GS ELEVATION: 795.1 TOC ELEVATION: 797.86 ft SHEET 1 of 2

DEPTH W.L.: 26.2 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/8/2016 TIME W.L.: N/A

I		SOIL PROFILE			100		AMPLE		7.86 ft	
_	Z O	SOIL PROFILE	1				AMPLE	-8		
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
0 -	 795 - -	0.00 - 3.00 silty SAND, plastic fines, orange brown, micaceous, firm, decreasing moisture with depth	SM		792.10					WELL CASING Interval: 0.0'-47' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded
5 —	- - - 790	3.00 - 7.00 poorly graded SAND, some silt, tan to black, white to red, micaceous, dry	SP		3.00	1		7.00 7.00		WELL SCREEN Interval: 46.6'-56.6' Material: U-Pack Schedule PVC
	- - -	7.00 - 10.50 some silt, tan to black, white to red, dry			788.10					Diameter: 2" Slot Size: 0.010" End Cap: Schedule 40 PV FILTER PACK Interval: 46.0'-57.0'
10 -	785 	10.50 - 18.00 some silt, tan to black, white to red, increasing biotite, dry			784.60 10.50	2		10.00 10.00	Portland _ Type 1	Type: #1 Sand FILTER PACK SEAL Interval: 37.5-46.0' Type: Bentonite Pellets and Chips
15 -	- 780 									ANNULUS SEAL Interval: 0.0'-37.5' Type: Portlant Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Aluminu
20 —	- - 775	18.00 - 19.00 silty SAND, red seam 19.00 - 27.00 poorlyly graded SAND, some silt, tan to black, white to red, plagioclase, dry	SM		777.10 18.00 776.10 19.00					DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
- - -	- - -		SP			3		10.00 10.00	Portland	
25	770 	27.00 - 30.00 no recovery			768.10					
30 -	- 765 - -	30.00 - 34.00 bedrock - biotite GNEISS, fresh to moderately weathered, medium strong to extremely strong, foliated	GNEIS		765.10 30.00	4		<u>5.00</u> 10.00		
35 —	- - - 760 -	34.00 - 37.00 biotite GNEISS, increased biotite, fresh to moderately weathered, medium strong to extremely strong, foliated			761.10 34.00					
	- - -	37.00 - 40.00 biotite GNEISS, fresh to moderately weathered, medium strong to extremely strong, fractured, foliated			758.10 37.00 755.10				Bentonite Pellets and – Chips	
40	755 	40.00 - 50.00 biotite GNEISS, fresh to moderately weathered, medium strong to extremely strong, foliated			40.00	5		<u>8.00</u> 10.00	- - -	
45 —	750 									
50 —	-	Log continued on next page			745.10	6		6.00 10.00	#1 Type	
DRIL	LING	LE: 1 in = 6.5 ft COMPANY: Cascade Drilling Tom Ardito		CHEC		r: Ra			/issman nan, PG	Golder



RECORD OF BOREHOLE YGWC-42/ PZ-42

DRILL RIG: Sonic PS-150 DATE STARTED: 7/7/16 DATE COMPLETED: 7/8/16

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 57.00 ft

LOCATION: Newnan, GA

NORTHING: 1,256,882.87 EASTING: 2,073,326.52 GS ELEVATION: 795.1 TOC ELEVATION: 797.86 ft SHEET 2 of 2

DEPTH W.L.: 26.2 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/8/2016 TIME W.L.: N/A

SOIL PROFILE SAMPLES ELEVATION (ft) DEPTH (ft) MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES WELL CONSTRUCTION Š GRAPHIC LOG ELEV. nscs TYPE SAMPLE REC DESCRIPTION **DETAILS** DEPTH (ft) 50 50.00 - 56.00 50.00 WELL CASING biotite GNEISS, fresh to moderately weathered, iron staining, medium strong to extremely strong, foliated Interval: 0.0'-47' Material: Schedule 40 PVC 0.010" Slotted Diameter: 2" Screen Joint Type: Threaded 6.00 10.00 6 WELL SCREEN Interval: 46.6'-56.6' Material: U-Pack Schedule 40 - 740 739.10 56.00 - 57.00 biotite GNEISS, fresh to moderately weathered, iron staining, medium strong to extremely strong, foliated, stained, fractured Slot Size: 0.010" End Cap: Schedule 40 PVC Boring completed at 57.00 ft FILTER PACK Interval: 46.0'-57.0' Type: #1 Sand 60 -- 735 FILTER PACK SEAL Interval: 37.5'-46.0' Type: Bentonite Pellets and Chips ANNULUS SEAL Interval: 0.0'-37.5' Type: Portlant Type 1 65 - 730 WELL COMPLETION Protective Casing: Aluminum DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic 70 -- 725 75 - 720 80 - 715 85 -- 710 9/26/17 PIEDMONT.GDT - 705 YATES BORING LOGS.GPJ 95 -- 700 100

LOG SCALE: 1 in = 6.5 ft

DRILLING COMPANY: Cascade Drilling

DRILLER: Tom Ardito

GA INSPECTOR: Courtney Vissman CHECKED BY: Rachel Kirkman, PG



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 77.00 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE

DRILL RIG: Sonic PS-150

DATE STARTED: 7/8/16

DATE COMPLETED: 7/9/16

PGS FI EVALUATION: 742.3 GS ELEVATION: 742.3 TOC ELEVATION: 744.96 ft

SHEET 1 of 2

DEPTH W.L.: 30.5 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/9/2016 TIME W.L.: N/A

		SOIL PROFILE			100		AMPLE		44.96 ft	
ᅟ	NO O	SOIL FROFILE					- IVIF LE	_5	MONITORING WELL/	WELL
(f)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	DEPTH (ft)	SAMPLE NO.	TYPE	REC	PIEZOMETER DIAGRAM and NOTES	CONSTRUCTION DETAILS
0 -	745 	0.00 - 5.00 SAND to silty SAND, 10-15% fines, red to tan to brown, micaceous, dry to moist			(1.7)	05				WELL CASING Interval: 0.0'-66.5' Material: Schedule 40 PV Diameter: 2"
5-	- - - 740 - -	5.00 - 6.00 10-15% fines, red to tan to brown, plagioclase, micaceous, dry to moist 6.00 - 14.00 10-15% fines, red to tan to brown, micaceous, dry to moist	SM		737.30 5.00 736.30 6.00			<u>8.00</u> 8.00	Portland Type 1	Joint Type: Threaded WELL SCREEN Interval: 66.5'-75.5' Material: U-Pack Schedu PVC Diameter: 2" Slot Size: 0.010" End Cap: Schedule 40 P' FILTER PACK Interval: 63.7'-77.0'
10 -	- 735 - -							<u>8.00</u> 9.00		Type: #1 Sand FILTER PACK SEAL Interval: 58.5'-63.7' Type: Bentonite Pellets a Chips
- 15 — -	- 730 	14.00 - 28.00 transitionally weathered rock - feldspathic GNEISS, moderately to highly weathered, medium strong to extremely strong, discolored, iron stains and deposits			728.30					ANNULUS SEAL Interval: 0.0'-58.5' Type: Portlant Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Alumin
20 —	- - 725 -		PWR	D				3.50 4.00		DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
25 — -	- - - - 720 -			0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	7			3.00 8.00		
30 —	- - 715 -	28.00 - 33.00 bedrock - feldspathic GNEISS, fresh to slighly weathered, medium strong to extremely strong, discolored, iron stains and deposits	GNEIS:		28.00				Portland Type 1	
35 —	- - - - 710	33.00 - 38.00 feldspathic GNEISS, fresh to lightly weathered, medium strong to extremely strong, fabric, discolored, some iron stains and deposits			709.30 33.00			<u>10.00</u> 10.00		
40 — -	- - - - 705 -	38.00 - 77.00 feldspathic GNEISS, fresh, olive colored mineral, some garnet, quartz, biotite			704.30 38.00					
45 — -	- - - 700 -							10.00 10.00		
- - 50 -	-	Log continued on next page			; F1 F2 F1			10.00		
DRII	LING	LE: 1 in = 6.5 ft COMPANY: Cascade Drilling Tom Ardito		CHEC		r: Ra			vissman nan, PG	Golder



RECORD OF BOREHOLE YGWC-43/ PZ-43

DRILL RIG: Sonic PS-150 DATE STARTED: 7/8/16 DATE COMPLETED: 7/9/16

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 77.00 ft

LOCATION: Newnan, GA

NORTHING: 1,257,547.41 EASTING: 2,073,199.65 GS ELEVATION: 742.3 TOC ELEVATION: 744.96 ft SHEET 2 of 2

DEPTH W.L.: 30.5 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/9/2016 TIME W.L.: N/A

SOIL PROFILE SAMPLES ELEVATION (ft) DEPTH (ft) MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES WELL CONSTRUCTION Š GRAPHIC LOG ELEV. nscs TYPE SAMPLE REC DESCRIPTION **DETAILS** DEPTH (ft) 50 10.00 - 695 38.00 - 77.00 WELL CASING feldspathic GNEISS, fresh, olive colored mineral, some garnet, Interval: 0.0'-66.5' Material: Schedule 40 PVC quartz, biotite (Continued) Diameter: 2" Joint Type: Threaded WELL SCREEN 10.00 10.00 Interval: 66.5'-75.5' Material: U-Pack Schedule 40 55 - 690 Slot Size: 0.010" End Cap: Schedule 40 PVC FILTER PACK Interval: 63.7'-77.0' Type: #1 Sand 60 685 Bentonite Pellets and FILTER PACK SEAL Interval: 58.5'-63.7'
Type: Bentonite Pellets and Chips Chips ANNULUS SEAL Interval: 0.0'-58.5' Type: Portlant Type 1 10.00 10.00 680 WELL COMPLETION Protective Casing: Aluminum 0.010" DRILLING METHODS Slotted Screen Soil Drill: 4" Sonic Rock Drill: 4" Sonic 70 -675 #1 Type Sand 7.00 10.00 75 - 670 Sump 665.30 Boring completed at 77.00 ft 80 665 85 - 660 9/26/17 PIEDMONT.GDT 655 YATES BORING LOGS.GPJ 95 650 100

LOG SCALE: 1 in = 6.5 ft

DRILLING COMPANY: Cascade Drilling

DRILLER: Tom Ardito

GA INSPECTOR: Courtney Vissman CHECKED BY: Rachel Kirkman, PG



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 75.90 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE YGWC-49/ PZ-49

DRILL RIG: Sonic PS-150 DATE STARTED: 7/12/16 DATE COMPLETED: 7/13/16

NORTHING: 1,259,375.23 EASTING: 2,074,337.51 GS ELEVATION: 780.1 TOC ELEVATION: 782.73 ft SHEET 1 of 2

DEPTH W.L.: 26.95 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/13/2016 TIME W.L.: 15:26

		SOIL PROFILE			100		AMPLE		32.73 ft	
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	ELEV. DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
0 — — — — — — — — — — — — — — — — — — —	- - - - - 775	0.00 - 10.00 No recovery; Hydrovac			(it)	0			Portland _ Type I	WELL CASING Interval: 0.0'-65.4' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded WELL SCREEN Interval: 65.4-75.4 Material: U-Pack Schedule 40 PVC Diameter: 2" Slot Size: 0.010"
10 — 	- - 770 - -	10.00 - 16.00 sitly SAND, fine sand, greyish brown to reddish brown, non-cohesive, dry, loose	SM		770.0	1		6.00 6.00	Portland _ Type I = -	End Cap: Schedule 40 PVC FILTER PACK Interval: 63.3'-75.9' Type: #1 Sand FILTER PACK SEAL Interval: 58.7'-62.2' Type: Bentonite Pellets and Chips ANNULUS SEAL
- 15 — - -	765 765 	16.00 - 26.00 No recovery due to soil washing out of core barrel			764.0 16.00					Interval: 0.0'-58.7' Type: N/A WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Aluminum
20 — -	- - 760 -					2		<u>0.00</u> 10.00		DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
25 —	755 	26.00 - 36.00 sity SAND, dark brown to grayish brown, relict laminations, fully			754.0 26.00				Portland Type I	
- - -	- 745	weathered schist, dry, loose (saprolite)	SM			3		<u>10.00</u> 10.00		
35 — — — — 40 —	745 740	36.00 - 46.00 softer zone			774.0 36.00					
- - - 45	- - - - - 735				734.0	4		9.00		
- - - 50 —	_ _ _ _ 730	46.00 - 54.00 some silt and some gravel increasing with depth, mottled dark brown to orange, relict laminations, dry, loose (saprolite) Log continued on next page	SP		46.00	5		8.00 8.00		

LOG SCALE: 1 in = 6.5 ft

DRILLING COMPANY: Cascade Drilling

DRILLER: Dale

BOREHOLE

GA INSPECTOR: Ben Hodges CHECKED BY: Rachel Kirkman, PG



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 75.90 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE

DRILL RIG: Sonic PS-150

DATE STARTED: 7/12/16

DATE COMPLETED: 7/13/16

PART OF BOREHOLE

NORTHING: 1,259,375.23

EASTING: 2,074,337.51

CS ELEVATION: 780.1

GS ELEVATION: 780.1 TOC ELEVATION: 782.73 ft SHEET 2 of 2

DEPTH W.L.: 26.95 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/13/2016 TIME W.L.: 15:26

so br (C	SOIL PROFILE DESCRIPTION 6.00 - 54.00 ome silt and some gravel increasing with depth, mottled dark rown to orange, relict laminations, dry, loose (saprolite) Continued)	nscs	GRAPHIC	ELEV.	SAMPLE NO.	TY PE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
50 46 so br (C - 54 tra	6.00 - 54.00 ome silt and some gravel increasing with depth, mottled dark rown to orange, relict laminations, dry, loose (saprolite)		GRAPHIC	DEPTH	MPLE NO.	IYPE	EC	PIEZOMETER	CONSTRUCTION
40 so brin (C	ome silt and some gravel increasing with depth, mottled dark rown to orange, relict laminations, dry, loose (saprolite)	0.0	2000		S.		Ŀ	DIAGRAM and NOTES	2220
55 - 725 tra		SP		726.0	5		8.00 8.00		WELL CASING Interval: 0.0'-65.4' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded
60 720	4.00 - 65.00 ansitionally weathered rock -biotite mica SCHIST, deeply stained, ghly weathered, friable	PWR	D 4 D 4 D 4 D 4 D 4 D 4 D 4 D 4 D 4 D 4		6		<u>0.50</u> 11.00	Bentonite Pellets and — Chips	WELL SCREEN Interval: 65.4-75.4 Material: U-Pack Schedule PVC Diameter: 2" Slot Size: 0.010" End Cap: Schedule 40 PV FILTER PACK Interval: 63.3'-75.9' Type: #1 Sand FILTER PACK SEAL Interval: 58.7'-62.2' Type: Bentonite Pellets an Chips
	5.00 - 76.00 ighly fractured		0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	1				#1 Sand	ANNULUS SEAL Interval: 0.0'-58.7' Type: N/A WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Alumin DRILLING METHODS Soil Drill: 4" Sonic
70 — 710 					7		<u>2.00</u> 11.00	0.010" - Slotted - Screen	Rock Drill: 4" Sonic
75 — 705			D					<u> </u> <u> </u> <u> </u> <u> </u> <u> </u>	
80 — 700 80 — 700 85 — 695 90 — 690 95 — 685	Boring completed at 75.90 ft			76.00				Sump - \	
00 - 680 LOG SCALE:	Air 0.56		GA INS					- - -	

LOG SCALE: 1 in = 6.5 ft

DRILLING COMPANY: Cascade Drilling

DRILLER: Dale

GA INSPECTOR: Ben Hodges CHECKED BY: Rachel Kirkman, PG





BORING YGWA-6S/PZ-6S

PAGE 1 OF 1 ECS37976

PROJECT Plant Yates Hydro-Geological Study SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING LOCATION Newnan, GA **COORDINATES:** N: 1,260,484.87 E: 2,074,786.49 **DATE STARTED** 4/22/2014 **COMPLETED** 5/19/2014 **SURF. ELEV.** 779.8 EQUIPMENT PS-150 METHOD Rotosonic CONTRACTOR Cascade Drilling ANGLE BEARING DRILLED BY D. Wilcox LOGGED BY B. Smelser CHECKED BY BORING DEPTH 37.2 ft. GROUND WATER DEPTH: DURING COMP. 14.77 ft. DELAYED **NOTES** Top of Casing Elevation = 782.47 STRATA DESCRIPTION **WELL DATA** Œ GRAPHIC DEPTH Surface: protective aluminum cover with bollards; 4-foot square concrete pad ELEV. (DEPTH) FLEV Poorly-graded Sand with Silt (SP-SM) Surface Seal: concrete - dark brown, damp, loose, no plasticity, lower fine to upper fine grain, 777.8 cohesive; trace organics; trace gravel sized rock fragments; angular GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 17:32 - NALTRCFS02\X2DBSMEL\$\GINT\PLANT YATES HYDRO-GEOLOGICAL STUDY.GPJ (2.0)to subangular grains 776.8 Silty Sand (SM) - rusty red, dry, medium dense, no plasticity, lower fine to upper fine grain, cohesive; slight increase in clay content; micaceous - SM: orangish brown to tan grading to light gray/white, dry, medium dense to loose, no plasticity, upper fine to lower medium with trace upper medium grain, saprolite, noncohesive; completely weathered to residual soil; trace gravel sized rock fragments; rock fragments range from brittle/friable to hard/more competent; micaceous; muscovite, chlorite, biotite, quartz, plagioclase identifiable Annular Fill: 90/10 Portland Cement/Bentonite Powder - SM: medium brown/orangish brown to medium gray with greenish gray mottling, damp, medium dense to loose, no plasticity, upper fine to upper medium grain, saprolite, noncohesive; completely weathered to residual soil; zones of remnant banding visible; brittle to hard rock fragments included; chlorite, biotite, muscovite, quartz, plagioclase identifiable: harder/more competent rock fragments tend to be felsic 다니 (quartz, plagioclase) and brittle/friable rock fragments tend to be mafic (biotite, chlorite) 759.0 T + T(20.8)- SM: white to light gray with zones of orangish brown to brown, damp, medium dense to loose, no plasticity, fine to upper medium grain, saprolite, noncohesive; completely weathered to residual soil; Annular Seal: 3/8 Hole Plug (medium bentonite chips) remnant banding visible in darker (more mafic) zones; gravel sized, angular rock fragments included; rock fragment size and strength 22 754.8 (hardness) increases with depth; micaceous (25.0)-Filter: 20/30 Silica Sand 752.9 (26.9)Well: 2" OD PVC (SCH 40) 30 - SM: white to light gray with zones of orangish brown to brown, -Screen: 10 ft; pre-pack damp, medium dense to loose, no plasticity, fine to upper medium grain, saprolite, noncohesive; completely weathered to residual soil; remnant banding visible in darker (more mafic) zones; gravel sized, angular rock fragments included; rock fragment size and strength (hardness) increases with depth; micaceous 742.9 Sump:0.30 ft. 742.6 Bottom of borehole at 37.2 feet.



2012 GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 17:36 - \\ALTRCFS02\X2DBSMEL\$\GINT\PLANT YATES HYDRO-GEOLOGICAL STUDY.GPJ

LOG OF TEST BORING AND WELL INSTALLATION

BORING YGWA-6I/PZ-6I

PAGE 1 OF 2

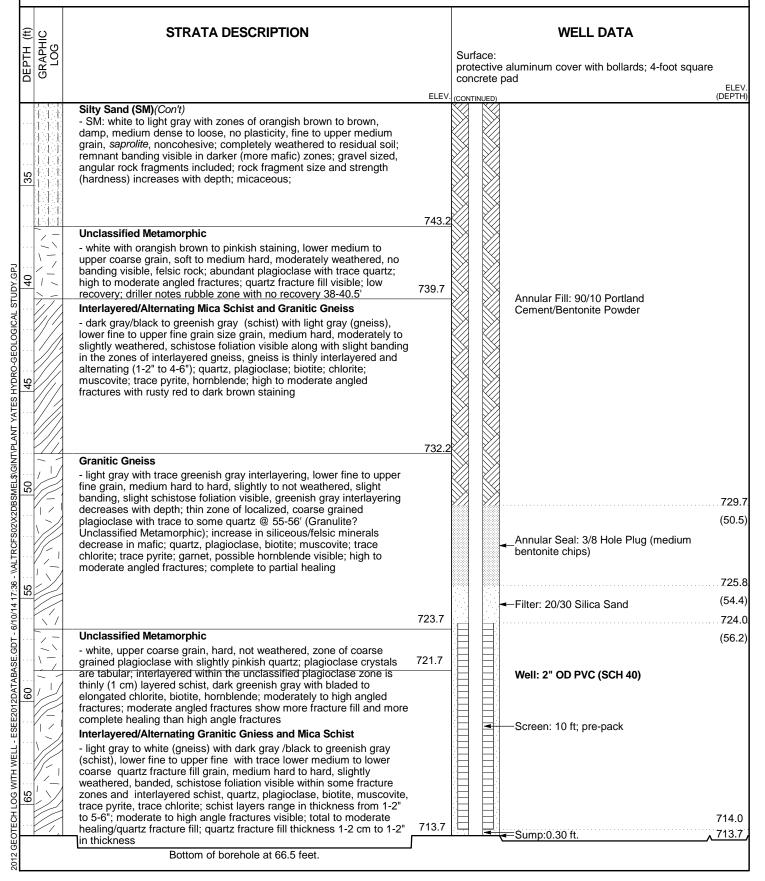
	COMPANY	AND WEL	L INSTA	LLA	TION			ECS37976
	UTHERN COMPANY SERVICES, INC. RTH SCIENCE AND ENVIRONMENTAI	ENCINEEDING			_	o-Geological Study		
EAI	RTH SCIENCE AND ENVIRONMENTAL	LENGINEERING	LOCATION	Newna	an, GA			
	E STARTED 4/21/2014 COMPLETED TRACTOR Cascade Drilling						,490.02 E:2,074,7	790.49
DRILI	LED BY D. Wilcox LOGGED BY	B. Smelser	CHECKED BY	<i></i>		ANGLE	BEARING	
	ING DEPTH 66.5 ft. GROUND WAT							
NOTE	ES _Top of Casing Elevation = 782.73							
<u>E</u> <u>O</u>	STRATA DESC	CRIPTION				WELL D	ATA	
GRAPHIC	90				Surface:			
티윤	_				protective concrete p	aluminum cover with bad	bollards; 4-foot sq	
				ELEV.				ELEV (DEPTH
	Poorly-graded Sand with Silt (SP-SM - dark brown, damp, loose, no plastic		er fine grain,	0.0	₽	←Surface Seal: conc	rete	
	cohesive; trace organics; trace grave to subangular grains	el sized rock fragment	s; angular	·				778.2
				777.2				(2.0)
	Silty Sand (SM) - rusty red, dry, medium dense, no pl	lasticity lower fine to	upper fine					
Ω	grain, cohesive; slight increase in cla							
	[4] [4]							
131 131 1	[4] [4]							
	 SM: orangish brown to tan grading dense to loose, no plasticity, upper fi 							
	upper medium grain, saprolite, nonco	hesive; completely w	eathered to					
6	residual soil; trace gravel sized rock from brittle/friable to hard/more comp	petent; micaceous; mu						
	chlorite, biotite, quartz, plagioclase ic	dentifiable		8				
	194 (1)							
	(1) (1)							
<u>হ</u>	▼ - SM: medium brown/orangish brown	to medium grav with	areenish					
14.4 14.4	gray mottling, damp, medium dense	to loose, no plasticity,	, upper fine					
	to upper medium grain, saprolite, nor to residual soil; zones of remnant bar	nding visible; brittle to	hard rock			Annular Fill: 90/10 Cement/Bentonite		
	fragments included; chlorite, biotite, i identifiable; harder/more competent i					Començ Bentonite i	owaci	
	(quartz, plagioclase) and brittle/friable mafic (biotite, chlorite)	e rock fragments tend	I to be					
8								
	Θ			K				
	- SM: white to light gray with zones o							
	damp, medium dense to loose, no pl grain, saprolite, noncohesive; comple	etely weathered to res	idual soil;					
	remnant banding visible in darker (mangular rock fragments included; roc	ore mafic) zones; gravik fragment size and s	vel sized, trenath					
22	(hardness) increases with depth; mic							
<u> </u>	690 1910							
						-		
	1919 1919			8				
	(현) (1년)							
ا ا								



BORING YGWA-6I/PZ-6I
PAGE 2 OF 2
ECS37976

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Yates Hydro-Geological Study





12 GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 19:09 - \\ALTRCFS02X2DBSMEL\$\GINT\PLANT YATES HYDRO-GEOLOGICAL STUDY.GP.

LOG OF TEST BORING AND WELL INSTALLATION

BORING PZ-4S PAGE 1 OF 1 ECS37976

PROJECT Plant Yates Hydro-Geological Study SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING LOCATION Newnan, GA DATE STARTED 4/11/2014 COMPLETED 5/21/2014 SURF. ELEV. 781.8 COORDINATES: N:1,254,442.86 E:2,075,454.20 EQUIPMENT PS-150 METHOD Rotosonic CONTRACTOR Cascade Drilling __ ANGLE _____ BEARING __ **DRILLED BY** D. Wilcox LOGGED BY B. Smelser CHECKED BY _ BORING DEPTH 30.3 ft. GROUND WATER DEPTH: DURING _____ COMP. 18.98 ft. DELAYED ___ **NOTES** Top of Casing Elevation = 784.25 STRATA DESCRIPTION **WELL DATA** Œ GRAPHIC DEPTH Surface: protective aluminum cover with bollards; 4-foot square concrete pad ELEV. (DEPTH) FLEV Sandy Silt (ML) - rusty red, damp, medium stiff, low plasticity, very fine grain, Surface Seal: concrete cohesive; micaceous; trace organics 779.3 (2.0)Silty Sand (SM) - reddish brown to light brown, dry, medium dense to loose, no plasticity, lower fine to upper medium grain, some to trace clay decreasing with depth; trace mica; trace organics 2 - SM: medium to light brown to tan, dry, loose, no plasticity, lower fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; noncohesive; trace rock fragments (brittle); trace Annular Fill: 90/10 Portland Cement/Bentonite Powder - SM: light brown to tan grading to reddish brown @ 15', damp, loose, no plasticity, lower fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; noncohesive; trace rock fragments (brittle); trace mica 15 (14.5)Annular Seal: 3/8 Hole Plug (medium - SM: reddish brown to tan to white with a greenish tan zone @ bentonite chips) approx. 18-20', moist, medium dense, no plasticity, lower fine to upper medium grain, saprolite, visible zones where saprolite has not 763.6 completely broken down to residual soil and remnant rock fabric visible; zone of more competent saprolite observed; gravel sized rock (18.2)fragments included; muscovite, biotite, chlorite phyllosilicates visible ⊢Filter: 20/30 Silica Sand ... 761.8 (20.0)Well: 2" OD PVC (SCH 40) Screen: 10 ft; pre-pack - SM: orangish brown to light gray to white, moist, medium dense, no plasticity, lower fine to upper medium grain, *saprolite*, increasing rock fragment size and abundance with depth; rock fragments range from coarse gravel to cobble size; angular fragments 751.8 751.5 **Sump:**0.30 ft. **7**51.5 Bottom of borehole at 30.3 feet.



2012 GEOTECH LOG WITH WELL - ESEE2012DATABASE.GDT - 6/10/14 18:37 - \\ALTRCFS02X2DBSMEL\$\G\NT\PLANT YATES HYDRO-GEOLOGICAL STUDY.GPJ

LOG OF TEST BORING AND WELL INSTALLATION

BORING PZ-5S PAGE 1 OF 1 ECS37976

PROJECT Plant Yates Hydro-Geological Study SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING LOCATION Newnan, GA **DATE STARTED** 4/10/2014 **COMPLETED** 5/21/2014 **SURF. ELEV.** 782.2 **COORDINATES:** N:1,254,404.42 E:2,076,211.43 EQUIPMENT PS-150 METHOD Rotosonic CONTRACTOR Cascade Drilling ANGLE BEARING **DRILLED BY** D. Wilcox LOGGED BY B. Smelser CHECKED BY BORING DEPTH 39.5 ft. GROUND WATER DEPTH: DURING COMP. 13.53 ft. DELAYED **NOTES** Top of Casing Elevation = 784.64 STRATA DESCRIPTION **WELL DATA** Œ GRAPHIC DEPTH (Surface: protective aluminum cover with bollards; 4-foot square concrete pad ELEV. (DEPTH) FLEV Silty Sand (SM) Surface Seal: concrete - brown, dry, medium dense, no plasticity, upper fine to lower medium grain, noncohesive; trace organics; angular to sub angular grains 780.2 - SM: brown, dry, medium dense, no plasticity, upper fine to lower 144 (2.0)medium grain, slight increase in clay content with depth; cohesive (slight); trace mica; trace coarse grains Poorly-graded Sand (SP) - light gray to off white, dry, very loose, no plasticity, upper fine to upper medium grain, noncohesive; upper coarse to coarse gravel sized weathered rock fragments; angular to subangular grains; angular rock fragments - SP: med gray to tan to light gray /white with orangish brown to greenish gray mottling, damp, medium dense, no plasticity, upper fine to lower medium grain, saprolite, noncohesive; completely Annular Fill: 90/10 Portland weathered to residual soil; zones of more competent (completely weathered) rock increasing with depth; trace lenses of silt/clay Cement/Bentonite Powder interbedded within the sand/saprolite; brittle upper coarse to lower 5 gravel sized rock fragments included; micaceous - SP: light gray/white grading to med gray with orangish brown to dark gray mottling, damp, medium dense, no plasticity, upper fine to lower medium grain, saprolite, noncohesive; completely weathered to residual soil; angular to subangular grains 2 758.5 (23.7)25 Annular Seal: 3/8 Hole Plug (medium bentonite chips) - SP: light gray to med gray, damp, loose, no plasticity, upper fine to 754.8 lower medium grain, saprolite, increasing in gravel sized rock -Filter: 20/30 Silica Sand (27.4)fragments (completely weathered, very brittle) 8 753.0 (29.2)Well: 2" OD PVC (SCH 40) Screen: 10 ft; pre-pack 35 - SP: medium gray to light gray, damp, loose, no plasticity, upper fine to upper coarse grain, saprolite, noncohesive; completely weathered to residual soil with zones of more competent but brittle rock fragments; angular grains; micaceous 743.0 742.7 Sump:0.30 ft. 742.7



BORING PZ-6D PAGE 1 OF 4 ECS37976

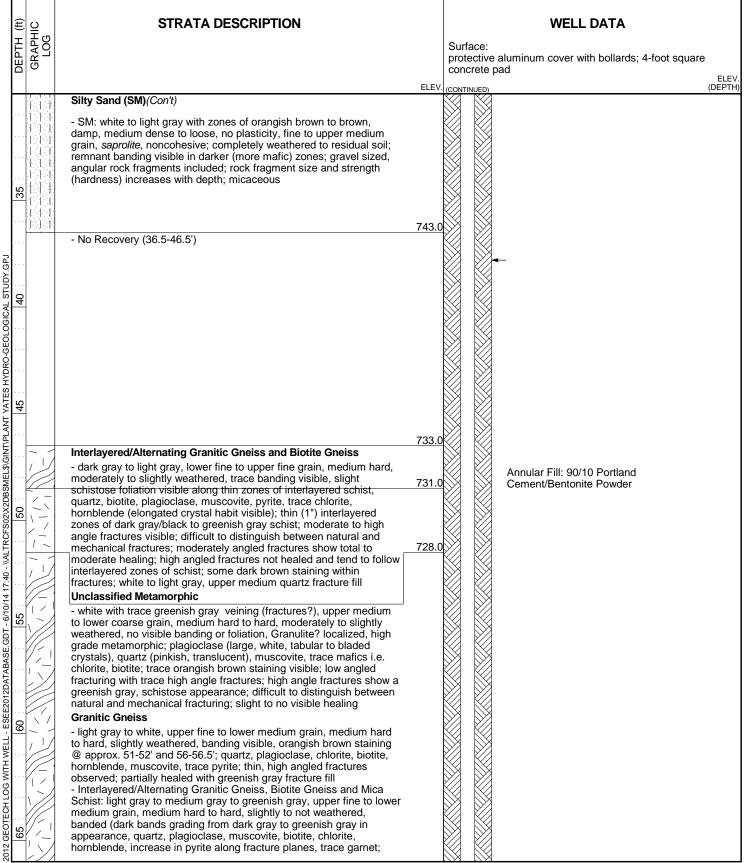
		D BY _D. WilcoxLOGGED BY _B. Smelser CHECKED BY _B. Smelser CHEC	_ COMI	MP. 18.93 ft. DELAYED	
DEPTH (ft)	GKAPHIC LOG	STRATA DESCRIPTION		WELL DATA Surface: protective aluminum cover with bollard concrete pad	s; 4-foot square
25		Poorly-graded Sand with Silt (SP-SM) - dark brown, damp, loose, no plasticity, lower fine to upper fine grain, cohesive; trace organics; trace gravel sized rock fragments; angular to subangular grains Silty Sand (SM) - rusty red, dry, medium dense, no plasticity, lower fine to upper fine grain, cohesive; slight increase in clay content; micaeous - SM: orangish brown to tan grading to light gray/white, dry, medium dense to loose, no plasticity, upper fine to lower medium with trace upper medium grain, saprolite, noncohesive; completely weathered to residual soil; trace gravel sized rock fragments; rock fragments range from brittle/friable to hard/more competent; micaeous; muscovite, chlorite, biotite, quartz, plagioclase identifiable - SM: medium brown/orangish brown to medium gray with greenish gray mottling, damp, medium dense to loose, no plasticity, upper fine to upper medium grain, saprolite, noncohesive; completely weathered to residual soil; zones of remnant banding visible; brittle to hard rock fragments included; chlorite, biotite, muscovite, quartz, plagioclase identifiable; harder/more competent rock fragments tend to be felsic (quartz, plagioclase) and brittle/friable rock fragments tend to be mafic (biotite, chlorite) - SM: white to light gray with zones of orangish brown to brown, damp, medium dense to loose, no plasticity, fine to upper medium grain, saprolite, noncohesive; completely weathered to residual soil; remnant banding visible in darker (more mafic) zones; gravel sized, angular rock fragments included; rock fragment size and strength (hardness) increases with depth; micaeous	776.5	Surface Seal: concrete	(C



BORING PZ-6D PAGE 2 OF 4 ECS37976

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Yates Hydro-Geological Study





BORING PZ-6D PAGE 3 OF 4 ECS37976

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Yates Hydro-Geological Study

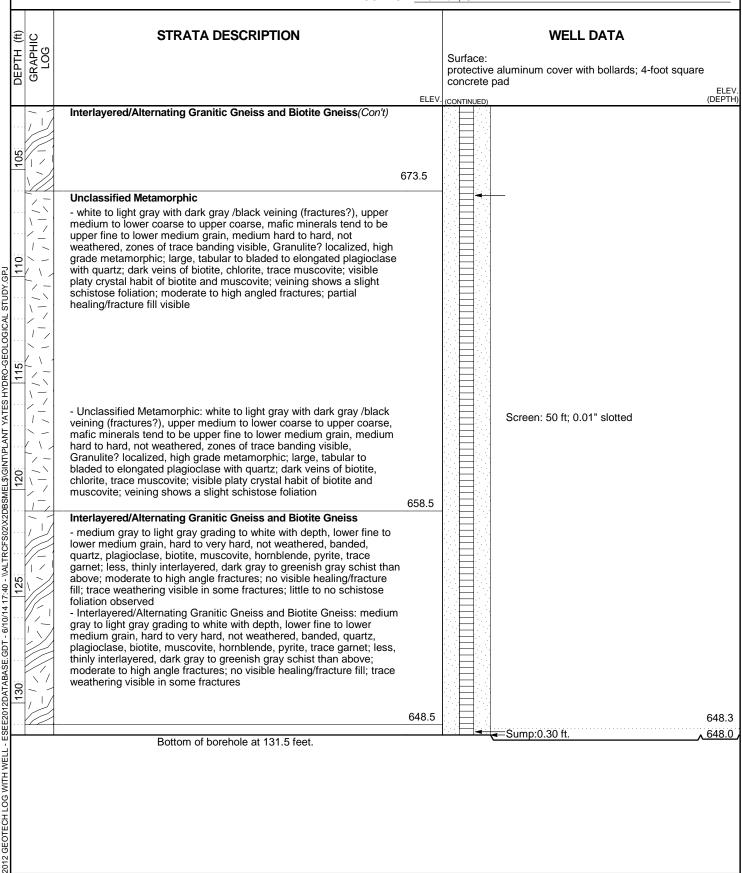
(ft)	GRAPHIC LOG	STRATA DESCRIPTION					WELL DATA
DEPTH	LOG SAPI				rfac		a aluminum aguar with hallanda. 4 fact aguara
	<u>в</u>						e aluminum cover with bollards; 4-foot square pad
			ELEV.	(CON	ITINU	JED)	ELEV. (DEPTH)
02		thin, high angled fractures observed; partially healed with greenish gray fracture fill; interlayered, greenish gray schist averages 1-2 mm thick throughout the unit Granitic Gneiss (<i>Con't</i>) - Interlayered/Alternating Granitic Gneiss, Biotite Gneiss and Mica Schist: light gray to medium gray to greenish gray, upper fine to upper medium grain, medium hard to hard, not weathered, banded (dark bands grading from dark gray to greenish gray in appearance, quartz, plagioclase, muscovite, biotite, chlorite, hornblende, increase in pyrite along fracture planes, trace garnet; thin, high angled fractures observed; partially healed with greenish gray fracture fill; interlayered, greenish gray schist averages 1-2 mm thick					Annular Fill: 90/10 Portland Cement/Bentonite Powder 705.9
[[\						(73.6)
75	1.14		704.5				
	\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\\	 Unclassified Metamorphic white to light gray with dark gray /black veining (fractures?), upper medium to lower coarse to upper coarse, mafic minerals tend to be upper fine to lower medium grain, medium hard to hard, slightly to not weathered, zones of trace banding visible, Granulite? localized, high grade metamorphic; large, tabular to bladed to elongated plagioclase 	700 T				Annular Seal: 3/8 Hole Plug (medium bentonite chips) 701.0
ا ا		with quartz; dark veins of biotite, chlorite, trace muscovite; visible platy crystal habit of biotite and muscovite; veining shows a slight	700.5				(78.5
80		schistose foliation					Filter: 20/30 Silica Sand
100		Interlayered/Alternating Granitic Gneiss and Biotite Gneiss - medium gray to light gray grading to white with depth, lower fine to lower medium grain, hard to very hard, not weathered, trace banding with some zones having a schistose foliation, quartz, plagioclase, biotite, muscovite, hornblende, pyrite, trace garnet; pyrite abundant in fractures; thinly (1-2 cm to 1-2") interlayered, dark gray to greenish gray schist; moderate to high angle fractures; trace weathering visible in some fractures					(81.2)
		- Interlayered/Alternating Granitic Gneiss and Biotite Gneiss: medium gray to light gray grading to white with depth, lower fine to lower medium grain, hard to very hard, not weathered, trace banding with some zones having a schistose foliation, quartz, plagioclase, biotite, muscovite, hornblende, pyrite, trace garnet; pyrite abundant in fractures; thinly (1-2 cm to 1-2") interlayered, dark gray to greenish gray schist; moderate to high angle fractures; trace weathering visible in some fractures					
	//-						Screen: 50 ft; 0.01" slotted
	$ \cdot $						
							Well: 2" OD PVC (SCH 40)
1001		- Interlayered/Alternating Granitic Gneiss and Biotite Gneiss: medium gray to light gray grading to white with depth, lower fine to lower medium grain, hard to very hard, not weathered, trace banding with some zones having a schistose foliation, quartz, plagioclase, biotite, muscovite, hornblende, pyrite, trace garnet; pyrite abundant in fractures; thinly (1-2 cm to 1-2") interlayered, dark gray to greenish gray schist; moderate to high angle fractures; trace weathering visible					



BORING PZ-6D PAGE 4 OF 4 ECS37976

SOUTHERN COMPANY SERVICES, INC. EARTH SCIENCE AND ENVIRONMENTAL ENGINEERING

PROJECT Plant Yates Hydro-Geological Study



ARCADIS Progres & Consultance	No.: PZ-24IA	
Boring Log/Well Construction Log	Sheet: 1 of	3
Project Name: Plant Yates Date Started: 06/02/2020 Logger: Cleme	ent Papafio	
Project Number: 30055278 Date Completed: 06/03/2020 Editor: Grant		
Project Location: Newnan, GA Weather Conditions: -		
Depth (feet) Sample Interval Counts Recovery (in.) Photo Log PID (ppm) Counts Description	Construction Details	Well
0	Surface completion consists of a locking monument above 2.85' ground surface with a weep hole, vent hole in well casing, pea gravel between well monument and well casing, 4'x4'x4" concrete pad, and four concrete bollards. Portland Cement with 6% Bentonite. 2 inch diameter Schedule 40 PVC riser.	
		X X
Drilling Co.: Cascade Drilling Sampling Method: Core Barrel		
Drilling Method: Rotosonic Water Level Start (ft. bgs.):		
ı l		
Drilling Fluid: Water Water Level Finish (ft. btoc.):		
اد	□ No	
Drilling Fluid: Water Water Level Finish (ft. btoc.):		
Drilling Fluid: Water Water Water Level Finish (ft. btoc.): Vestage		

Project Project	Name: Number:	Plant 3005	Yates 5278	nstruction			Date Started: 06/02/2020 ate Completed: 06/03/2020	Logger: <u>Cleme</u> Editor: <u>Grant</u> nditions: -	Willford		
Depth (feet)		Blow	Recovery	Photo Log	PID (ppm)	Graphic Log		Tiditions	Construction Details	<u> </u>	Vell
							(29.0-39.0') Silty sand (SM); grayish br 5/2); fine grained to medium grained so oxidation present. (33.0-34.0') Color change to mottled ve (10YR 7/4) and white (10YR 8/1). (34.0-39.0') Color change to very pale 8/2); loose; dry; saprolitic. (39.0-44.0') Clayey silty sand (SM-SC) brown (10YR 6/4); fine grained to med sand; trace clay; loose; slight plasticity moist. (44.0-48.0) Silty sand (SM); pale brown fine grained to medium grained sand; loose grained sand; weathered quartz (49.0-52.0') Poorly graded sand (SP); (10YR 8/3); fine grained to medium grasome silt; loose; dry. (52.0-54.0') Silty sand (SM); mottled lig (7.5YR 6/3) and light olive gray (5Y 6/2 to medium grained sand; mica present (54.0-59.0') Well graded sand (SW); motwn (2.5YR 6/4) and white (10YR 8/1 to coarse grained sand; loose to mediur esidual rock gravel; possible transitior weathered bedrock (PWR); dry. (59.0-71.0') Granitic gneiss; light gray feldspar, quartz, mica mineralogy; high moderately weathered; intensely fractuhorizontal to horizontal fractures; staininurface medium foliation; joints and fracture medium foliation; joints and fracture medium; low recovery; very poor RC	ery pale brown brown (10YR ilight yellowish um grained ; mica present; in (10YR 6/3); oose; moist. in (10YR 6/3); oose; moist.	Portland Cement with 6% Bentonite. 2 inch — diameter Schedule 40 PVC riser.		

ARCADIS Source Educations of the Property and State Source State S				Boring	No.: <u>PZ-24IA</u>	
Boring Log/Well Cou Project Name: Plant Yates Project Number: 30055278 Project Location: Newnan, GA		Da	Date Started: 06/02/2020 ate Completed: 06/03/2020 Weather Co	Logger: <u>Cleme</u> Editor: <u>Grant</u> nditions: -	ent Papafio Willford	3
Depth Sample Blow Recovery (feet) Interval Counts (in.)	Photo Log PID (ppm		Description		Construction Details	Well
63			(59.0-71.0') Granitic gneiss; light gray feldspar, quartz, mica mineralogy; high moderately weathered; intensely fractionizontal to horizontal fractures; stain surface medium foliation; joints and fraction foliation; low recovery; very poor RC (71.0-82.5') Granitic gneiss; gray (7.5) feldspar, quartz, mica mineralogy; thin foliation moderately weathered; model staining on fracture surface.	red; sub ing on fracture actures parallel 2D. (R 5/1); to moderate	Portland Cement with 6% Bentonite. 2 inch — diameter Schedule 40 PVC riser.	
74	77:-29.7.A (m: 587)		(75.0-82.5') Near horizontal fractures; foliated; slightly weathered to fresh; ha color change to white (7.5YR 8/1). (82.5-87.0') Granitic gneiss; light gray feldspar, quartz, mica mineralogy; inte moderately fractured; near horizontal t fractures; very slightly weathered to fre moderatly hard; very poor RQD.	(5Y 7/1); nsely to o sub horizontal	Bentoniteseal. Filter pack: _#1A (12-40) _sand 2 inch _diameter Sch. 40 PVC U-Pack dual _wall 0.010-inch slotted screen.	
88			End of boring 87.0 ft bgs.			

RECORD OF BOREHOLE PZ-35

PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 47.10 ft LOCATION: Newnan, GA DRILL RIG: Sonic PS-150 DATE STARTED: 7/13/16 DATE COMPLETED: 7/20/16

NORTHING: 1,258,593.16 EASTING: 2,073,805.60 GS ELEVATION: 740.9 TOC ELEVATION: 743.81 SHEET 1 of 1
DEPTH W.L.: 9.99 ft (bgs)
ELEVATION W.L.: (amsl)
DATE W.L.: 7/20/16
TIME W.L.: N/A

SOIL PROFILE ft SAMPLES ELEVATION (ft) DEPTH (ft) MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES WELL CONSTRUCTION 9 ELEV. GRAPHIC LOG nscs TYPE SAMPLE REC DESCRIPTION **DETAILS** DEPTH (ft) 0.00 - 10.00 WELL CASING 740 No recovery; Hydrovac Interval: 0.0'-36.0' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded WELL SCREEN Interval: 36 0'-46 0' Material: U-Pack Schedule 40 735 Slot Size: 0.010" End Cap: Schedule 40 PVC FILTER PACK Interval: 34.0'-46.0' Type: #1 Type Sand 730.90 10 10.00 - 11.00 10.007 29.90 11.00 SM FILTER PACK SEAL silty SAND, ash in core sample possibly recovered from hydrovac zone above 730 Interval: 31.0'-34.0' Type: Bentonite Pellets and Chips СН CLAY, dark reddish browm, cohesive, moist ANNULUS SEAL Interval: 0.0'-31.0' 726.90 14.00 - 17.00 14.00 Type: Portlant Type 1 SAND, fine sand, mottled brown and grey, moist, loose 15 Portland SP WELL COMPLETION Type 1 725 723.90 Protective Casing: Aluminum 17 00 - 27 00 17.00 fine sand, mottled brown and grey, relict laminations, moist, loose DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic 20 720 10.00 10.00 2 25 - 715 713.90 27.00 - 30.00 27.00 silty SAND, dull grey to brown, moist, loose SM 710.90 30 30.00 - 32.00 30.00 mottled brown to tan, lamination, dry to moist, loose 710 708.90 3 Bentonite 32.00 - 37.00 32.00 Pellets and mottled brown to tan, lamination, micaceous, dry to moist, loose Chips 35 9/26/17 705 0.010" 703.90 Slotted 37.00 - 42.00 Screen PIEDMONT.GDT grey, dry, loose #1 Type 700 698.90 4.00 YATES BORING LOGS.GPJ 4 42.00 - 47.00 42.00 transitionally weathered rock - biotite/muscovite GNEISS, weathered, friable PWR 695 Sump Boring completed at 47.10 ft 47.00

LOG SCALE: 1 in = 6.5 ft

DRILLING COMPANY: Cascade Drilling

DRILLER: Dale

GA INSPECTOR: Ben Hodges CHECKED BY: Rachel Kirkman, PG



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 47.00 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE PZ-37

DRILL RIG: Sonic PS-150
DATE STARTED: 6/29/16
DATE COMPLETED: 7/6/16

REASTING: 2,074,699
GS EL EVATION: 75

NORTHING: 1,256,471.14 EASTING: 2,074,699.59 GS ELEVATION: 758.0

SHEET 1 of 1 DEPTH W.L.: 6.0 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/6/2016 TIME W.L.: 07:40

-	Т	4			TOC			N: 76	60.78
	z	SOIL PROFILE ft					AMPLE	s	
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC	ELEV. DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES DETAILS
0 -		0.00 - 3.00 silty CLAY, red to brown, micaceous	CL-ML		755.00	0,			WELL CASING Interval: 0.0'-36' Material: Schedule 40 F Diameter: 2 Joint Type: Threaded
5 —	755 _ - - -	3.00 - 5.00 silty SAND, trace gravel, pale grey to green, plagioclase nodules (saprolite) 5.00 - 7.00 trace clay, white and orange nodules, plagioclase, dry (saprolite)	SM		3.00 753.00 5.00	1		7.00 7.00	WELL SCREEN Interval: 36.5'-46.5' Material: U-Pack Sched
-	- 750	7.00 - 9.50 trace gravel, white to dark brown, muscovite			751.00 7.00				Diameter: 2 Slot Size: 0.010" Slotter Screen End Cap: Schedule 40
10 -	-	9.50 - 17.00 trace clay, pale yellow brown, plagioclase nodules, dry (saprolite)			749.00 9.50			10.00	FILTER PACK Interval: 33.0'-47.0' Type: #1 Type Sand FILTER PACK SEAL
-	- 745 -					2		10.00	Portland _ Type 1 Interval: 25.0'-33.0' Type: Bentonite Pellets Chips ANNULUS SEAL Interval: 0.0'-25.0'
15 -	- - -	17.00 - 27.00			741.00 17.00				Thetval. U.7-23.0 Type: Portland Type 1 WELL COMPLETION Pad: 4'\4'\4'' Protective Casing: Alun
20	740 	pale yellow brown to brown, white and brown nodules, saprolitic gneiss, dry							DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
- - - -	- - - 735					3		10.00 10.00	
25 —	- - -								
-	- 730 	27.00 - 37.00 transitionally weathered rock-feldspathic GNEISS, muscovite, biotite, feldspar, quartz, foliated and layered, dark green, amphiboile lense		D	731.00 27.00				Bentonite Pellets and — Chips
30 —	- - - 725		PWR	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Å; A	4		10.00 10.00	
35 —	- - -				721.00				
40 —	720 720	37.00 - 43.00 bedrock - feldspathic GNEISS, biotite, muscovite, quartz, plagioclase, grey to white, more feldspar and quartz, oxidized and fractured	GNEIS		37.00				
-	- - - 715	43.00 - 47.00			715.00 43.00	5		8.00 10.00	0.010" Slotted –
45 —	- - -	more granitic, lots of feldspar and quartz							#1 Type
=	- 710 -	Boring completed at 47.00 ft			711.00				
		LE: 1 in = 6.5 ft COMPANY: Cascade Drilling			ISPECT				



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 56.10 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE

DRILL RIG: Sonic PS-150

DATE STARTED: 7/11/16

DATE COMPLETED: 7/12/16

PAGE 15/16

DATE COMPLETED: 7/12/16

DATE COMPLETED: 7/12/16

PAGE 15/16/16

CS E1 EVALUABLE 777.2 GS ELEVATION: 777.2 TOC ELEVATION: 779.83 ft

SHEET 1 of 2

DEPTH W.L.: 17.71 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/13/2016 TIME W.L.: 15:25

		0011 000511 5			100				9.83 ft	
_	NO	SOIL PROFILE					AMPLE	:S		
OEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	DEPTH	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
0 -	- 775	0.00 - 10.00 No recovery; hydrovac			(ft)	Ŝ				WELL CASING Interval: 0.0'-45.8' Material: Schedule 40 PV0
-	— 775 - -									Diameter: 2" Joint Type: Threaded WELL SCREEN Interval: 45.8'-55.8'
5 —	- - -									Material: U-Pack Schedul PVC Diameter: 2" Slot Size: 0.010'
-	 770 - -				767.00				_	End Cap: Schedule 40 P\ FILTER PACK Interval: 43.1'-56.1' Type: #1 Type Sand
10 -	_ _ 	10.00 - 12.00 clayey SAND, coarse sand, dark red, cohesive	sc		767.20 10.00 765.20					FILTER PACK SEAL Interval: 38.5'-43.1' Type: Bentonite Pellets a
-	— 765 - -	12.00 - 14.00 sandy SILT to silty SAND, pinkish orange 14.00 - 16.00	SP-ML		763.20 14.00	1		6.00 6.00		Chips ANNULUS SEAL Interval: 0.0'38.5' Type: Portland Type 1
15 —	-	sandy SILT, reddish brown, moist loose 16.00 - 22.00 sandy SILT to silty SAND, dark reddish brown, trace gravel, dry,	ML		761.20 16.00				- -	Type: Portland Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Alumir
-	— 760 – –	loose	SP-ML						Portland	- DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
20 —	-				755.20	2		10.00 10.00	Type 1	
-	— 755 - -	22.00 - 26.00 silty SAND, mottled white and grey, high plagioclase content	SM		22.00					-
25 — - -	- - 750	26.00 - 32.00 poorly sorted, greyish brown, dry, loose			751.20 26.00				Portland	
30 —	- - -									
-	- 745	32.00 - 36.00 transitionally weathered rock, pegmatite GRANITE, highly		D A D A	745.12	3		10.00		
35 —	-	weathered, large plagioclase and quartz crytals	PWR		741.20				-	
-	- 740 	36.00 - 46.00 biotite GNEISS, some staining near top of core, pyrite inclusions			36.00					
- 40 — -	- - -		GNEISS			4		10.00 10.00	Bentonite Pellets and —	
-	 735 							13.00	Chips -	
45 — -	-	46.00 - 49.00			731.20				0.010"	
-	— 730 — —	biotite GNEISS			728.20 49.00	5		10.00 10.00	Slotted – Screen – -	
50 —		Log continued on next page		نسيسن	1				:'	1
		LE: 1 in = 6.5 ft COMPANY: Cascade Drilling			SPECT(KED BY				es nan, PG	Golder
		Dale			9/29/1					Golder Associat



PROJECT: SCS Plant Yates PROJECT NUMBER: 1660300 DRILLED DEPTH: 56.10 ft LOCATION: Newnan, GA

RECORD OF BOREHOLE

DRILL RIG: Sonic PS-150

DATE STARTED: 7/11/16

DATE COMPLETED: 7/12/16

PAGE 15/16

DATE COMPLETED: 7/12/16

DATE COMPLETED: 7/12/16

PAGE 15/16/16

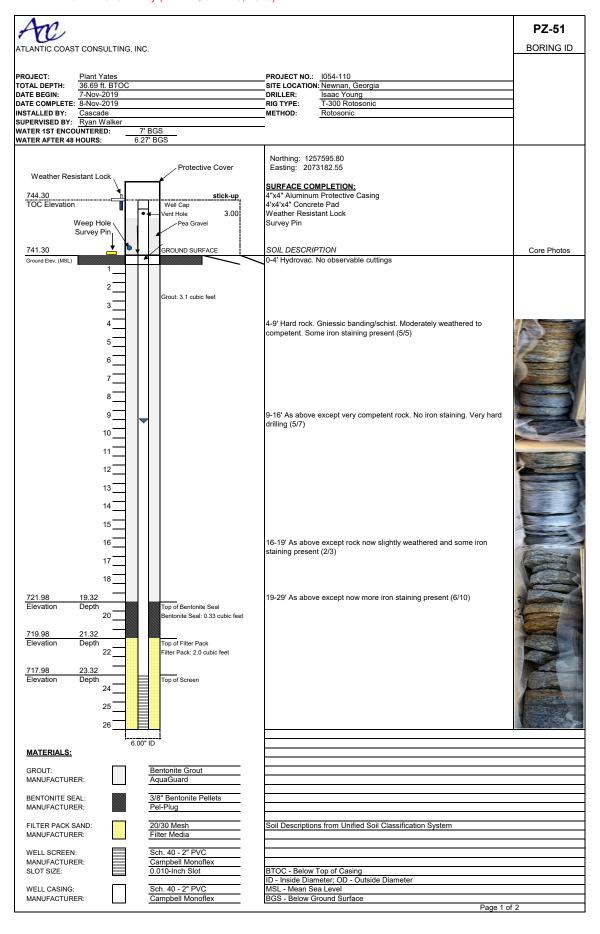
CS EL EVALUABLE 777.2 GS ELEVATION: 777.2 TOC ELEVATION: 779.83 ft

SHEET 2 of 2

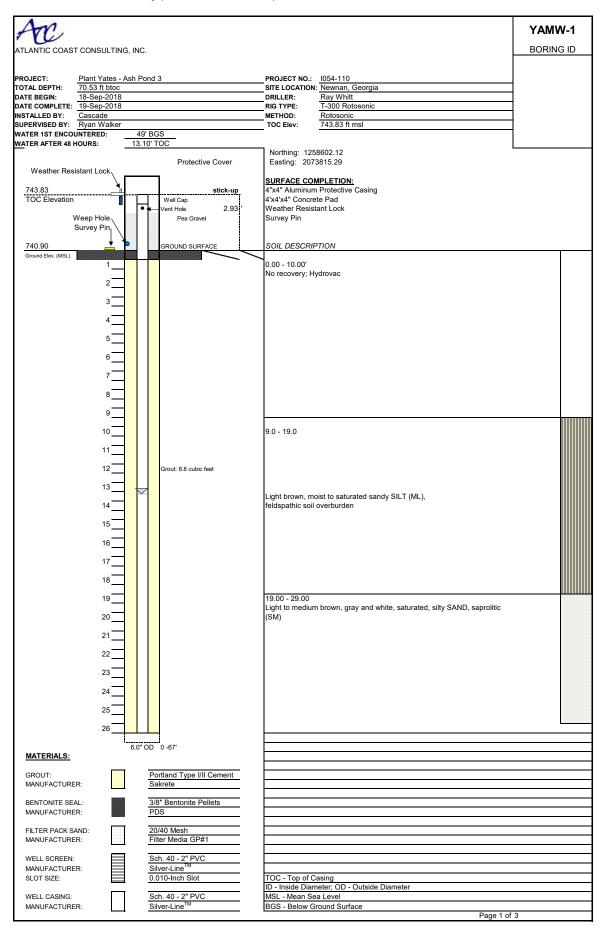
DEPTH W.L.: 17.71 ft (bgs) ELEVATION W.L.: (amsl) DATE W.L.: 7/13/2016 TIME W.L.: 15:25

		SOIL PROFILE					AMPLE		3.00 11	
DEPTH (ft)	ELEVATION (ft)	DESCRIPTION	nscs	GRAPHIC LOG	ELEV. DEPTH (ft)	SAMPLE NO.	TYPE	REC	MONITORING WELL/ PIEZOMETER DIAGRAM and NOTES	WELL CONSTRUCTION DETAILS
50 — - - -	- - 725 -	49.00 - 52.00 biotite GNEISS, migmatic gneiss bands with iron staining (Continued) 52.00 - 56.00 biotite GNEISS			725.20	5		10.00 10.00	#1 Type	WELL CASING Interval: 0.0'-45.8' Material: Schedule 40 PVC Diameter: 2" Joint Type: Threaded
55 —	- - -	Boring completed at 56.10 ft			721.20 56.00				Sump –	WELL SCREEN Interval: 45.8'-55.8' Material: U-Pack Schedule 4 PVC Diameter: 2"
	— 720 –								- -	Slot Size: 0.010' End Cap: Schedule 40 PVC FILTER PACK Interval: 43.1'-56.1'
60 —	- -								-	Type: #1 Type Sand FILTER PACK SEAL Interval: 38.5'-43.1'
- - -	— 715 – –								- - -	Type: Bentonite Pellets and Chips ANNULUS SEAL Interval: 0.0'38.5'
65 —	-								- - -	Type: Portland Type 1 WELL COMPLETION Pad: 4'x4'x4" Protective Casing: Aluminui
-	710 - -								- -	DRILLING METHODS Soil Drill: 4" Sonic Rock Drill: 4" Sonic
70 — - -	- - - 705								_ - -	
75	-								- - -	
75 — - -	- - 700								- - -	
80 —	- - -								- - -	
-	- 695								- - -	
- 85 —	-								- -	
=	- 690 -								- - -	
90 — -	- -								- - -	
-	— 685 –								- -	
95 — -	- -								- - -	
-	— 680 – –								- - -	
100 —									_	
DRII		LE: 1 in = 6.5 ft COMPANY: Cascade Drilling Dale	(CHEC	SPECT KED BY 9/29/1	r: Ra			es nan, PG	Golder



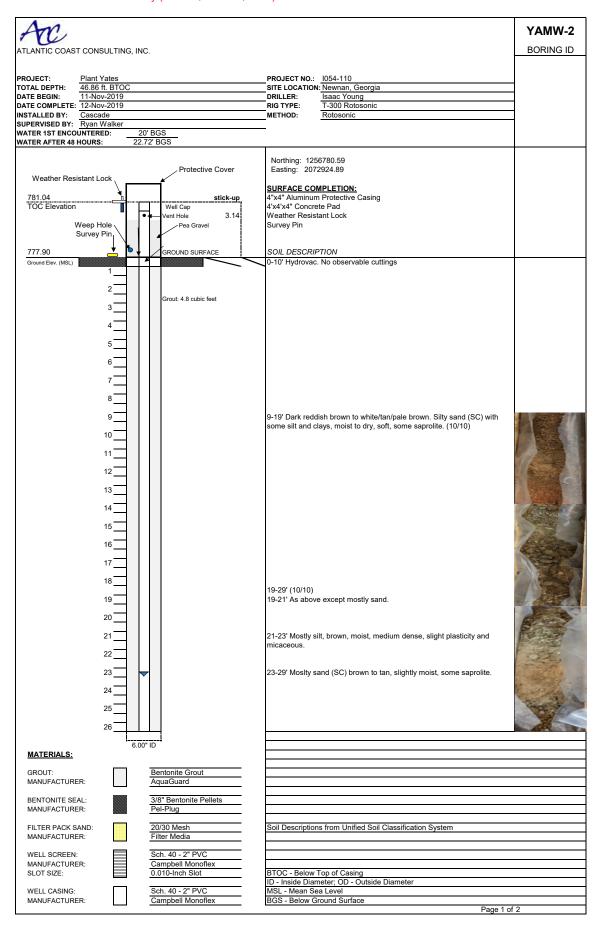


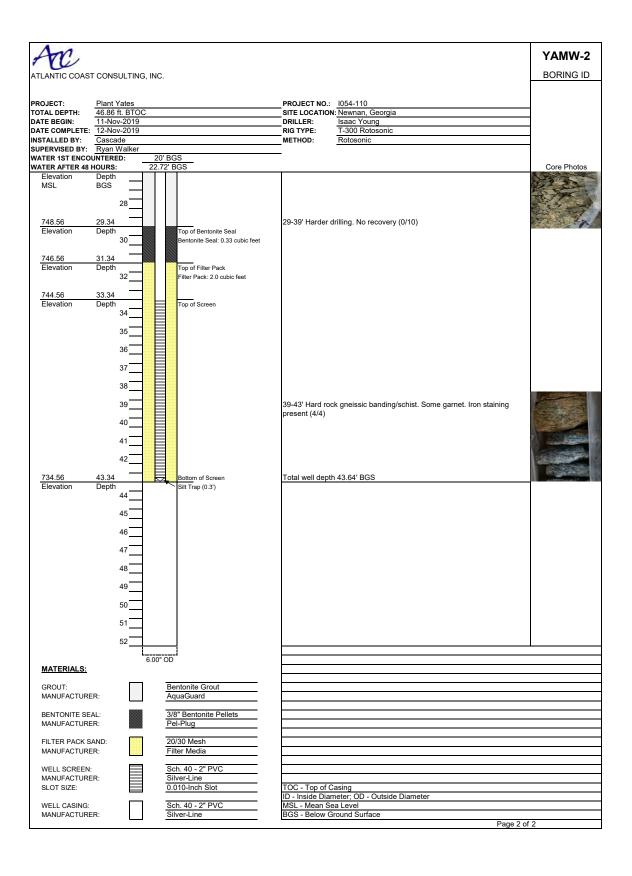
An				PZ-51
ATLANTIC COAS	T CONSULTING,	INC.		BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 36.69 ft. BTOC		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	7-Nov-2019		DRILLER: Isaac Young	
DATE COMPLETE	: 8-Nov-2019 Cascade		RIG TYPE: T-300 Rotosonic METHOD: Rotosonic	
INSTALLED BY: SUPERVISED BY:			METHOD: ROTOSOFIIC	
WATER 1ST ENCO	OUNTERED:	7' BGS	=	
WATER AFTER 48		6.27' BGS		Core Photos
Elevation MSL	28 29 30 31 31 32		29-33' As above except rock now very competent. No iron staining present. Very hard drilling (4/4)	
707.98 Elevation	33.32 Depth	Bottom of Screen Silt Trap (0.3')	Total well depth 33.62' BGS	-
	34			
MATERIALS:	6	i.00" OD		
GROUT:		Bentonite Grout		
MANUFACTUR	ER:	AquaGuard		
BENTONITE SE		3/8" Bentonite Pellets Pel-Plug		
FILTER PACK S MANUFACTUR		20/30 Mesh Filter Media		
WELL SCREEN MANUFACTUR SLOT SIZE:		Sch. 40 - 2" PVC Silver-Line 0.010-Inch Slot	TOC - Top of Casing	
		Sch. 40 - 2" PVC	ID - Inside Diameter; OD - Outside Diameter MSL - Mean Sea Level	
WELL CASING: MANUFACTUR		Sch. 40 - 2" PVC Silver-Line	MSL - Mean Sea Level BGS - Below Ground Surface Page 2 of	2

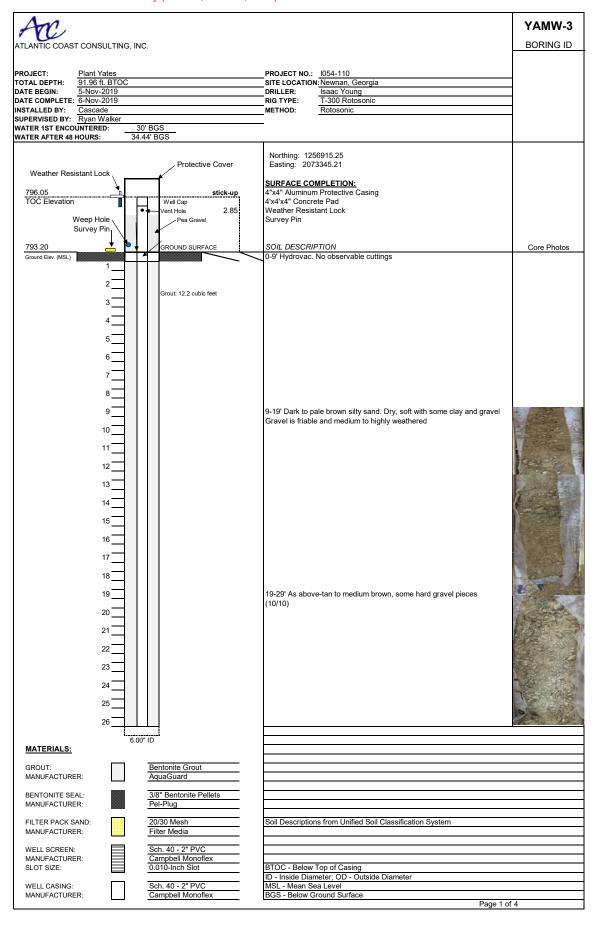


ACC ATLANTIC COAS	T CONSULTING,	INC.				YAMW-1 BORING ID
PROJECT: TOTAL DEPTH: DATE BEGIN: DATE COMPLETE: INSTALLED BY: SUPERVISED BY: WATER 1ST ENCO	Plant Yates - As 70.53 ft. TOC 18-Sep-2018 19-Sep-2018 Cascade Ryan Walker JNTERED:	sh Pond 49' BC	38	PROJECT NO.: SITE LOCATION: DRILLER: RIG TYPE: METHOD: TOC Elev:	I054-110 Newnan, Georgia Ray Whitt T-300 Rotosonic Rotosonic 743.83 ft msl	
Elevation MSL	30 Depth BGS 28 30 30 31 32 33 34 35 36 37 38 38	13.10' 7		(SM) 19.0 -29.0 29.0 - 39.0 Rec	to brown, gray and white, saturated, silty SAND, saprolitic covery (10/10) white and gray, dry silty SAND, micacous and saprolitic	
	39			39.0 - 44.0 Rect Medium brown, (SM)	overy (5/5) white and gray, dry silty SAND, micacous and saprolitic	
	45				rown, Biotite/muscovite gneiss,weathered, ning at 54' (easily fractured)	
<u>Materials:</u>	51 52	6.0" OD	0 -67'	Gray, white brov	wn Gneiss, iron staining	
GROUT: MANUFACTURE BENTONITE SEA MANUFACTURE	AL:	Sa 3/8	ortland Type I/II Cement akrete 8" Bentonite Pellets			
FILTER PACK SA MANUFACTURE WELL SCREEN: MANUFACTURE SLOT SIZE:	:R:	Sc Sil	l/40 Mesh Iter Media GP#1 ch. 40 - 2" PVC Iver-Line TM 010-Inch Slot		eter; OD - Outside Diameter	
WELL CASING: MANUFACTURE	R:		ch. 40 - 2" PVC lver-Line TM	MSL - Mean Sea BGS - Below Gr	a Level	of 3

ACC ATLANTIC COAS	T CONSULTIN	IG. INC.				YAMW-1 BORING ID
						BORINGIB
PROJECT: TOTAL DEPTH:	Plant Yates - 70.53 ft. TOO		3	PROJECT NO.: SITE LOCATION:	Newnan, Georgia	+
DATE BEGIN:	18-Sep-2018			DRILLER:	Ray Whitt	†
DATE COMPLETE:	19-Sep-2018			RIG TYPE:	T-300 Rotosonic	İ I
INSTALLED BY:	Cascade			METHOD:	Rotosonic	Į I
SUPERVISED BY:	Ryan Walker		20	TOC Elev:	743.83 ft msl	<u> </u>
WATER 1ST ENCO WATER AFTER 48		49' B0				
Elevation	Depth	13.10	100			1 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
MSL	BGS					
687.9	53.0					
Elevation	Depth		Top of Seal	49.0 - 59.0 Red	covery (5/10)	
			Bentonite Seal: 0.3 cubic feet			De a tort
685.9	55.0			Gray white brow	wn Gneiss, iron staining, highly fractured	26 2000
Elevation	Depth	_	Top of Filter Pack	Gray, write bro	wir Grieiss, iion stailling, nignly fractured	
2.074.1011			rop of risks r dok			
]				The same of the sa
683.9	57.0					
Elevation	Depth	1 🗐	Top of Screen (56.67')			
	_	- 6				
	59	- 6	Filter Pack: 2.0 cubit feet			7 1 1 7
		1 🗐	Tiller Fack. 2.0 cubit lect			-
	60] 🗐		59.0 - 62.0 Rec	overy (2.1/3.0)	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1
	_					
	61	- ■		Gray, white brow	wn Gneiss, iron staining, highly fractured	
	62	- 6				H ONE HAS
	02	1 🗐				
	63	1 🗐				
		1 🗐		62.0 - 70.0 Rec		
673.9	67.0		Bottom of Screen (66.67')	Hard rock. Visib	ly harder drilling. Biotite/muscovite gneiss, non-friable.	
Elevation	Depth	-	Silt Trap (4.0")	Grav white hro	wn Gneiss, iron staining, highly fractured	
	_	1		Gray, write bro	wir dreibe, non stammig, riiginy riadtarea	
						(M.R.W.)
	_	-		Boring terminate	ed at 70.0' BGS	
		-				
	_					
	_					
		-				
		1				
	_					
	_					
		-				
		-				
		1				
	_					
		-				
		-				
		1				
]			
		6.0" OD	0 -67'			
MATERIALS:						
GROUT:		Po	ortland Type I/II Cement			
MANUFACTURE	R:		akrete			
BENTONITE SE			8" Bentonite Pellets			
MANUFACTURE	R:	PI PI	DS			
FILTER PACK S	AND:	20	0/40 Mesh			
MANUFACTURE			Iter Media GP#1			
WELL SCREEN:			ch. 40 - 2" PVC			
MANUFACTURE	R:		ilver-Line TM	T00 T		
SLOT SIZE:		0.	010-Inch Slot	TOC - Top of C	asing neter; OD - Outside Diameter	
WELL CASING:		Sc	ch. 40 - 2" PVC	MSL - Mean Se		
MANUFACTURE	R:		ilver-Line TM	BGS - Below G	round Surface	
	_				Page 3 of	3



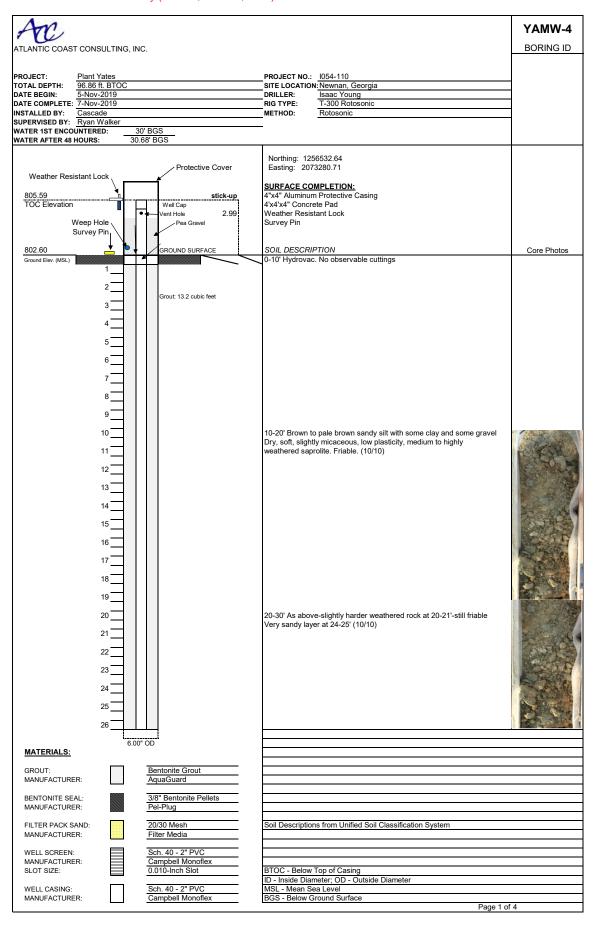




Arc					YAMW-3
ATLANTIC COAS	T CONSULTING	G, INC	D.		BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 91.96 ft. BTOO	7		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	5-Nov-2019			DRILLER: Isaac Young	
DATE COMPLETE:				RIG TYPE: T-300 Rotosonic	
INSTALLED BY: SUPERVISED BY:	Cascade Rvan Walker			METHOD: Rotosonic	
WATER 1ST ENCO	UNTERED:		'BGS		
WATER AFTER 48 Elevation	Depth	34.4	I4' BGS		Core Photos
MSL	BGS				
	28				
	20				
	29			29-39' Hard rock-metamorphic gneiss banding/schist, fractured core	
	30			iron staining (4.5/10)	
					-
	31				
	32				-
	33				
	34				
	35				
	36				
	37		,		
	38	ľ			
	30				
	39			39-49' As above (4.5/10)	
	40				
					4
	41				100
	42				N Basella
	43				
	44				1
	45				1
	46				
	47				7 3500
	48				
	49			49-59' As above-less fractured rock (8/10)	100
				10 00 710 42000 1000 114044104 10011 (0,110)	The same of the sa
	50				
	51				The same of
	52				
	02				
MATERIALS:		6.00"	OD		
		1	Danta da Casad		
GROUT: MANUFACTURI	ER:		Bentonite Grout AquaGuard		
DENTONITE CE			2/0" Pontonito Polleto		
BENTONITE SE MANUFACTURI			3/8" Bentonite Pellets Pel-Plug		
FILTER PACK S	SAND:		20/30 Mesh		
MANUFACTUR			Filter Media		
WELL SCREEN	. =		Sch. 40 - 2" PVC		
MANUFACTURI			Silver-Line		
SLOT SIZE:			0.010-Inch Slot	TOC - Top of Casing	
WELL CASING:			Sch. 40 - 2" PVC	ID - Inside Diameter; OD - Outside Diameter MSL - Mean Sea Level	
MANUFACTURI			Silver-Line	BGS - Below Ground Surface	4

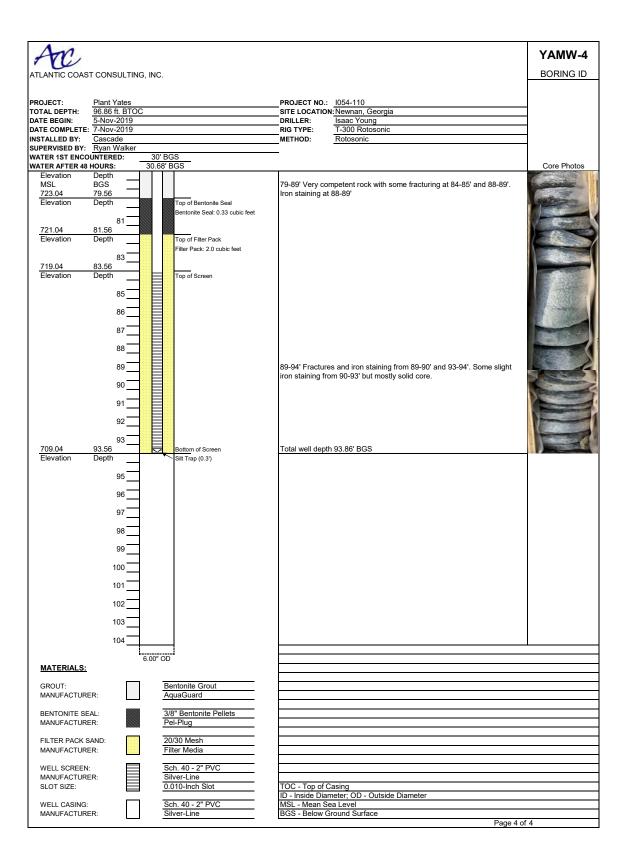
ACC					YAMW-3
ATLANTIC COAS	T CONSULTING	G, INC).		BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 91.96 ft. BTO	С		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	5-Nov-2019			DRILLER: Isaac Young	
DATE COMPLETE: INSTALLED BY:	6-Nov-2019 Cascade			RIG TYPE: T-300 Rotosonic	_
SUPERVISED BY:	Ryan Walker			METIOD: INDICATION	-
WATER 1ST ENCO WATER AFTER 48			' BGS 4' BGS		Core Photos
Elevation	Depth	J4.4	1 1		Core i notos
MSL	BGS				
	54				-
	55				
	56				
	57				
	58				
	59	-		59-69' As above, large rock pieces/less fractures No iron staining (5/10)	A Real Property of the Party of
	60			ite non-standing (cr. to)	197
	61				
	62				VI-S
	63				
	64				
	_				
	65				
	66				
	67				
	68				
	69			69-79' (10/10) 72-74' more fractured than previous	
	70			74-79' lots of iron staining. Some quartzite.	Contract of the Contract of th
	71				V
	70				
	72				1
	73				
	74				
718.61 Elevation	74.59 Depth		Top of Bentonite Seal		A Comment
Lievation			Bentonite Seal: 0.33 cubic feet		-1)
716.61	76 76.59				
Elevation	Depth		Top of Filter Pack		7
	78		Filter Pack: 2.0 cubic feet		
MATERIALS:		6.00"	OD		
GROUT:		1	Bentonite Grout		
MANUFACTURE	ER:		AquaGuard		
BENTONITE SE	AL:		3/8" Bentonite Pellets		
MANUFACTURE			Pel-Plug		
FILTER PACK S	SAND:		20/30 Mesh		
MANUFACTURE			Filter Media		
WELL SCREEN:	:		Sch. 40 - 2" PVC		
MANUFACTURE SLOT SIZE:			Silver-Line	TOC. Top of Cooling	
		1 -	0.010-Inch Slot	TOC - Top of Casing ID - Inside Diameter; OD - Outside Diameter	
WELL CASING: MANUFACTURE	-R·		Sch. 40 - 2" PVC Silver-Line	MSL - Mean Sea Level BGS - Below Ground Surface	
IVIAINOFACTURE	-1%	1	OII 7 GI-LII IG	Page 3 o	f 4

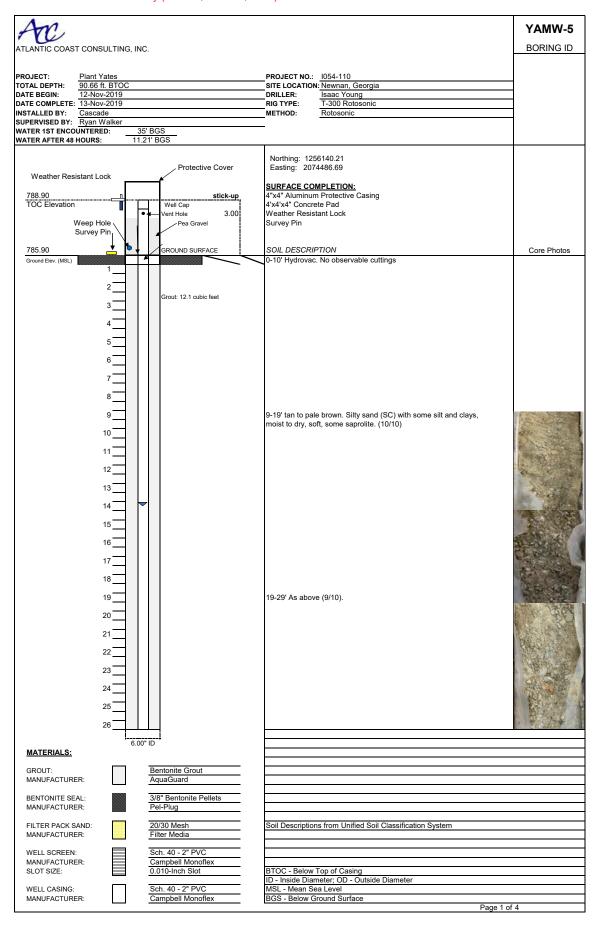
AC				YAMW-3
ATLANTIC COAS	T CONSULTING, IN	NC.		BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 91.96 ft. BTOC		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	5-Nov-2019		DRILLER: Isaac Young	1
DATE COMPLETE:			RIG TYPE: T-300 Rotosonic METHOD: Rotosonic	
INSTALLED BY: SUPERVISED BY:	Cascade Ryan Walker		METHOD: Rotosonic	-
WATER 1ST ENCO WATER AFTER 48		30' BGS 4.44' BGS		Core Photos
714.61	78.59	Top of Screen	79-88' As above. Iron staining stops at ~81'	Core Priotos
Elevation	Depth			
	80			发展报
	81			
	01			
	82			
	83			(BEELE STATES
	84			100
	04			
	85			
	86			
	97			
	87			
704.61	88.59	0.41	Total well depth 88.89' BGS	
Elevation	Depth	Bottom of Screen Silt Trap (0.3')	Total well deptit 66.69 BGS	
	90			
	90			
	91			
	92			
	93			
	94			
	95			
	96			
	97			
	98			
	99			
	100			
	101			
	102			
	103			
	104			
		001 OD		•
MATERIALS:	6.0	00" OD		
GROUT:		Pontonita Crout		
MANUFACTURE	ER:	Bentonite Grout AquaGuard		
BENTONITE SE	:AI ·	3/8" Bentonite Pellets		
MANUFACTURE		Pel-Plug		
FILTER PACK S	SAND:	20/30 Mesh		
MANUFACTURE		Filter Media		
WELL SCREEN	: =	Sch. 40 - 2" PVC		
MANUFACTURE		Silver-Line		
SLOT SIZE:		0.010-Inch Slot	TOC - Top of Casing ID - Inside Diameter; OD - Outside Diameter	
WELL CASING:		Sch. 40 - 2" PVC	MSL - Mean Sea Level	
MANUFACTURE	ER:	Silver-Line	BGS - Below Ground Surface Page 4 of	4



AC					YAMW-4
ATLANTIC COAS	T CONSULTING	, INC.			BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 96.86 ft. BTOC	:		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	5-Nov-2019			DRILLER: Isaac Young	
DATE COMPLETE: INSTALLED BY:	7-Nov-2019 Cascade			RIG TYPE: T-300 Rotosonic METHOD: Rotosonic	
SUPERVISED BY:	Ryan Walker			NCIOSONIC TODOS	
WATER 1ST ENCO WATER AFTER 48			BGS 8' BGS		Core Photos
Elevation	Depth				
MSL	32 33 34 35 36 37 37 37	•		30-35' Mostly sand silt with 10% gravel-hard to friable 35-37' Clayey sand-high to medium plasticity 37-40' Return to silty sand with medium to highly weathered rock. Micaceous	
	38			40-44' As above (5/5)	
	45 46 47 48			45-49' Hard rock. Metamorphic gneiss banding/schist. Iron staining present. 44-45' Moderately hard, somewhat friable. (5/5)	
	49 50 51 52			49-59' As above, moderately weathered, somewhat friable, iron staining present (2/10)	
MATERIALS:	·	6.00" (OD		
			Denterite C. :		
GROUT: MANUFACTURE	ER:		Bentonite Grout AquaGuard		
BENTONITE SE	:A1 :		3/8" Bentonite Pellets		
MANUFACTURE			Pel-Plug		
FILTER PACK S	SAND:		20/30 Mesh		
MANUFACTURE			Filter Media		
WELL SCREEN MANUFACTURE			Sch. 40 - 2" PVC Silver-Line		
SLOT SIZE:			0.010-Inch Slot	TOC - Top of Casing	
WELL CASING:			Sch. 40 - 2" PVC	ID - Inside Diameter; OD - Outside Diameter MSL - Mean Sea Level	
MANUFACTURE			Silver-Line	BGS - Below Ground Surface	
			·	Page 2 of 4	1

Arc			YAMW-4
ATLANTIC COAST CONSULTING, II	NC.		BORING ID
PROJECT: Plant Yates TOTAL DEPTH: 96.86 ft. BTOC		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN: 5-Nov-2019		DRILLER: Isaac Young	
DATE COMPLETE: 7-Nov-2019 INSTALLED BY: Cascade		RIG TYPE: T-300 Rotosonic METHOD: Rotosonic	
SUPERVISED BY: Ryan Walker WATER 1ST ENCOUNTERED:	30' BGS	_	
WATER AFTER 48 HOURS: 30	0.68' BGS		Core Photos
Elevation Depth MSL S4 55 56 56 57 56 56 57 58 59 60 61 62 63 64 65 66 66 67 68 68 68 59		59-69' As above except less weathered rock, non friable. Still some iron staining present (8/10)	
69		69-79' As aboveexcept rock is more weathered. Section of quartzite at 73-74'. Iron staining ends at 77'. (7/10)	
6.0 MATERIALS:	0" OD		
	Dantanita Cassi		
GROUT: MANUFACTURER:	Bentonite Grout AquaGuard		
BENTONITE SEAL: MANUFACTURER:	3/8" Bentonite Pellets Pel-Plug		
FILTER PACK SAND: MANUFACTURER:	20/30 Mesh Filter Media		
WELL SCREEN: MANUFACTURER:	Sch. 40 - 2" PVC Silver-Line		
SLOT SIZE:	0.010-Inch Slot	TOC - Top of Casing	
WELL CASING:	Sch. 40 - 2" PVC	ID - Inside Diameter, OD - Outside Diameter MSL - Mean Sea Level	
MANUFACTURER:	Silver-Line	BGS - Below Ground Surface Page 3 of	4





Arc				YAMW-5
ATLANTIC COAS	T CONSULTING, II	NC.		BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 90.66 ft. BTOC		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	12-Nov-2019		DRILLER: Isaac Young	
DATE COMPLETE: INSTALLED BY:	Cascade		RIG TYPE: T-300 Rotosonic METHOD: Rotosonic	-
SUPERVISED BY:	Ryan Walker	OF LDOO		
WATER 1ST ENCO WATER AFTER 48		35' BGS 1.21' BGS		Core Photos
Elevation MSL	Depth BGS 28 29 30 31 31 32 33 34 34 34		29-39' Harder drilling. Gniessic banding with schist. Iron and manganese staining. Some fractures present.	
	35		39-49' As above except no iron staining (7/10)	
	46 47 48 49 50 51 52 600	00° OD	49-59' As above (8/10)	
MATERIALS:	0.0			
GROUT:		Bentonite Grout		
MANUFACTURE		AquaGuard		
BENTONITE SE MANUFACTURE		3/8" Bentonite Pellets Pel-Plug		
FILTER PACK S MANUFACTURE		20/30 Mesh Filter Media		
WELL SCREEN		Sch. 40 - 2" PVC		
MANUFACTURE SLOT SIZE:	ER:	Silver-Line 0.010-Inch Slot	TOC - Top of Casing	
			ID - Inside Diameter; OD - Outside Diameter	
WELL CASING: MANUFACTURE		Sch. 40 - 2" PVC Silver-Line	MSL - Mean Sea Level BGS - Below Ground Surface Page 2 of	4
			Fage 2 01	

AC)	YAMW-5
ATLANTIC COAST CONSULTING, INC.	BORING ID
PROJECT: Plant Yates PROJECT NO.: 1054-110 TOTAL DEPTH: 90.66 ft. BTOC SITE LOCATION: Newnan, Georgia	
DATE BEGIN: 12-Nov-2019 DRILLER: Isaac Young	
DATE COMPLETE: 13-Nov-2019 RIG TYPE: T-300 Rotosonic INSTALLED BY: Cascade METHOD: Rotosonic	
SUPERVISED BY: Ryan Walker	
WATER 1ST ENCOUNTERED: 35' BGS WATER AFTER 48 HOURS: 11.21' BGS	Core Photos
Elevation Depth MSL BGS	
54	
55	
56	
57	
58	
59—59 As above except more competent rock. Iron staining at fractures present (6/10)	nd A
60	Mary 1
62	
63	
64 🗔 📗	
65	
66	
67	
69 69-79' As above except no iron staining (9/10)	
70	
71	
72 7000	
712.56 73.34 Top of Bentonite Seal Bentonite Seal Bentonite Seal 0.33 cubic feet	
710.56 75.34	201
Elevation Depth Top of Filter Pack Filter Pack: 2.0 cubic feet	
708.56 77.34 Elevation Depth Top of Screen	
78	
6.00" OD MATERIALS:	
GROUT: Bentonite Grout MANUFACTURER: AquaGuard	
MANUFACTURER: AquaGuard BENTONITE SEAL: 3/8" Bentonite Pellets	
MANUFACTURER: Pel-Plug	
FILTER PACK SAND: 20/30 Mesh MANUFACTURER: Filter Media	
WELL SCREEN: Sch. 40 - 2" PVC MANUFACTURER: Silver-Line	
SLOT SIZE: 0.010-lnch Slot TOC - Top of Casing ID - Inside Diameter; OD - Outside Diameter	
WELL CASING: Sch. 40 - 2" PVC MSL - Mean Sea Level MANUFACTURER: Silver-Line BGS - Below Ground Surface	Page 3 of 4

An				YAMW-5
ATLANTIC COAS	T CONSULTING, IN	C.		BORING ID
PROJECT: TOTAL DEPTH:	Plant Yates 90.66 ft. BTOC		PROJECT NO.: 1054-110 SITE LOCATION: Newnan, Georgia	
DATE BEGIN:	12-Nov-2019		DRILLER: Isaac Young	
DATE COMPLETE: INSTALLED BY:	13-Nov-2019 Cascade		RIG TYPE: T-300 Rotosonic METHOD: Rotosonic	
SUPERVISED BY:	Ryan Walker		- Notosomo	
WATER 1ST ENCO WATER AFTER 48		.21' BGS .21' BGS		Core Photos
Elevation MSL	Depth BGS 80 81 81 81		79-87' As above except rock slightly weathered. Iron staining and some fractures present (6/10)	
	82 83 84 85 85			直
698.56	87.34	Bottom of Screen	Total well depth 87.64' BGS	
Elevation	Depth 88	Silt Trap (0.3')		
	102 103 104			
MATERIALC	6.00)" OD		
MATERIALS:				
GROUT: MANUFACTURE	ER:	Bentonite Grout AquaGuard		
BENTONITE SE MANUFACTURE		3/8" Bentonite Pellets Pel-Plug		
FILTER PACK S MANUFACTURE		20/30 Mesh Filter Media		
WELL SCREEN MANUFACTURE SLOT SIZE:		Sch. 40 - 2" PVC Silver-Line 0.010-Inch Slot	TOC - Top of Casing TO - Top of Casing TO - Top of Casing	
WELL CASING: MANUFACTURE		Sch. 40 - 2" PVC Silver-Line	ID - Inside Diameter; OD - Outside Diameter MSL - Mean Sea Level BGS - Below Ground Surface Page 4 of	4

APPENDIX B GEOLOGIC MAPPING AND LINEAMENT ANALYSIS

October 13, 2017 1659442, 1772632

Mr. Joju Abraham Southern Company Services, Inc. 241 Ralph McGill Blvd NE Atlanta, GA 30308 jabraham@southernco.com (404) 506-7239

RE: GEOLOGIC MAPPING AND LINEAMENT ANALYSIS
GEORGIA POWER PLANT YATES NEWNAN, GEORGIA

Dear Joju:

We have enclosed our Geologic Mapping and Lineament Analysis for Plant Yates. Golder appreciates the opportunity to work with SCS on this project. Should you require additional information related to this lineament analysis, please do not hesitate to contact us at (770) 496-1893.

Sincerely,

GOLDER ASSOCIATES INC.

Timothy I. Richards, PG Associate and Senior Environmental Consultant

Enclosure: Geologic Mapping and Lineament Analysis

Rachel P. Kirkman, PG Associate and Senior Consultant

Radul & Midwer

GEOLOGIC MAPPING AND LINEAMENT ANALYSIS

GEORGIA POWER - PLANT YATES
COWETA COUNTY, GEORGIA

Submitted By: Golder Associates Inc. 3730 Chamblee Tucker Rd. Atlanta, Georgia 30341

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Appendix A Tabulated DEM lineation orientations

1.0 INTRODUCTION

Georgia Power Company operates a natural gas-fired station at Plant Yates approximately 7 miles northwest of Newnan, Georgia (GA) Coweta County. The plant property encompasses approximately 2,400 acres, bounded by the Chattahoochee River to the north and west, Highway16 to the east, and residential property along Wagers Mill Road to the south.

The plant currently operates two natural gas-fired units (units 6 and 7) to produce electricity. It is our understanding that Georgia Power has retired the five coal-fired units (units 1 through 5) and will continue to operate this station as a natural gas-fired plant. Plant Yates has seven coal combustion residual (CCR) ash ponds and one permitted "dry stacking" landfill as identified in the RFP. Ash Pond 1 is dry and uncovered; Ash Ponds 2 and 3 are wet ponds; and Ash Ponds A, B, and C are dry and covered, with C partially underneath the R6 Landfill. The B Ash Pond is currently being used to dewater wet ash before being landfilled at R6.

Golder Associates Inc. (Golder) was retained by Southern Company Services (SCS) to perform an evaluation of the geologic conditions in and around Plant Yates. For this evaluation, a review of limited site-specific information was conducted, a desktop study was conducted to review existing information on geologic conditions within the vicinity of Plant Yates; a lineament analysis was performed to identify linear features that may control groundwater flow within the bedrock; and detailed geologic mapping was conducted by Golder's subcontractor Petrologic Solutions, Inc. (Petrologic). Information collected during this investigation can be used by SCS to provide a framework for development a site conceptual groundwater model.

Information that we relied upon for the desk study, lineament analysis and geologic mapping includes:

- 1. Limited review during geologic mapping of available rock core collected by SCS on Plant property
- 2. Boring logs for PZ-35 through PZ-49 from Golder's 2017 Piezometer Installation Report
- 3. Publically Available Information
 - Whitesburg, GA USGS 7.5 minute topographic quadrangle
 - 10-m Digital Elevation Model taken from the USGS Whitesburg and Newnan North, Georgia, 7.5 minute quadrangles.
 - McConnell, K. and Abrams, C., 1984. Geology of the Greater Atlanta Region, Georgia: Geological Survey Bulletin 96.
 - Redwine, J., Tinsely, R., Patel, D., and Bloomberg, D., 1991. Geology, Hydrogeology, and Ground-Water Chemistry of the Plant Yates Gypsum Stacking Area, Coweta County, Georgia: Proceedings of the 1991 Ground Water Resources Conference March 19-21, 1991, pp 326-330.

 Steltenpohl, M., Kish, S., and Neilson, M., 1990. Geology of the Southern Inner Piedmont, Alabama and Southwest Georgia: Guidebook for Field Trip VII April 7-8, 1990.

2.0 BACKGROUND INFORMATION

2.1 General

Information presented in this section is based on published literature, discussion with local geologic experts, and experience working in this geologic terrain. This section is presented to provide an overview of regional geology in the area. A detailed discussion of large-scale, site-specific geology is presented in Section 3.0.

2.2 Physiography

Plant Yates is located within the Piedmont Physiographic Province of western GA, which is characterized by gently rolling hills and narrow valleys, with locally pronounced linear ridges. Topographic relief within the vicinity of the site is greater than 150 feet, with topographic highs of over 860 feet above mean sea level (ft. MSL) occurring on small hilltops, and topographic lows of less than 700 ft. MSL near the Chattahoochee River. The site has generally been constructed in a valley with several north-south to northwest-trending tributaries to the Chattahoochee River, as shown on Figure 1.

2.3 Regional Geologic Setting

Plant Yates is located within the northeast quarter of the Whitesburg, GA United States Geological Survey (USGS) 7.5-minute topographic quadrangle. The Piedmont/Blue Ridge geologic province contains some of the oldest rocks in the Southeastern United States. Since their origin, approximately 276 to 1,100 million years ago (Ma), these late Precambrian (Neoproterozoic) to late Paleozoic (Permian) rocks have undergone repeated cycles of igneous intrusions and extrusions, metamorphism, folding, faulting, shearing, and silicification. The latest regional metamorphism and associated deformation has been attributed to the collision of the North America plate with the Eurasian plate approximately 200 to 230 Ma. More recent deformation and emplacement of mafic dikes is associated with the rifting of the North American craton during the Mesozoic and Cenozoic Eras.

The metamorphic and igneous rocks that underlie the area have been subjected to physical and chemical weathering which has created a landscape dissected by creeks and streams forming a dendritic drainage pattern. These rocks are deeply weathered due to the humid climate and bedrock is typically overlain by a variably thick blanket of residual soils and saprolite. The overall depth of weathering in the Piedmont/Blue Ridge is generally about 20 to 60 feet; however, the depth of weathering along discontinuities and/or very feldspathic rock units may extend to depths greater than 100 feet. Because of such variations in rock types and structure, the depth of weathering can vary significantly over short horizontal distances. Locally, significant accumulations of alluvial soils are also present in the area. Sand and gravel alluvial

terraces related to changing stream base grade elevations occur above residual soils in areas near larger rivers.

2.4 Regional Stratigraphy and Structure

Two major structural/stratigraphic packages occur within the Piedmont/Blue Ridge around the site. These packages are separated by The Katy Creek Fault, a regional fault that occurs northwest of the Plant property and traverses northeast-southwest through this area. This Fault is a thrust fault, dipping at a lower angle than the strike-slip faults present in the region. This thrust fault developed at a relatively shallower depth in the crust than the strike-slip faults, resulting in less-pronounced ductile structural fabrics. Uplift associated with building of the Appalachian Mountains and subsequent erosion has allowed modern exposure of these structural features.

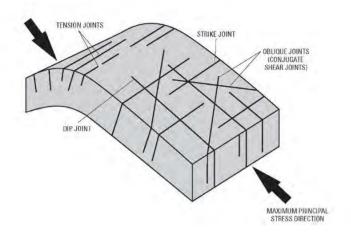
The Katy Creek Fault forms boundary between a regional zone of deformation, referred to as the Brevard Zone, and the Dadeville Complex. Lithologic contacts and major structural features in the Brevard Zone generally trend northeast-southwest. In addition to strike-slip and thrust faults, structural features within this shear zone consist of northwest-verging, doubly-plunging, overturned folds that have been overprinted by a shear-induced foliation.

The Brevard Zone includes fine-grained and porphyroblastic schists, button schists, phyllonitic schists, biotite schists, and schists locally interlayered with amphibolite/hornblende gneiss, and ultamafic bodies; mylonites, ultramylonites, and flinty crush rock; metagraywacke and feldspathic quartzite; and granitic gneiss. Rocks within these various lithologic units have been intensely deformed, sheared, chemically altered, silicified and are generally repeated because of movement along faults both within and outside of the Brevard Zone.

The Dadeville Complex occurs southeast of the Brevard Zone and is considered to represent an Ordovician-age Island Arc. Rocks within this complex are generally more mafic near the Brevard Zone, becoming more felsic in areas further southeast of the Brevard. The mafic portion of the Dadeville Complex primarily consists of biotite gneiss and thick, mappable layers of amphibolite/hornblende gneiss. The biotite gneiss is also interlayered with thin, discontinuous layers, lenses and pods of amphibolite. The continuous and discontinuous amphibolite layers and lenses weather more deeply but less uniformly than the surrounding biotite gneiss in this area. These mafic units are observed northwest of Plant Yates. Units underlying that area around Plant Yates and further southeast are more felsic, being comprised of a mixture of granitic intrusives, migamitic and biotite gneiss, and alumnosilicate schists.

Typically up to four different joint sets formed in this area due to tectonic stresses imposed upon the bedrock. Dip joints form parallel to the regional dip direction of foliation/compositional layering and are typically perpendicular to fold limbs, representing extension perpendicular to the maximum principal

stress direction or direction of compression. These joints are commonly near vertical. Strike joints develop parallel to the strike of foliation/compositional layering and fold limbs, typically forming from tension during relaxation of the maximum principal stress. The dip direction and angle of these joints is orthogonal to the dip direction and angle of compositional layering. Oblique joints develop diagonal (± 30°) to the principal stress direction and represent conjugate sets formed from shear along the intermediate principal stress.



Schematic diagram showing the typical joint patterns

3.0 SITE GEOLOGIC CONDITIONS

3.1 Geologic Mapping Methodology

Detailed geologic mapping was performed by Petrologic within and around the site within the more felsic portion of the Dadevile Complex, using the Whitesburg, GA USGS 7.5-minute topographic quadrangle as a base map. Mafic lithologies of the Dadeville that occur further northwest of the site, labeled OZa, OZssbg and Ozgg on Figure 2, are not considered to influence groundwater conditions around Plant Yates, and therefore are not discussed in this report.

Figure 2 presents interpretation of structural and lithologic features encountered during mapping of the area, Information recorded at each map station included: lithology and mineralogy; orientation and characteristics of structural discontinuities including, shearing, faulting, jointing, cleavage, and compositional layering; and depth and type of weathering characteristics of the rock. Bedrock discontinuity orientations were statistically analyzed by Petrologic using lower hemisphere equal-area stereonets, presented as Figure 3, to determine dominant orientations for each discontinuity type (i.e., faults, joints, and foliation). Map station locations were recorded using a hand-held, Wide Area Augmentation System (WAAS)-enabled Global Positioning System (GPS).

3.2 Alluvial and Residual Soils

Limited boring logs were available for review based on Golder's June and July 2017 piezometer installation. A review of boring logs indicates that surficial soils are composed of a mixture of sands, silts, and clays.

Also, both alluvial and residual soils were observed at ground surface around the site. Alluvial soils primarily consisted of courser material that may be related to a former Chattahoochee River terrace, consisting of silty-sand, clayey silt, and silty clay with well-rounded gravel and cobbles. The terrace was observed primarily within point bars of the Chattahoochee with up to 20 feet of relief. Overbank, floodplain-type deposits are anticipated to underlie the Plant, as well as former river channels related to previous migration of the Chattahoochee.

Residual soils were also observed at ground surface. These soils were derived from physical and chemical weathering of the underlying bedrock, and consist primarily of silty sand, sandy silt, and silty clay, with local gravel-sized angular fragments of weathered bedrock. A variably-thick blanket of residual soils is anticipated to overlie bedrock across most of the site.

Based on the detailed geologic mapping, rock types present at the site includes granitic and migmatitic gneiss, biotite gneiss and amphibolite, all of which have highly variably mineralogy, texture and chemistry, and a muscovite schist that serves as a structural marker horizon. Residual soils developed from weathering of these rock types may have variable geochemical characteristics. These descriptions are consistent with bedrock descriptions from Golder's June-July 2017 piezometer installation report, which indicate the presence of biotite gneiss, granitic gneiss, and amphibolite beneath the site.

3.3 Lithologic Units.

During geologic mapping, distinct lithologic units were identified to underlie the site. A brief description of these lithologies is presented below and the aerial distribution of each unit is shown on Figure 2.

<u>Massive Granite (OZgr)</u>: Massive, weakly foliated, poorly jointed, fine-to medium-grained granite. Large, relatively unweathered, exfoliation boulders are commonly observed within the residual and saprolitic soils. Weathering of this granite may yield Uranium and daughter products (e.g., Radium 226/228); however, this unit is separated from the site by the Chattahoochee River. Consequently it is unlikely that this unit will impact groundwater quality on site.

Migmatitic Gneiss (OZmgn): Highly contorted, well layered, well foliated, poorly jointed, medium-grained muscovite-biotite-quartz-feldspar migmaticite gneiss. Granite is locally interlayered with biotite gneiss and pods/lenses of ultramafic bodies, which occur as relatively fresh, well foliated, unjointed boulders of medium- to course-grained actinolite-chlorite schist. A relatively large body of granite occurs within the migmatitic gneiss southwest of the site. Overall, this unit does not weather deeply, but exhibits differential weathering where the magmatic gneiss and schist are relatively fresh compared to the biotite gneiss. Most of the biotite gneiss occurs as unmappable layers within the magmatic gneiss; however, one mappable lens of biotite gneiss was observed north of the Chattahoochee River (labeled OZog on

Figure 2). Weathering of ultramafic bodies could produce naturally elevated metals in groundwater, this unit occurs north of the Chattahoochee but also underlies the property and may potentially impact groundwater quality on site.

Feldspathic Gneiss (OZggn): Variably well foliated to weakly foliated, slightly sheared, weakly jointed, fine- to medium-grained micaceous, feldspathic granitic gneiss. Due to the high feldspar content, this unit weathers more deeply than others observed on site, as indicated by the formation of a natural valley in areas where this unit occurs.

<u>Porphyroblasite Schist (OZgss):</u> Well foliated, crenulated, highly sheared, poorly jointed, medium- to course-grained, silliminate-quartz-muscovite schist with porphyroblastic garnet and staurolite. This unit is very distinctive and is used as a marker horizon to delineate structure on site, discussed further in Section 3.4. Although this unit is typically fresh, weathering of the porphyroblastic garnets and staurolite may promote naturally enriched Fe, Mn, Ca, and Zn in groundwater.

<u>Biotite Gneiss (OZog):</u> Well layered, well foliated, moderately jointed, fine- to medium-grained, biotite-uartz-feldspar gneiss. This unit weathers fairly deeply relative to adjacent lithologies, forming a distinctive dark red, vermiculitic soil from weathering of biotite. The gneiss locally contains thin lenses of chlorite-actinolite schist and feldspar-hornblende gneiss/amphibolite, increasing in concentration of these ultramafic and mafic bodies to the southeast.

<u>Amohibolite (OZa):</u> Thinly laminated, moderately jointed, fine- to medium-grained hornblende-plagioclase gneiss/amphibolite with interlayered chlorite-actinolite schist. Weathering of this unit may promote naturally enriched Fe, Mn, Mg, and Ca in groundwater.

In general, the lithologic units observed to underlie the site do not weather deeply and do not have well-developed secondary permeability; consequently, preferential pathways for groundwater flow are likely not enhanced, significantly limiting the potential for groundwater flow in the bedrock aguifer systems.

3.4 Geologic Structure

Based on detailed geologic mapping by Petrologic, indication of folding and faulting has been observed at and near the site. As shown on Figure 2, the migmatitic gneiss occurs both north and south of the Chattahoochee River. This unit is truncated northwest and southwest of the site by an unnamed, strike-slip fault, referred to in this report as the Yates Fault. This fault occurs subparallel to the Chattahoochee River near the site but abruptly turns south in the southwest corner of the site. A relatively hard knot of migmatitic granite occurs in the gneiss where the fault deflects south. There is likely to be a rheologic contrast between the migmatitic gneiss and the mass of granite, with the granite being much harder than the surrounding gneiss. This hard mass likely forced the fault to deflect around the mass in this area. Rocks that were being transported in the southeastern block of the fault appear to have been "piled" up, creating an upright, slightly overturned, west verging antiformal structure, as shown on Figure 2.

The core of this fold in comprised of feldspathic gneiss, which weathers more deeply than other lithologic units observed on site. Rimming this unit is the porphyroblastic schist, which serves as a distinctive marker horizon for delineating structure on site. Lithologies related to the OZa Amphibolite unit, which is

observed in the mafic portion of the Dadeville Complex are also present adjacent to the fault on the southeastern block.

Foliation measured during geologic mapping near the site reflects the dominantly north-south trending structures in this area, with an average foliation of N9E, 38SE, as graphically shown on the equal-area, lower hemisphere stereonet and rose diagram presented as Figure 3 (prepared by Petrologic). Mapping further northwest of the fault reflects the more regional orientation of foliation characteristic of the Brevard Zone, which is generally oriented about N40-45E.

Because the evaluation of joints is visual and judgmental, an effort is made for consistency in describing the relative frequency of occurrence using the following designations: Abundant (A); Common (C); and Scarce (S). These designations are relative to one another but are used consistently in descriptions made throughout the study area. An effort is made to record all of the different joint sets and, if an exposure is large, several same (or similar) joints may be recorded at the same Map Station. This deliberate method of visual evaluation in the field is more scientifically relevant and efficient than saturation-measurement of joints.

Four major joint sets were recorded during the detailed geologic mapping. Equal-area stereonet analysis of all joints measured in all lithologies is presented in Figure 4.

The four major joint sets are (quadrant and azimuth, right hand rule):

- J1: N48E 61NW (228/61) regional strike joint
- J2: N50E 72SE (050/72) regional strike joint
- J3: N44W 72SW (136/72) regional dip joint
- J4: N3W 87SW (177/87) local strike joint

Joint sets 1 and 2 are oriented subparalell to regional foliation, the dip of which varies in response to the dip of foliation. These joints are referred to above as regional strike joints. Joint set 3 is oriented perpendicular to regional foliation and is referred to as a regional dip joint. Joint 4 appears to have formed locally in response to the north-south trending structures mapped on the site and southwest of the site. This joint set is referred to as the local strike joint.

3.5 Lineament Analysis

3.5.1 Methodology

Preferential weathering of subsurface geologic discontinuities such as lithologic contacts between resistant or non-resistant units, fracture zones, jointing, shear planes, and faults often results in ground surface expressions that can be identified through analysis of photographic and topographic images. These topographic expressions, which are referred to as lineaments, commonly have enhanced porosity and permeability in the rock mass due to differential weathering. Ground water in igneous and

metamorphic rocks generally moves along discontinuities in the bedrock, enhancing the differential weathering processes.

Many lineaments observed on the small scale imagery or maps are related to fence, property, and section lines. However, many lineaments are related to local and regional geologic anomalies. Because discontinuity zones are typically less resistant to weathering, they are often expressed as natural topographic lows, such as straight stream valley segments, swales, aligned depressions and gaps in ridges or as linear tonal or vegetative alignments due to variations in soil thickness and moisture (see inset). Faults tend to be long linear features that are often difficult to detect at ground surface, but generally form photographic and topographic lineaments.

Inset - Block diagram shows how lineament/fracture trace is a surface manifestation of an underlying bedrock fracture zone.

Lineaments were identified for this project using shaded relief maps generated from USGS 10-meter digital elevation model (DEM) data, presented as Figure 5. Use of DEM as a base has an advantage over imagery and topographic maps in that the

horizontal and vertical light (sun position) can be adjusted to enhance the visibility of a lineament. Additionally, other lineaments may be well defined on the different image maps. Lineaments identified using the different sun angles are graphically represented on the rose diagrams presented as Figure 6. The center rose diagram on this figure is a composite of lineaments identified using all sun angles. Once lineaments have been identified on the DEMs, they can be correlated with discontinuities identified during geologic mapping.

3.5.2 Discussion of Lineaments

As discussed above, orientations and relative persistence of lineaments identified on the DEM are illustrated on Figure 5 and summarized in rose diagrams presented on Figure 6. As shown on the summary rose diagram, a total of 297 lineaments were identified on the DEM viewed with light source from various angles (i.e., NW, NE, SE, and SW as shown on the individual rose diagrams). A total of two major lineaments and two minor lineaments were identified:

L1: N30-50E

L2: N10W–N10E

L3: N20-40W

L4: N70-80W

Major lineaments L1 and L2 were identified using every light source angle; although not as dominant, L3 was also identified on every light source. Lineament L4 was observed as a minor lineament on only the southwest light source (lower left rose diagram). DEM lineation orientations are tabulated in Appendix A.

3.5.3 Discontinuity Mapping and Lineament Analysis Correlation

Structural weaknesses in rocks are reflected by the fractures formed, which subsequently can be weathered to form lineaments. These fractures are caused by application of directional stresses on the rock body. Generally, the stress is due to regional tectonics and/or unloading due to weathering and erosion but can also be influenced by local structure.

The project area is characterized by four lineament sets whose orientations are consistent with the structural stresses experienced in this area (as indicated by foliation and joint orientation). L1 is subparallel to regional foliation and the measured orientations of J1 and J2, which are considered to be strike joints related to the regional foliation. These features likely developed in response to Brevard Zone shearing. L3 is subparallel to J3 and is considered to represent dip joints formed in response to regional foliation. L2 is subparallel to local foliation and J4, which are related to structures that occur near the site (Yate Fault and antiformal fold axis). Although a dip joint related to these local structures was not observed during geologic mapping, the orientation of lineament L4 is consistent with the orientation that would be predicted for a dip joint forming in response to these structures. Because lineament orientations identified during this analysis correlate with known regional tectonic fabrics and local structures, it is likely that most are true manifestations of subsurface fracture zones or low-resistance stratigraphic layers within the rock formations underlying the site.

4.0 DISCUSSION

Groundwater likely occurs within overburden residual and alluvial soils beneath the site. The alluvial soils are locally present adjacent to the Chattahoochee River and may be present near an unnamed tributary that flows through the site. These soils will have greater permeability relative to adjacent residual soils and will likely be the preferred pathway for groundwater flow beneath the site. The hydrochemistry of alluvial soils may differ from that of the residual soils.

Different lithologic units will develop residual soils of variable geochemistry; consequently, geochemistry of groundwater flowing through residual soils may be locally influenced by naturally occurring elements and metals in the groundwater. These soils are expected to be fairly fine-grained in nature, creating lower hydraulic conductivity characteristics. Consequently, groundwater is expected to flow through the overburden, more slowly through residual soils, with preferential flow paths through alluvial soils where present, ultimately discharging into the Chattahoochee River.

Groundwater may also occur in a series of discreet bedrock aquifer systems beneath the site. Flow characteristics related to the different geologic units on site are largely anticipated to be consistent. Geologic units OZa, OZggn, OZmgn, and OZgss are all generally massive and weakly jointed and anticipated to have low water-bearing potential. Groundwater flow potential is relatively greater in areas of deeper weathering associated with discontinuities, particularly in the OZggn and OZmgn units. The OZog geologic unit observed on site may have preferred groundwater flow relative to other units on site due to differential weathering of amphobilite lenses within the well-jointed gneiss, shown to rim the antiformal structure on Figure 2.

Naturally occurring uranium and daughter products (e.g., ²²⁶Ra and ²²⁸Ra) may be associated with granite in the OZgr unit. The distance between the mapped occurrence of this unit and the site is significant, however, and therefore considered unlikely to influence groundwater chemistry upgradient of the site. Naturally occurring metals may be associated with the ultramafic bodies present in the OZmgn unit. This unit occurs southeast of the Yates Fault and is likely upgradient of many of the CCR units at the site. Naturally elevated concentrations of Fe, Mn, Ca, and Zn (associated with porphyroblastic garnet and staurolite) may occur in groundwater flowing through the OZgss unit; and naturally elevated concentrations of Fe, Mg, Mn, and Ca may occur in groundwater flowing thought the OZa (amphibolite) unit.

PLANT YATES GEOLOGIC MAPPING AND LINEAMENT ANALYSIS OCTOBER 2017

	INDEX OF FIGURES
FIGURE	DESCRIPTION
0	COVER SHEET
1	SITE LOCATION MAP
2	GEOLOGIC MAP
3	DISCONTINUITY DATA FROM GEOLOGIC MAPPING
4	COMPARISON OF MEASURED DISCONTINUITIES AND LINEAMENTS
5	REMOTE SENSING LINEAMENT MAP
6	DEM ROSE LINEAMENT DIAGRAMS

PREPARED FOR:



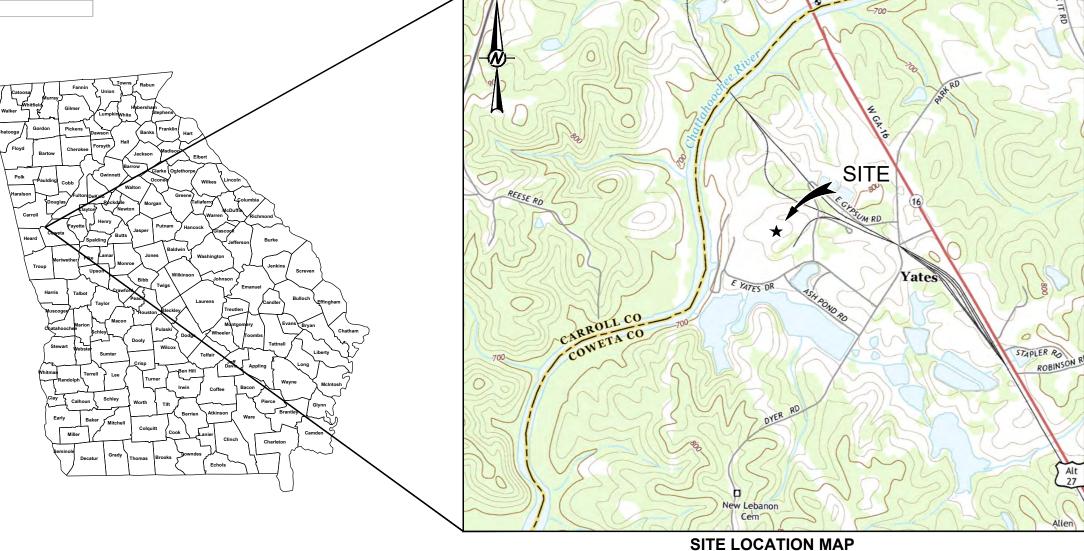
SOUTHERN COMPANY SERVICES, INC. 42 INVERNESS CENTER PARKWAY BIRMINGHAM, AL 35424

SITE ADDRESS AND OPERATOR: GEORGIA POWER 708 DYER ROAD NEWNAN, GA 30263

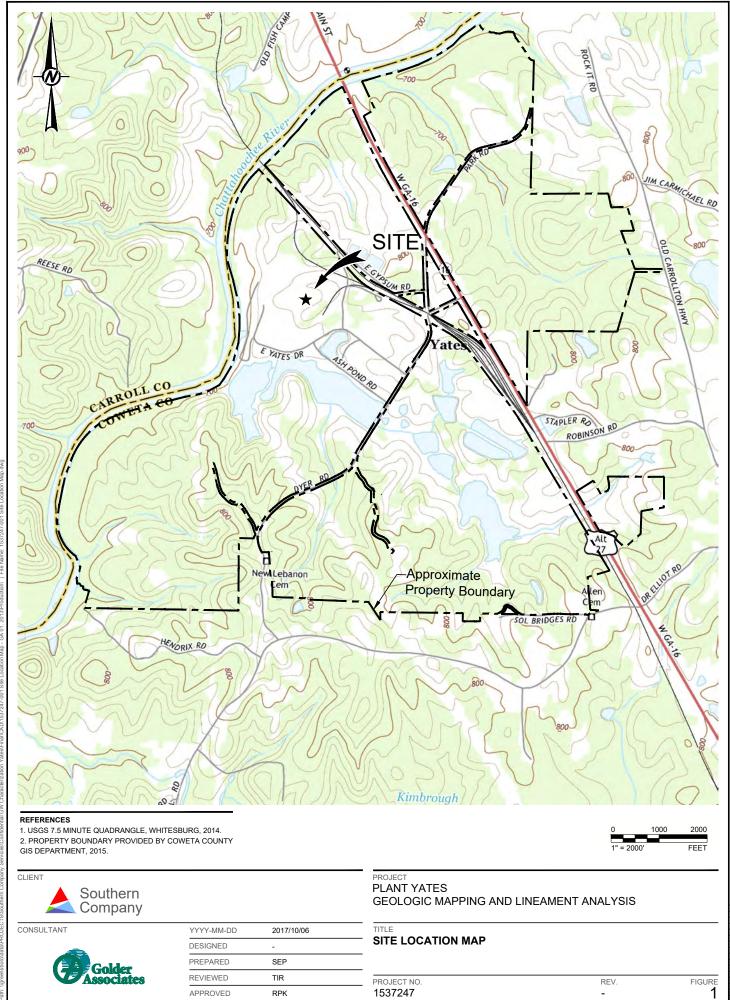
CLIENT CONTACT: MR. JOJU ABRAHAM

PHONE NUMBER: 205-992-6075

PLANS PREPARED BY:
GOLDER ASSOCIATES INC.
3730 CHAMBLEE TUCKER ROAD
ATLANTA, GA 30341
TELEPHONE: 770-496-1893

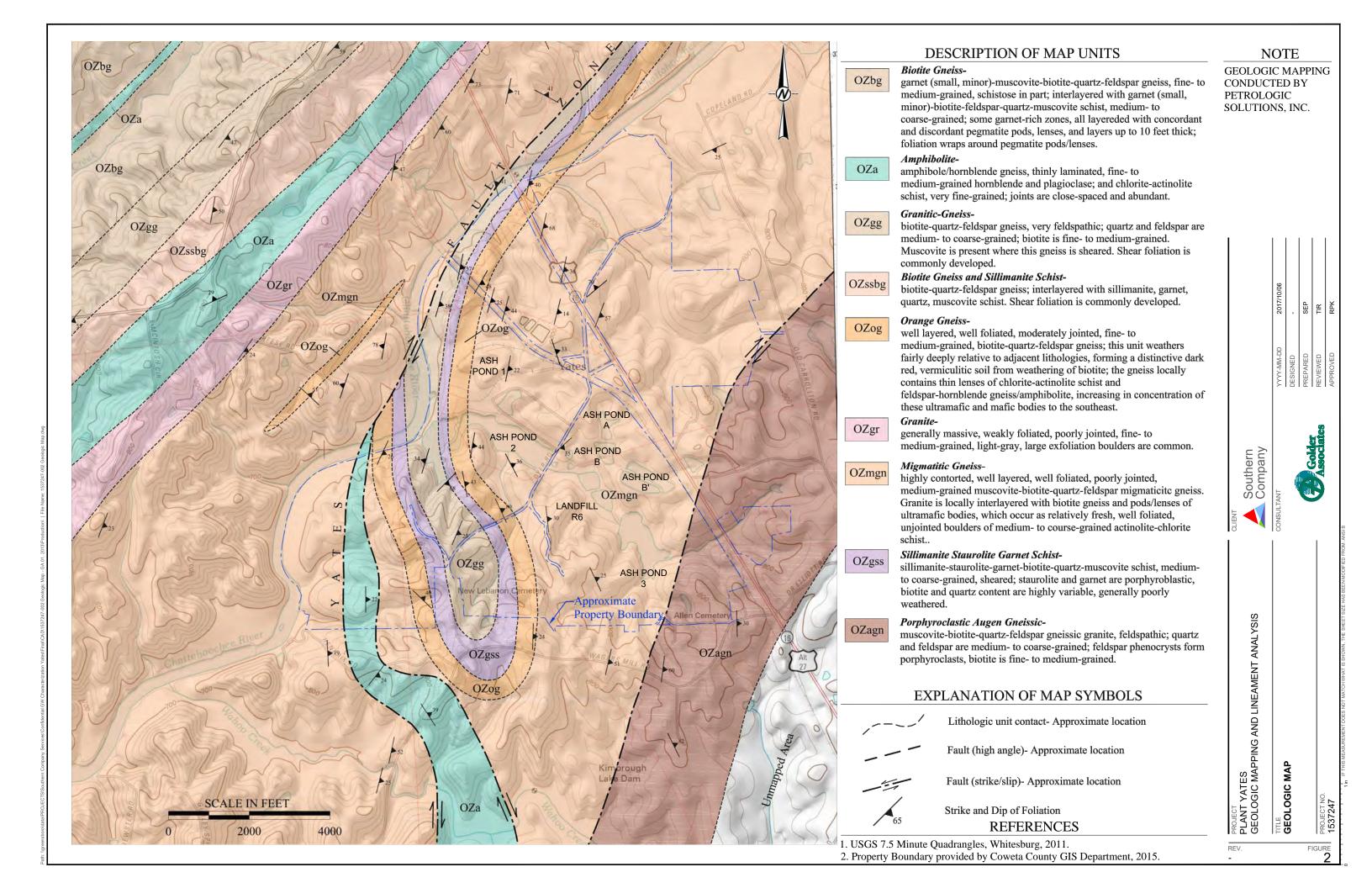


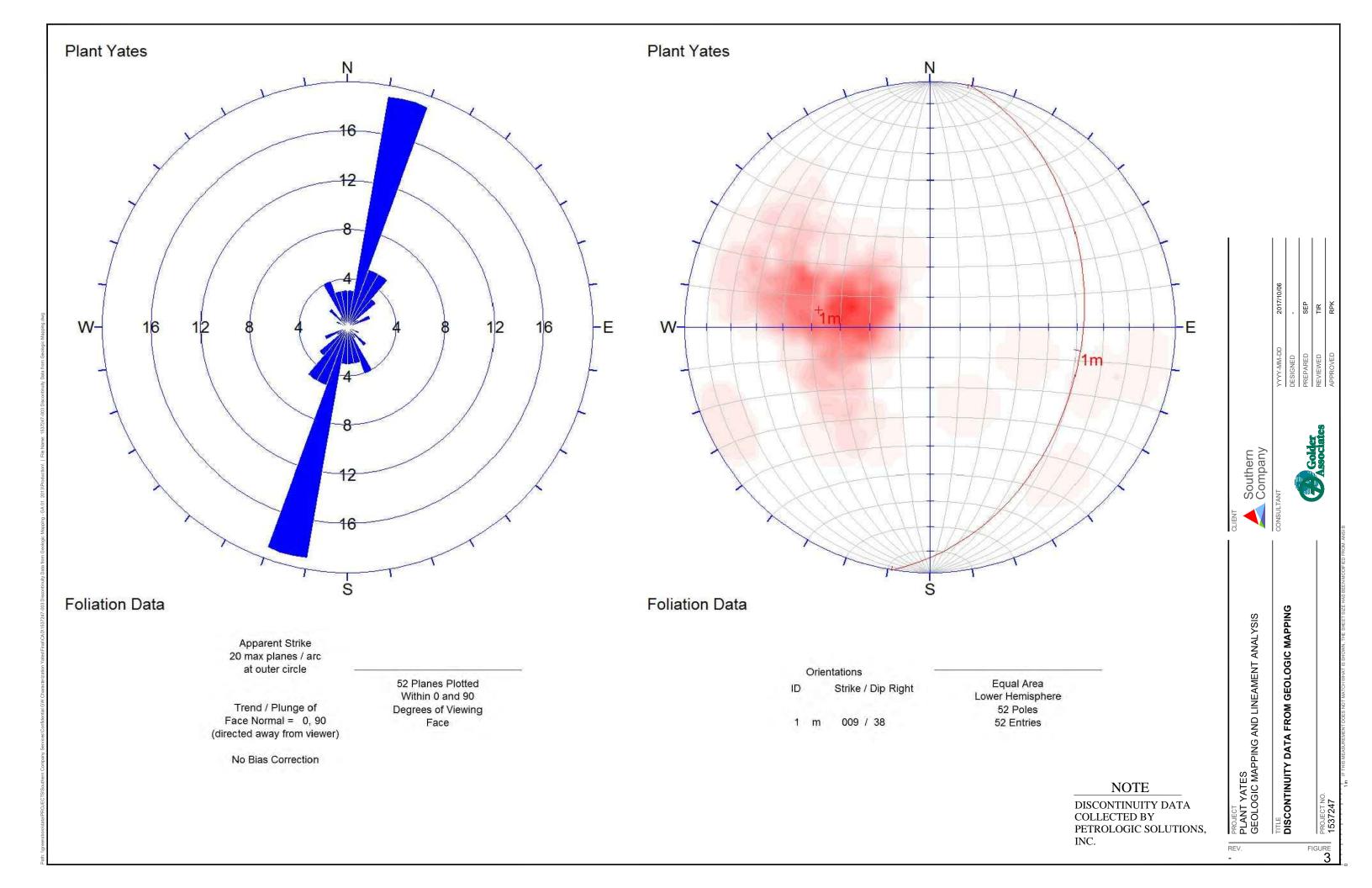


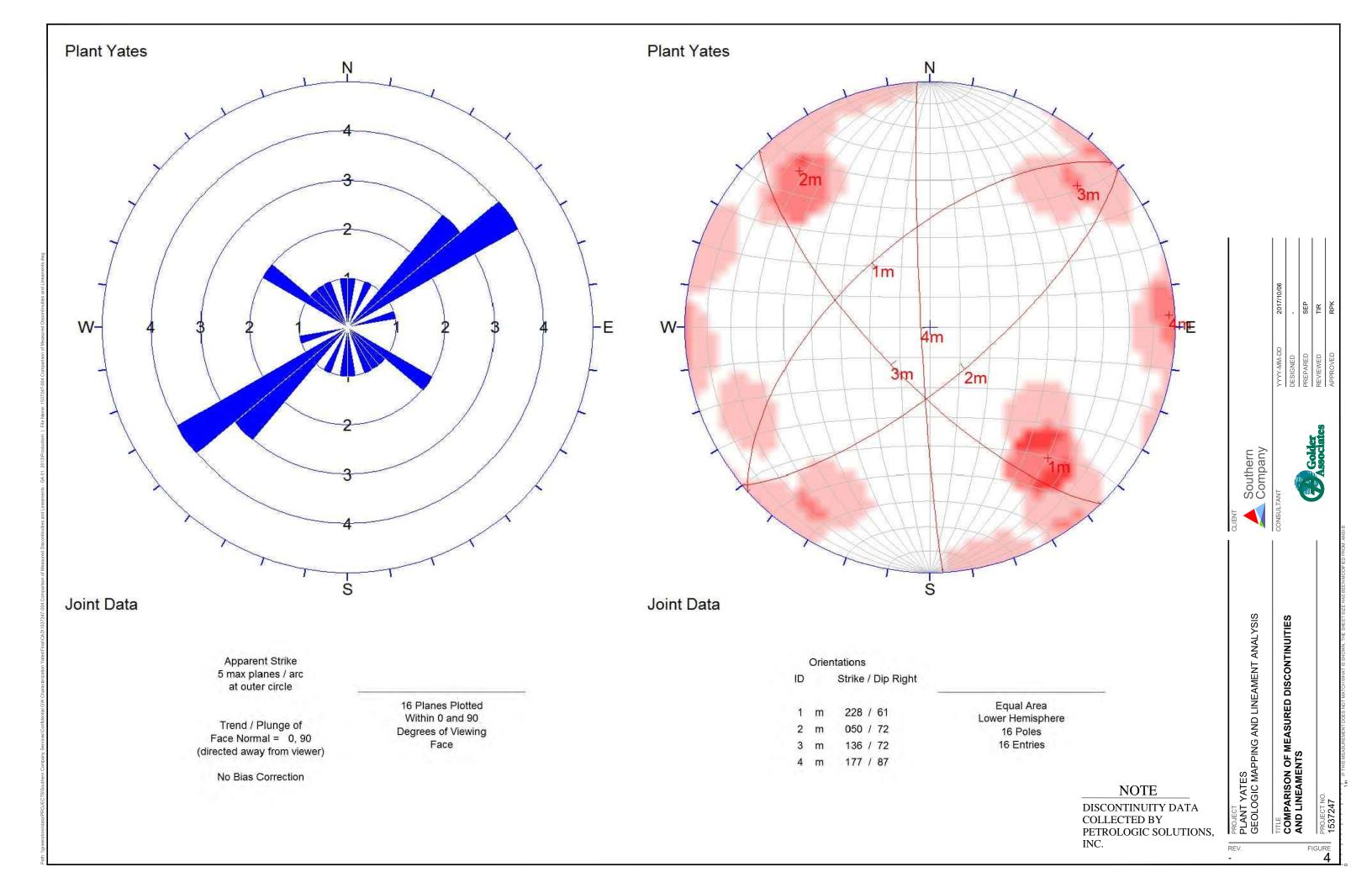


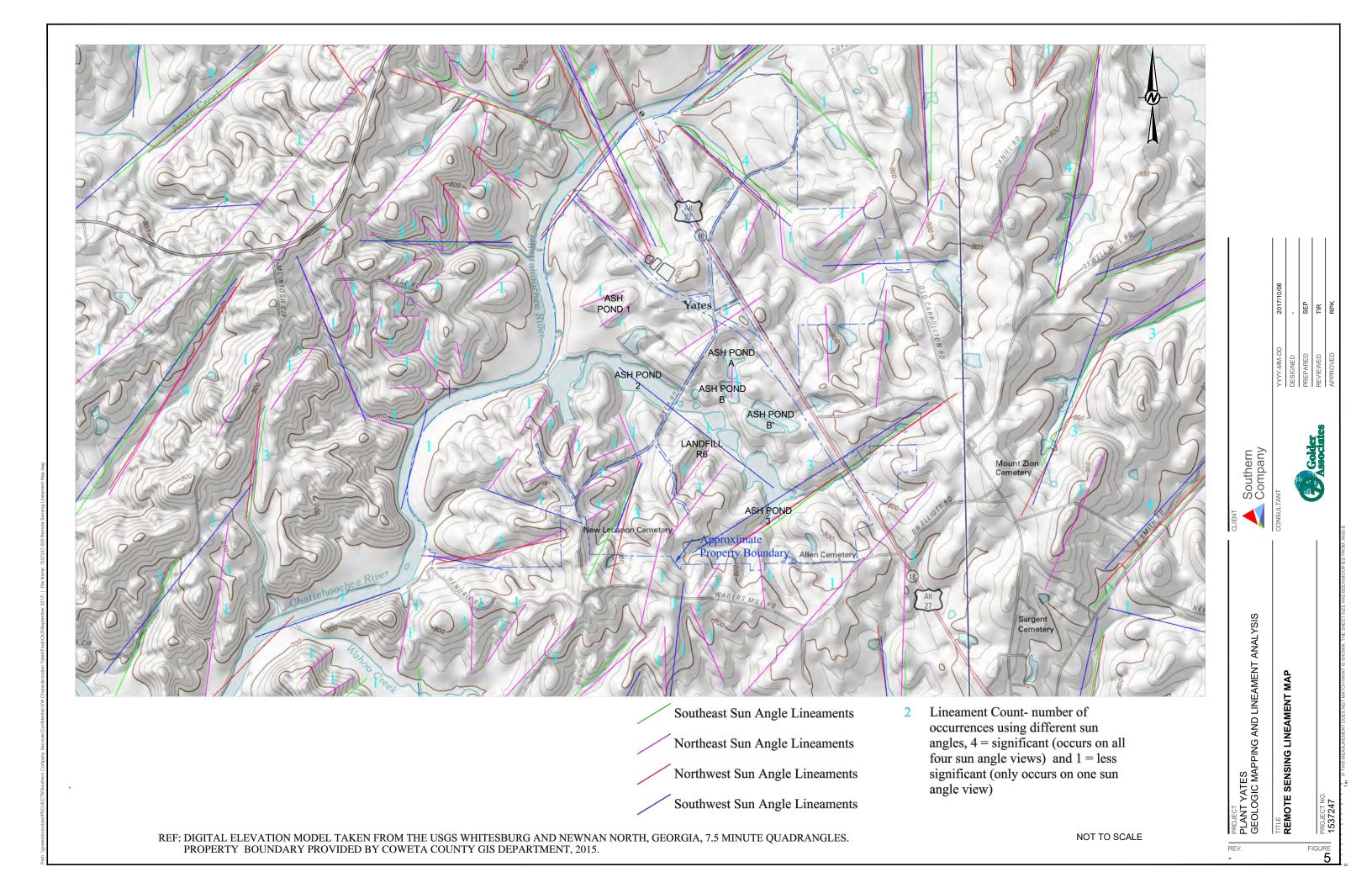
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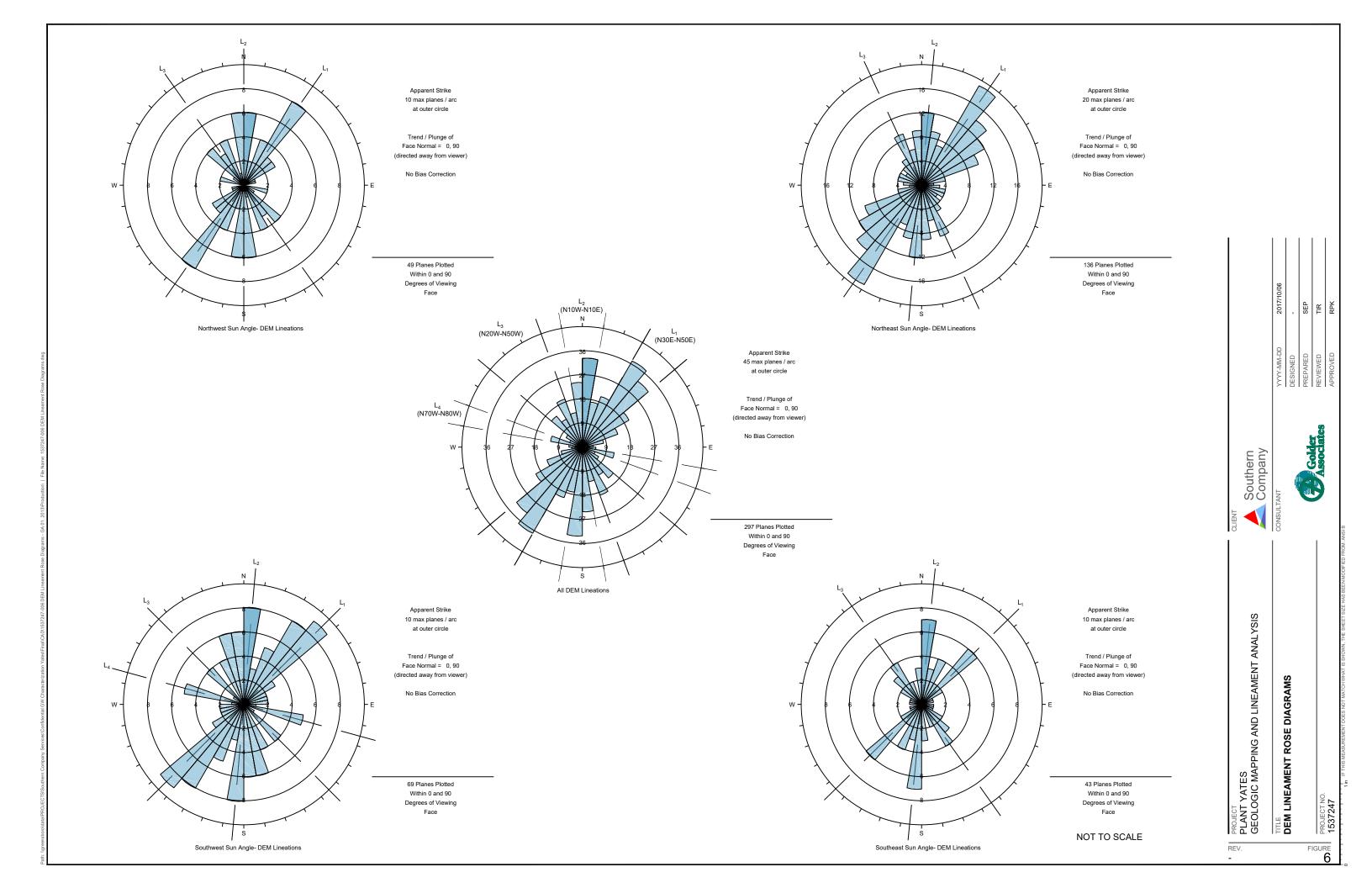
RPK











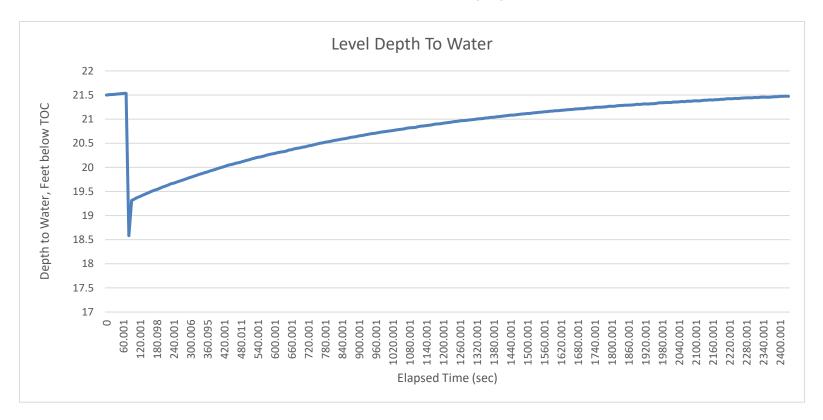
Golder Associates Inc. 3730 Chamblee Tucker Rd. Atlanta, GA 30341 Tel: (770 496-1893

Fax: (770) 934-9476



APPENDIX C HYDRAULIC CONDUCTIVITY TESTING AND RESULTS

PZ-4i Test 1 (in)



Log Configuration					
Log Name	PZ-04 I				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 10:00 AM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

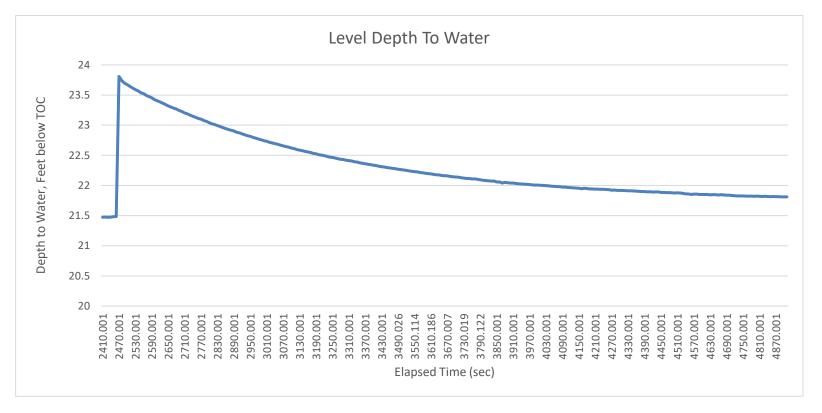
Other Log Settings					
Depth of Probe: 18.9311 (ft)					
Head Pressure:	8.19895 (PSI)				
Temperature:	60.3496 (F)				

Level Reference Settings At Log Creation					
Level Measurement Mode	Level Depth To Water				
Specific Gravity	0.999				
Level Reference Mode:	Set new reference				
Level Reference Value:	84				
Level Reference Head Pressure	23				

Device Properties				
Device	Level TROLL 500			
Site	Plant Yates			
Device Name				
Serial Number	160731			
Firmware Version	2.04			
Hardware Version	3			
Device Address	1			
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)			
Used Memory(%)	21.45 (ft)			
Used Battery(%)	8.19992 (PSI)			

10 21.507 460 20.084 910 20.662 1360 21.028 18 20 21.511 470 20.097 920 20.672 1370 21.038 18 30 21.516 480 20.112 930 20.684 1380 21.039 18 40 21.521 490 20.130 940 20.694 1390 21.049 18 50 21.528 500 20.147 950 20.703 1400 21.055 18 60 21.534 510 20.160 960 20.711 1410 21.060 18 70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19	21.267 210 21.266 220 21.278 230 21.282 240 21.284 250 21.290 260 21.288 270 21.293 280 21.299 290 21.307 200 21.306 210 21.312 220 21.312
20 21.511 470 20.097 920 20.672 1370 21.038 18 30 21.516 480 20.112 930 20.684 1380 21.039 18 40 21.521 490 20.130 940 20.694 1390 21.049 18 50 21.528 500 20.147 950 20.703 1400 21.055 18 60 21.534 510 20.160 960 20.711 1410 21.060 18 70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 1	220 21.278 230 21.282 240 21.284 250 21.290 260 21.288 270 21.293 280 21.299 290 21.307 200 21.306 210 21.312
30 21.516 480 20.112 930 20.684 1380 21.039 18 40 21.521 490 20.130 940 20.694 1390 21.049 18 50 21.528 500 20.147 950 20.703 1400 21.055 18 60 21.534 510 20.160 960 20.711 1410 21.060 18 70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19 <td>21.282 240 21.284 250 21.290 260 21.288 270 21.293 280 21.299 290 21.307 200 21.306 21.312</td>	21.282 240 21.284 250 21.290 260 21.288 270 21.293 280 21.299 290 21.307 200 21.306 21.312
40 21.521 490 20.130 940 20.694 1390 21.049 18 50 21.528 500 20.147 950 20.703 1400 21.055 18 60 21.534 510 20.160 960 20.711 1410 21.060 18 70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	21.284 250 21.290 260 21.288 270 21.293 280 21.299 290 21.307 200 21.306 210 21.312
50 21.528 500 20.147 950 20.703 1400 21.055 18 60 21.534 510 20.160 960 20.711 1410 21.060 18 70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	250 21.290 260 21.288 270 21.293 280 21.299 290 21.307 200 21.306 210 21.312
60 21.534 510 20.160 960 20.711 1410 21.060 18 70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	21.288 270 21.293 280 21.299 290 21.307 200 21.306 210 21.312
70 21.536 520 20.180 970 20.721 1420 21.069 18 80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	270 21.293 280 21.299 290 21.307 200 21.306 210 21.312
80 18.583 530 20.195 980 20.733 1430 21.076 18 90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	21.299 190 21.307 100 21.306 110 21.312
90 19.315 540 20.209 990 20.741 1440 21.083 18 100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	290 21.307 200 21.306 21.312
100 19.344 550 20.219 1000 20.749 1450 21.085 19 110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	21.306 210 21.312
110 19.373 560 20.232 1010 20.755 1460 21.091 19 120 19.398 570 20.253 1020 20.765 1470 21.099 19	21.312
120 19.398 570 20.253 1020 20.765 1470 21.099 19	
	11 11 1
130 13.424 360 20.200 1030 20.774 1460 21.100 13	
140 19.450 590 20.279 1040 20.783 1490 21.111 19	940 21.319
	950 21.321
	960 21.329
	770 21.338
	980 21.339
	990 21.344
	000 21.347
	21.343
	21.353
230 19.659 680 20.398 1130 20.857 1580 21.164 20	21.356
240 19.673 690 20.405 1140 20.866 1590 21.167 20	21.359
250 19.694 700 20.421 1150 20.875 1600 21.174 20	21.365
260 19.714 710 20.428 1160 20.882 1610 21.174 20	21.362
270 19.735 720 20.446 1170 20.896 1620 21.182 20	21.372
280 19.753 730 20.456 1180 20.902 1630 21.188 20	21.368
	990 21.378
	.00 21.382
	.10 21.379
	.20 21.386
	.30 21.391
	.40 21.394
	.50 21.400
	.60 21.397
	.70 21.405
	.80 21.403
	.90 21.413 200 21.411
	21.411
	21.422
	21.422
	21.423

PZ-4i Test 1 (out)



Log Configuration					
Log Name	PZ-04 I				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 10:00 AM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

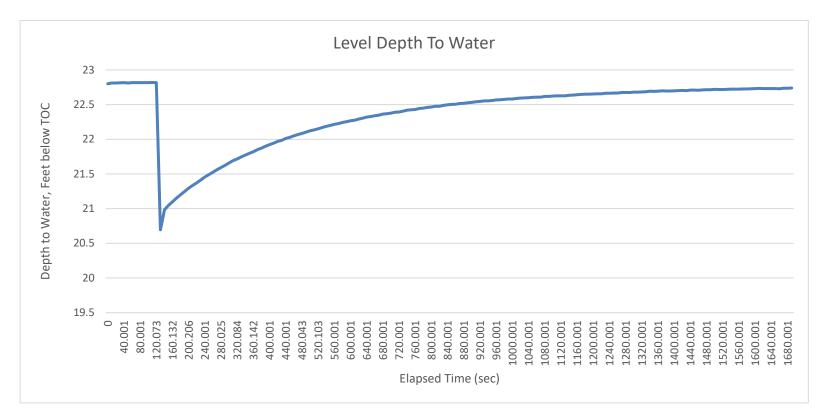
Other Log Settings				
Depth of Probe:	18.9311 (ft)			
Head Pressure:	8.19895 (PSI)			
Temperature:	60.3496 (F)			

Level Reference Settings At Log Creation					
Level Measurement Mode	Level Depth To Water				
Specific Gravity	0.999				
Level Reference Mode:	Set new reference				
Level Reference Value:	21.45 (ft)				
Level Reference Head Pressure	8.19992 (PSI)				

Device Properties				
Device	Level TROLL 500			
Site	Plant Yates			
Device Name				
Serial Number	160731			
Firmware Version	2.04			
Hardware Version	3			
Device Address	1			
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)			
Used Memory(%)	84			
Used Battery(%)	23			

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
2400		2850	22.958	3300	22.42	3750	22.118	4200	21.943
2410	21.473	2860	22.945	3310	22.413	3760	22.108	4210	21.94
2420	21.477	2870	22.928	3320	22.405	3770	22.109	4220	21.94
2430	21.472	2880	22.915	3330	22.394	3780	22.102	4230	21.935
2440	21.476	2890	22.904	3340	22.387	3790	22.094	4240	
2450	21.483	2900	22.882	3350	22.375	3800	22.087	4250	
2460	21.486	2910	22.873	3360	22.367	3810	22.082	4260	21.929
2470	23.809	2920	22.856		22.36		22.076		
2480	23.739	2930	22.839	3380	22.351	3830	22.073	4280	
2490 2500	23.697	2940 2950	22.825	3390	22.345	3840	22.073	4290 4300	
2510	23.672 23.641	2960	22.815 22.798	3400 3410	22.336 22.329	3850 3860	22.06 22.058	4310	21.918 21.917
2510	23.614	2970	22.786	3420	22.329	3870	22.038	4310	
2530	23.591	2980	22.77	3430	22.313	3880	22.053	4320	21.912
2540	23.569	2990	22.756	3440	22.306	3890	22.047	4340	21.911
2550	23.54	3000	22.741	3450	22.298	3900	22.039	4350	21.909
2560	23.523	3010	22.732	3460	22.29	3910	22.039	4360	21.906
2570	23.493	3020	22.716	3470	22.283	3920	22.035	4370	
2580	23.476	3030	22.704	3480	22.277	3930	22.028	4380	21.9
2590	23.453	3040	22.692	3490	22.27	3940	22.025	4390	21.9
2600	23.424	3050	22.685	3500	22.263	3950	22.023	4400	21.895
2610	23.404	3060	22.67	3510	22.256	3960	22.019	4410	21.896
2620	23.385	3070	22.656	3520	22.25	3970	22.017	4420	21.891
2630	23.365	3080	22.643	3530	22.243	3980	22.01	4430	21.893
2640	23.343	3090	22.634	3540	22.237	3990	22.008	4440	21.893
2650	23.32	3100	22.619	3550	22.229	4000	22.008	4450	
2660	23.3	3110	22.611	3560	22.226	4010	22.004	4460	21.885
2670	23.282	3120	22.597	3570	22.215	4020	21.999	4470	21.88
2680	23.268		22.585		22.21		21.996		
2690	23.243	3140	22.577	3590	22.203	4040	21.994	4490	
2700	23.227	3150	22.566		22.196		21.988		
2710 2720	23.203 23.187	3160 3170	22.557 22.547	3610	22.192 22.185	4060 4070	21.985 21.981	4510 4520	
2720	23.167	3170	22.547	3620 3630	22.185	4070	21.981		
2730	23.147	3190	22.535		22.179		21.978		
2750	23.147	3200	22.511		22.175		21.974		
2760	23.112	3210	22.503	3660	22.162	4110	21.968		
2770	23.099	3220	22.496		22.16		21.965	4570	
2780	23.078	3230	22.484	3680	22.151	4130	21.961	4580	
2790	23.062	3240	22.472	3690	22.143	4140	21.958		
2800	23.042	3250	22.463		22.142		21.956		
2810	23.024	3260	22.454		22.139		21.946		
2820	23.01	3270	22.444	3720	22.128	4170	21.954	4620	21.85
2830	22.992	3280	22.434	3730	22.122	4180	21.95	4630	21.846
2840	22.977	3290	22.43	3740	22.118	4190	21.944	4640	21.847

PZ-4s Test 1 (in)



Log Configuration					
Log Name	PZ-04 S				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 9:01 AM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

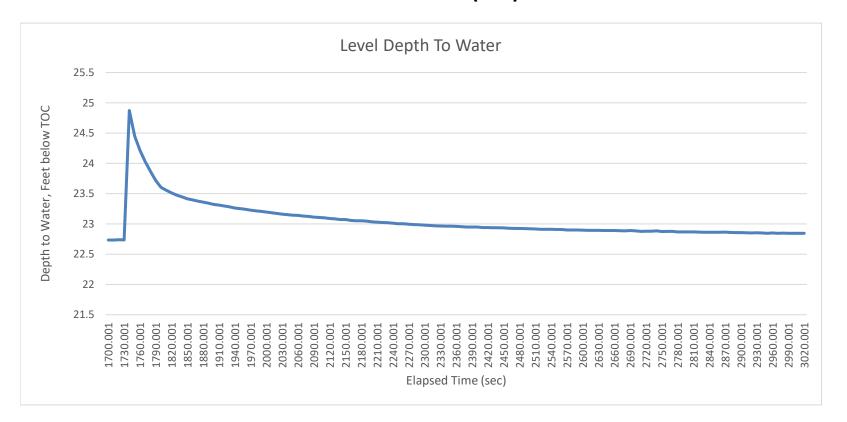
Other Log Settings					
Depth of Probe:	9.73925 (ft)				
Head Pressure:	4.21801 (PSI)				
Temperature:	60.8266 (F)				

Level Reference Settings At Log Creation						
Level Measurement Mode	Level Depth To Water					
Specific Gravity	0.999					
Level Reference Mode:	Set new reference					
Level Reference Value:	22.74 (ft)?					
Level Reference Head Pressure	4.21807 (PSI)					

Device Properties					
Device	Level TROLL 500				
Site	Plant Yates				
Device Name					
Serial Number	160731				
Firmware Version	2.04				
Hardware Version	3				
Device Address	1				
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)				
Used Memory(%)	81				
Used Battery(%)	23				

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
0	22.802	450	22.029	900	22.532	1350	22.692		
10	22.812	460	22.050	910	22.541	1360	22.694		
20	22.813	470	22.070	920	22.545	1370	22.698		
30	22.814	480	22.086	930	22.553	1380	22.696		
40	22.818	490	22.104	940	22.553	1390	22.697		
50	22.813	500	22.123	950	22.560	1400	22.700		
60	22.816	510	22.135	960	22.567	1410	22.703		
70	22.817	520	22.153	970	22.571	1420	22.705		
80	22.817	530	22.170		22.576	1430	22.702		
90	22.816	540	22.186	990		1440	22.710		
100	22.818	550	22.201	1000	22.581	1450	22.710		
110	22.819	560	22.216	1010	22.589	1460	22.708		
120	22.818	570	22.228	1020	22.594	1470	22.712		
130	20.695	580	22.242	1030		1480	22.716		
140	20.985	590	22.255	1040		1490	22.716		
150	21.046	600	22.268			1500	22.721		
160	21.101	610	22.275	1060	22.607	1510	22.718		
170	21.154	620	22.292	1070	22.607	1520	22.718		
180	21.202	630	22.304	1080		1530	22.721		
190	21.249	640	22.320	1090		1540	22.722		
200	21.295	650	22.330		22.624	1550	22.723		
210	21.336	660	22.340			1560	22.724		
220	21.375	670	22.347	1120	22.628	1570	22.725		
230	21.415	680	22.363	1130		1580	22.725		
240	21.457 21.492	690 700	22.370 22.377	1140		1590 1600	22.729		
250		+		1150			22.730		
260 270	21.528 21.563	710 720	22.390 22.394	1160 1170		1610 1620	22.735 22.732		
280	21.595		22.407				22.732		
290	21.626	740	22.420				22.732		
300	21.660		22.426			1650	22.732		
310	21.692	760	22.432	1210		1660	22.732		
320	21.717	770	22.445	1210		1670	22.725		
330	21.746	770	22.451			1680	22.736		
340	21.773	790	22.461	1240		1690	22.739		
350	21.799	800	22.466						
360	21.823	810	22.476						
370	21.851	820	22.476						
380	21.874	830	22.490						
390	21.901	840	22.497	1290					
400	21.924	850	22.502	1300					
410	21.945	860	22.507	1310	22.681				
420	21.970	870	22.518	1320	22.683				
430	21.986	880	22.519	1330	22.687				
440	22.013	890	22.528	1340	22.694				

PZ-4s Test 1 (out)



Log Configuration					
Log Name	PZ-04 S				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 9:01 AM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

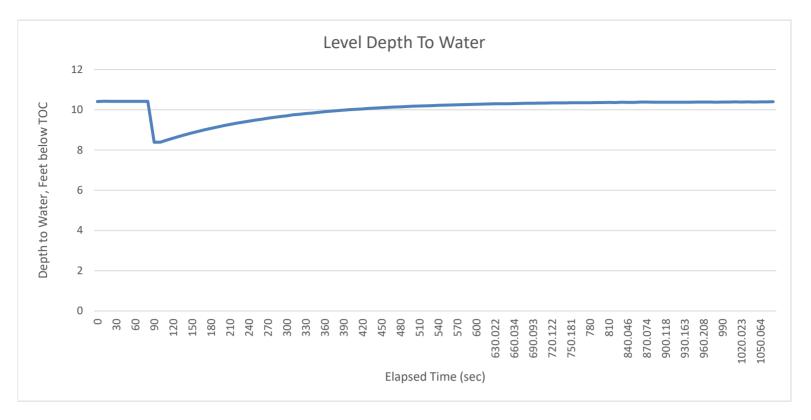
Other Log Settings					
Depth of Probe:	9.73925 (ft)				
Head Pressure:	4.21801 (PSI)				
Temperature:	60.8266 (F)				

Level Reference Settings At Log Creation					
Level Measurement Mode	Level Depth To Water				
Specific Gravity	0.999				
Level Reference Mode:	Set new reference				
Level Reference Value:	22.74 (ft) ²				
Level Reference Head Pressure	4.21807 (PSI)				

Device Properties					
Device	Level TROLL 500				
Site	Plant Yates				
Device Name					
Serial Number	160731				
Firmware Version	2.04				
Hardware Version	3				
Device Address	1				
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)				
Used Memory(%)	81				
Used Battery(%)	23				

1690 1700 1710 1720 1730 1740 1750 1760 1770 1780	22.734 22.734 22.739 22.735 24.873 24.448	2140 2150 2160 2170 2180	23.073 23.071 23.059 23.051	2600	22.901		
1710 1720 1730 1740 1750 1760	22.734 22.739 22.735 24.873 24.448	2160 2170 2180	23.059				
1720 1730 1740 1750 1760 1770	22.739 22.735 24.873 24.448	2170 2180			22.897		
1730 1740 1750 1760 1770	22.735 24.873 24.448	2180	22 NE 1	2610	22.894		
1740 1750 1760 1770	24.873 24.448		25.051	2620	22.895		
1750 1760 1770	24.448		23.051	2630	22.893		
1760 1770		2190	23.046	2640	22.891		
1770		2200	23.034	2650	22.89		
	24.215	2210	23.03	2660	22.891		
1780	24.028	2220	23.024	2670	22.889		
	23.864	2230	23.019	2680	22.885		
1790	23.717	2240	23.013	2690	22.89		
1800	23.604	2250	23.003	2700	22.884		
1810	23.555	2260	23.003	2710	22.878		
1820	23.513	2270	22.994	2720	22.88		
1830	23.474	2280	22.988	2730	22.88		
1840	23.445	2290	22.987	2740	22.885		
1850	23.415	2300	22.981	2750	22.874		
1860	23.398	2310	22.975	2760	22.877		
1870	23.377	2320	22.968	2770	22.876		
1880	23.359	2330	22.967	2780	22.868		
1890	23.342	2340	22.964	2790	22.869		
1900	23.323	2350	22.963	2800	22.868		
1910	23.311	2360	22.959	2810	22.869		
1920	23.296	2370	22.955	2820	22.865		
1930	23.281	2380	22.949	2830	22.863		
1940	23.263	2390	22.949	2840	22.863		
1950	23.254	2400	22.949	2850	22.861		
1960	23.242	2410	22.94	2860	22.861		
1970	23.228	2420	22.94	2870	22.864		
1980	23.216	2430	22.937	2880	22.859		
1990	23.206	2440	22.936	2890	22.856		
2000	23.196	2450	22.933	2900	22.857		
2010	23.183	2460	22.928		22.855		
2020	23.173	2470	22.926		22.851		
2030	23.162	2480	22.926		22.853		
2040	23.154	2490	22.924	2940	22.851		
2050	23.144	2500	22.92	2950	22.846		
2060	23.142	2510	22.917	2960	22.85		
2070	23.129	2520	22.911	2970	22.846		
2080	23.124	2530	22.911	2980	22.847		
2090	23.113	2540	22.91	2990	22.844		
2100	23.106	2550	22.908	3000	22.845		
2110	23.101	2560	22.909	3010	22.846		
2120	23.089	2570	22.90	3020	22.844		
2130	23.085	2580	22.9		22.044		

PZ-5d Test 1 (in)



Log Configuration					
Log Name	PZ-05 D				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 1:15 PM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

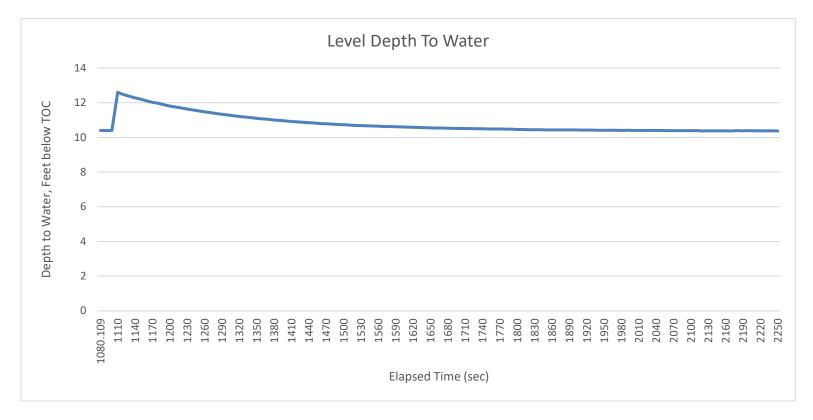
Other Log Settings				
Depth of Probe:	43.0264 (ft)			
Head Pressure:	18.6345 (PSI)			
Temperature:	62.0456 (F)			

Level Reference Settings At Log Creation					
Level Measurement Mode	Level Depth To Water				
Specific Gravity	0.999				
Level Reference Mode:	Set new reference				
Level Reference Value:	10.43 (ft)				
Level Reference Head Pressure	18.6373 (PSI)				

Device Properties					
Device	Level TROLL 500				
Site	Plant Yates				
Device Name					
Serial Number	160731				
Firmware Version	2.04				
Hardware Version	3				
Device Address	1				
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)				
Used Memory(%)	93				
Used Battery(%)	23				

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
0	10.412	450	10.104	900	10.378				
10	10.425	460	10.120	910	10.379				
20	10.422	470	10.134	920	10.378				
30	10.423	480	10.149	930	10.378				
40	10.422	490	10.164	940	10.381				
50	10.422	500	10.176	950	10.382				
60	10.422	510	10.187	960	10.383				
70	10.421	520	10.201	970	10.383				
80	10.416	530	10.210	980	10.381				
90	8.385	540	10.221	990	10.387				
100	8.391	550	10.232	1000	10.384				
110	8.492	560	10.242	1010	10.393				
120	8.590	570	10.251	1020	10.389				
130	8.683	580	10.259	1030	10.398				
140	8.772	590	10.269	1040	10.389				
150	8.854	600	10.276	1050	10.396				
160	8.932	610	10.283	1060	10.397				
170	9.008	620	10.288	1070	10.400				
180	9.079	630	10.297						
190	9.146	640	10.301						
200	9.210	650	10.304						
210	9.270	660	10.312						
220	9.330	670	10.317						
230	9.386	680	10.324						
240	9.438	690	10.326						
250	9.487	700	10.330						
260	9.533	710	10.336						
270	9.584	720	10.339						
280	9.622	730	10.342						
290	9.667	740	10.346						
300	9.704	750	10.348						
310	9.750	760	10.350						
320	9.775	770	10.353						
330	9.810	780	10.353						
340	9.841	790	10.360						
350	9.874	800	10.358						
360	9.903	810	10.369						
370	9.928	820	10.362						
380	9.955	830	10.375						
390	9.981	840	10.367						
400	10.007	850	10.367						
410	10.026	860	10.383						
420	10.046	870	10.382						
430	10.068		10.375						
440	10.086	890	10.378						

PZ-5d Test 1 (out)



Log Configuration					
Log Name	PZ-05 D				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 1:15 PM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

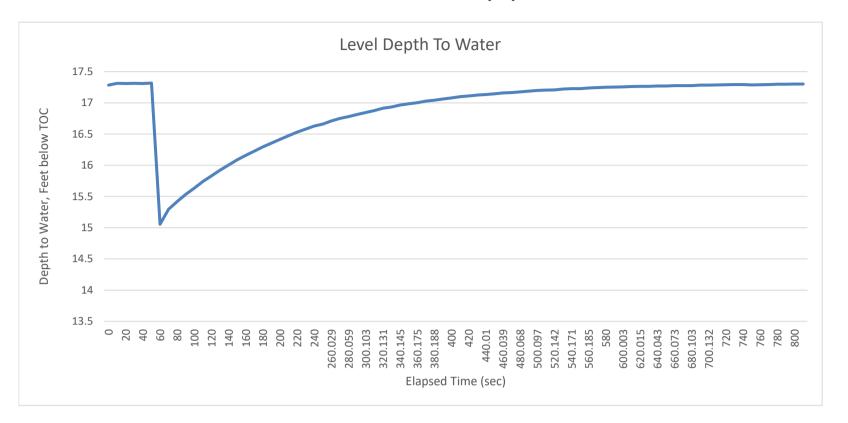
Other Log Settings					
Depth of Probe:	43.0264 (ft)				
Head Pressure:	18.6345 (PSI)				
Temperature:	62.0456 (F)				

Level Reference Settings At Log Creation						
Level Measurement Mode Level Depth To Water						
Specific Gravity	0.999					
Level Reference Mode:	Set new reference					
Level Reference Value:	10.43 (ft)					
Level Reference Head Pressure	18.6373 (PSI)					

Device Properties					
Device	Level TROLL 500				
Site	Plant Yates				
Device Name					
Serial Number	160731				
Firmware Version	2.04				
Hardware Version	3				
Device Address	1				
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)				
Used Memory(%)	93				
Used Battery(%)	23				

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
20		470		920		1370	11.043	1820	10.45
30		480		930		1380	11.009	1830	10.444
40		490		940		1390	10.98	1840	10.443
50		500		950		1400	10.951	1850	10.439
60		510		960		1410	10.923	1860	10.437
70		520		970		1420	10.897	1870	10.431
80		530		980		1430	10.874	1880	10.437
90		540		990		1440	10.85	1890	10.434
100		550		1000		1450	10.828	1900	10.433
110		560		1010		1460	10.804	1910	10.423
120 130		570 580		1020 1030		1470 1480	10.788 10.77	1920 1930	10.423 10.42
140		590		1030		1490	10.77	1930	10.42
150		600		1040		1500	10.731	1940	10.418
160		610		1060		1510	10.738	1960	10.418
170		620		1070		1520	10.698	1970	10.414
180		630		1080	10.401	1530	10.688	1980	10.409
190		640		1090	10.394	1540	10.676	1990	10.411
200		650		1100	10.402	1550	10.662	2000	10.407
210		660		1110	12.599	1560	10.646	2010	10.404
220		670		1120	12.472	1570	10.634	2020	10.404
230		680		1130	12.373	1580	10.624	2030	10.402
240		690		1140	12.284	1590	10.615	2040	10.399
250		700		1150	12.203	1600	10.606	2050	10.399
260		710		1160	12.114	1610	10.595	2060	10.396
270		720		1170	12.026	1620	10.585	2070	10.395
280		730		1180	11.97	1630	10.576	2080	10.39
290		740		1190	11.885	1640	10.566	2090	10.392
300		750		1200	11.808	1650	10.556	2100	10.388
310		760		1210	11.759	1660	10.547	2110	10.389
320		770		1220	11.699	1670	10.541	2120	10.375
330		780		1230	11.641	1680	10.53	2130	10.382
340		790		1240	11.587	1690	10.526	2140	10.384
350		800		1250	11.53	1700	10.52	2150	10.383
360		810		1260	11.477	1710	10.511	2160	10.381
370		820		1270	11.429	1720	10.507	2170	10.375
380		830		1280	11.382	1730	10.502	2180	10.388
390		840		1290	11.334		10.495	2190	10.383
400		850		1300	11.294		10.49		10.391
410		860		1310	11.25		10.489		10.385
420		870		1320	11.21	1770			10.377
430		880		1330	11.177	1780	10.482	2230	10.378
440		890		1340	11.14				10.379
450		900		1350	11.106		10.455	2250	10.376
460		910		1360	11.071	1810	10.455	2260	

PZ-5i Test 1 (in)



Log Configuration					
Log Name	PZ-05 I				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 12:26 PM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

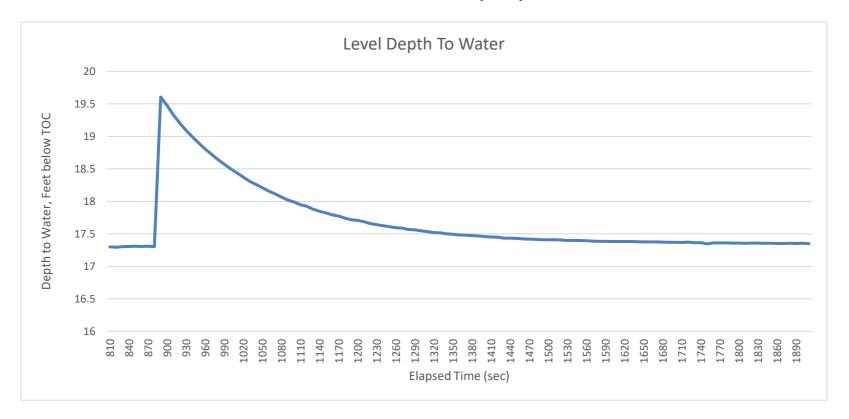
Other Log Settings					
Depth of Probe:	30.4527 (ft)				
Head Pressure:	13.1889 (PSI)				
Temperature:	61.7533 (F)				

Level Reference Settings At Log Creation						
Level Measurement Mode Level Depth To Water						
Specific Gravity	0.999					
Level Reference Mode:	Set new reference					
Level Reference Value:	17.31 (ft)					
Level Reference Head Pressure	13.1886 (PSI)					

	Device Properties					
Device	Level TROLL 500					
Site	Plant Yates					
Device Name						
Serial Number	160731					
Firmware Version	2.04					
Hardware Version	3					
Device Address	1					
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)					
Used Memory(%)	90					
Used Battery(%)	23					

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
0	17.284	450	17.146						
10	17.312	460	17.159						
20	17.311	470	17.165						
30	17.313	480	17.177						
40	17.311	490	17.188						
50	17.317	500	17.198						
60	15.056	510	17.206						
70	15.297	520	17.207						
80	15.421	530	17.221						
90	15.534	540	17.227						
100	15.636	550	17.226						
110	15.740	560	17.238						
120	15.833	570	17.245						
130	15.922	580	17.251						
140	16.005	590	17.252						
150	16.088	600	17.255						
160	16.158	610	17.262						
170	16.229	620	17.264						
180	16.295	630	17.265						
190	16.357	640	17.270						
200	16.417	650	17.271						
210	16.475	660	17.275						
220	16.532	670	17.275						
230	16.580	680	17.277						
240	16.628	690	17.285						
250	16.661	700	17.285						
260	16.711	710	17.286						
270	16.751	720	17.290						
280	16.780		17.292						
290	16.815		17.294						
300	16.847	750	17.286						
310	16.876		17.291						
320	16.914		17.292						
330	16.934		17.298						
340	16.965		17.299						
350	16.985	-	17.302						
360	17.002		17.301						
370	17.028								
380	17.041								
390	17.062								
400	17.079								
410	17.098								
420	17.110	-							
430	17.125	-							
440	17.133								

PZ-5i Test 1 (out)



Log Configuration					
Log Name	PZ-05 I				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 12:26 PM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

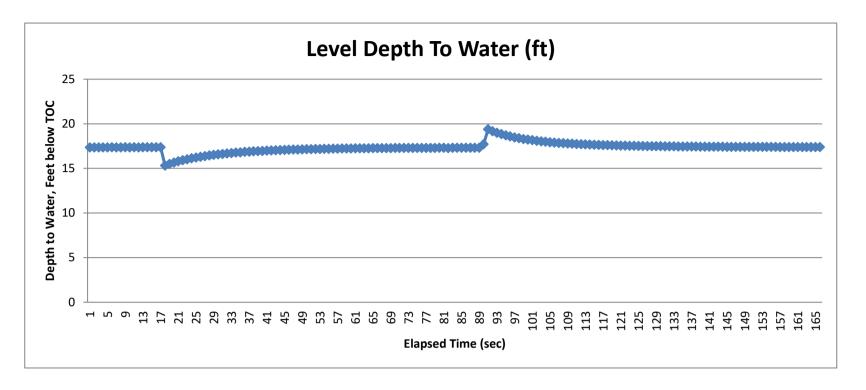
Other Log Settings				
Depth of Probe:	30.4527 (ft)			
Head Pressure:	13.1889 (PSI)			
Temperature:	61.7533 (F)			

Level Reference Settings At Log Creation					
Level Measurement Mode	Level Depth To Water				
Specific Gravity	0.999				
Level Reference Mode:	Set new reference				
Level Reference Value:	17.31 (ft)				
Level Reference Head Pressure	13.1886 (PSI)				

Device Properties					
Device	Level TROLL 500				
Site	Plant Yates				
Device Name					
Serial Number	160731				
Firmware Version	2.04				
Hardware Version	3				
Device Address	1				
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)				
Used Memory(%)	90				
Used Battery(%)	23				

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
0		450		900	19.476	1350	17.493	1800	17.357
10		460		910	19.33	1360	17.483	1810	17.356
20		470		920	19.204	1370	17.478	1820	17.357
30		480		930	19.091	1380	17.473	1830	17.357
40		490		940	18.99	1390	17.467	1840	17.356
50		500		950	18.894	1400	17.459	1850	17.355
60		510		960	18.805	1410	17.452	1860	17.353
70		520		970	18.724	1420	17.448	1870	17.353
80		530		980	18.644	1430	17.435	1880	17.355
90		540		990	18.569	1440	17.433	1890	17.351
100		550		1000	18.5	1450	17.43	1900	17.354
110		560		1010	18.438	1460	17.423	1910	17.349
120		570		1020	18.372	1470	17.418		
130		580		1030	18.309	1480	17.415		
140		590		1040	18.26	1490	17.41		
150		600		1050	18.209	1500	17.41		
160		610		1060	18.158	1510	17.409		
170		620		1070	18.113	1520	17.406		
180		630		1080	18.067	1530	17.396		
190		640		1090	18.02	1540	17.4		
200		650		1100	17.989	1550	17.396		
210		660		1110	17.946	1560	17.395		
220		670		1120	17.924	1570	17.389		
230		680		1130	17.878	1580	17.386		
240		690		1140	17.846	1590	17.386		
250		700		1150	17.824	1600	17.382		
260		710		1160	17.792	1610	17.382		
270		720		1170	17.774	1620	17.382		
280		730		1180			17.382		
290		740		1190 1200			17.38		
300 310		750 760		1200	17.706 17.685		17.376 17.376		
320		770		1210			17.375		
330		770		1230			17.373		
340		790		1240		1690	17.369		
350		800		1250		1700	17.371		
360		810	17.301	1260			17.368		
370		820	17.294				17.372		
380		830	17.303		17.567	1720	17.365		
390		840	17.307				17.363		
400		850	17.308		17.545	1750	17.346		
410		860	17.306				17.362		
420		870	17.308				17.362		
430		880	17.302				17.36		
440		890	19.605			1790	17.358		

PZ-05 S



Log Configuration					
Log Name	PZ-05 S				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 11:50 AM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Type	Fast Linear				

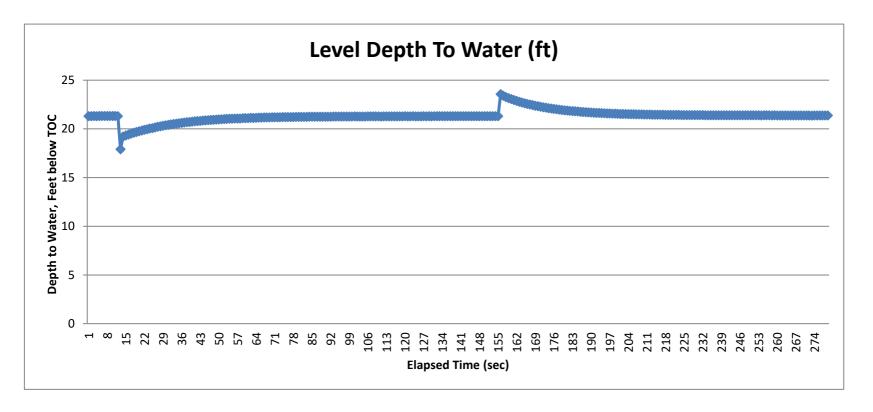
Other Log Settings				
Depth of Probe:	24.4049 (ft)			
Head Pressure:	10.5696 (PSI)			
Temperature:	65.1142 (F)			

Level Reference Settings At Log Creation					
Level Measurement Mode	Level Depth To Water				
Specific Gravity	0.999				
Level Reference Mode:	Set new reference				
Level Reference Value:	17.35 (ft)				
Level Reference Head Pressure	10.5736 (PSI)				

Device Properties				
Device Level TROLL 500				
Site	Plant Yates			
Device Name				
Serial Number	160731			
Firmware Version	2.04			
Hardware Version	3			
Device Address	1			
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)			
Used Memory(%)	87			
Used Battery(%)	23			

Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)	Elapsed Time (sec)	Level Depth To Water (ft)
0	17.348	450	17.098	900	19.398	1350	17.454		
10	17.357	460	17.113	910	19.174	1360	17.446		
20	17.357	470	17.122	920	18.996	1370	17.451		
30	17.364	480	17.144	930	18.842	1380	17.442		
40	17.36	490	17.153	940	18.706	1390	17.441		
50	17.37	500	17.166	950	18.588	1400	17.438		
60	17.362	510	17.174	960	18.48	1410	17.436		
70	17.362	520	17.186			1420	17.429		
80	17.367	530	17.193			1430	17.428		
90	17.365	540	17.195			1440	17.425		
100	17.369	550	17.213		18.164	1450	17.42		
110	17.365	560	17.22			1460	17.419		
120	17.371	570	17.23			1470	17.414		
130	17.369	580	17.234			1480	17.417		
140	17.374	590	17.243		17.943	1490	17.411		
150	17.373	600	17.251	1050	17.904	1500	17.411		
160	17.372	610	17.253			1510	17.408		
170	15.328	620	17.263			1520	17.411		
180	15.505	630	17.263			1530	17.409		
190	15.659	640	17.269			1540	17.409		
200	15.794	650	17.273		17.739	1550	17.408		
210	15.919	660	17.277	1110		1560	17.402		
220 230	16.027 16.132	670	17.281 17.285	1120		1570	17.403 17.404		
240	16.132	680 690	17.285	1130 1140		1580 1590	17.404		
250	16.309	700	17.287	1140	17.64	1600	17.401		
260	16.385	700	17.291	1160		1610	17.401		
270	16.456	720	17.291	1170		1620	17.399		
280	16.52		17.294				17.397		
290	16.576		17.297				17.4		
300	16.623	750	17.3			1650	17.389		
310	16.68		17.304						
320	16.725	770	17.305						
330	16.77	780	17.305						
340	16.807	790	17.309						
350	16.85	800	17.314						
360	16.882	810	17.276						
370	16.917	820	17.317	1270	17.498				
380	16.947	830	17.323	1280	17.493				
390	16.967	840	17.319	1290	17.486				
400	16.993	850	17.317	1300	17.479				
410	17.021	860	17.322	1310	17.474				
420	17.043	870	17.321	1320	17.467				
430	17.062	880	17.321	1330	17.462				
440	17.078	890	17.721	1340	17.455				

PZ-06 D



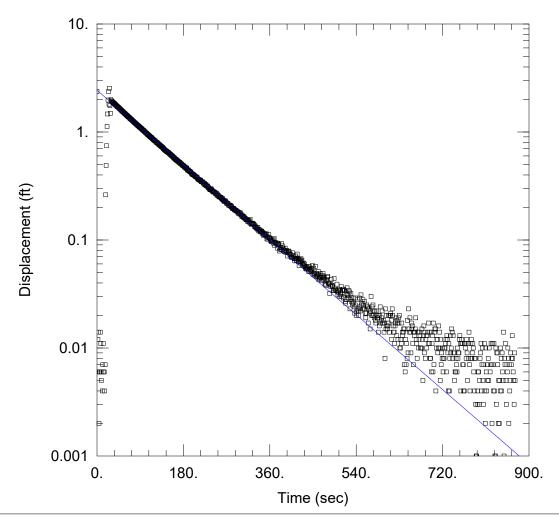
Log Configuration					
Log Name	PZ-06 D				
Created By	X2WSHAUG				
Computer Name	X2WSHAUGH				
Application	WinSitu.exe				
Application Version	5.6.25.0				
Create Date	7/10/14 7:04 AM				
Log Setup Time Zone	Central Daylight Time				
Notes Size(bytes)	4096				
Overwrite when full	Disabled				
Scheduled Start Time	Manual Start				
Scheduled Stop Time	No Stop Time				
Туре	Fast Linear				

Other Log Settings					
Depth of Probe:	88.7382 (ft)				
Head Pressure:	38.432 (PSI)				
Temperature:	63.6079 (F)				

Level Reference Settings At Log Creation							
Level Measurement Mode	Level Depth To Water						
Specific Gravity	0.999						
Level Reference Mode:	Set new reference						
Level Reference Value:	21.33 (ft)						
Level Reference Head Pressure	38.4335 (PSI)						

Device Properties					
Device	Level TROLL 500				
Site	Plant Yates				
Device Name					
Serial Number	160731				
Firmware Version	2.04				
Hardware Version	3				
Device Address	1				
Device Comm Cfg	19200,8,Even,1,(Modbus-RTU)				
Used Memory(%)	78				
Used Battery(%)	23				

10	Elapsed Time (sec)	Level Depth To Water (ft)								
20	0	21.313	450	20.898	900	21.257	1350	21.301	1800	21.883
30	10	21.314	460	20.921	910	21.258	1360	21.299	1810	21.851
40	20	21.324	470	20.935	920	21.261	1370	21.3	1820	21.826
S0	30	21.305	480	20.956	930	21.262	1380	21.302	1830	21.803
60 21.319 510 21.008 960 21.268 1410 21.301 1860 22 70 21.324 530 21.031 970 21.266 1420 21.302 1870 22 80 21.324 530 21.038 980 21.271 1430 21.304 1880 22 100 21.321 550 21.064 1000 21.293 1450 21.304 1880 22 110 21.304 560 21.057 1010 21.275 1460 21.298 1910 22 120 17.9 570 21.068 1020 21.276 1470 21.305 1920 22 130 19.231 590 21.105 1040 21.276 1480 21.305 1920 22 140 19.321 590 21.105 1040 21.278 1500 21.305 1940 22 150 19.42 600 21.097 <t< td=""><td>40</td><td>21.318</td><td>490</td><td>20.973</td><td>940</td><td>21.26</td><td>1390</td><td>21.3</td><td>1840</td><td>21.796</td></t<>	40	21.318	490	20.973	940	21.26	1390	21.3	1840	21.796
To 21.323 520 21.021 970 21.266 1420 21.302 1870 22.	50	21.325	500	20.987	950	21.264	1400	21.305	1850	21.755
80		21.319	510	21.008	960	21.269	1410	21.301	1860	21.733
90			520	21.021	970	21.266	1420		1870	21.713
100	-		530		980		1430		1880	21.696
110										21.681
120	100		550		1000		1450			21.662
130										21.645
140			570							21.632
150	-									21.616
160 19.512 610 21.132 1060 21.281 1510 21.306 1960 22 170 19.6 620 21.119 1070 21.279 1520 21.305 1970 22 180 19.681 630 21.144 1080 21.279 1530 21.305 1970 22 200 19.829 650 21.161 1100 21.274 1550 23.555 2000 22 210 19.901 660 21.169 1110 21.28 1560 23.395 2010 22 220 19.953 670 21.174 1120 21.288 1570 23.27 2020 22 230 20.036 680 21.181 1130 21.288 1570 23.27 2020 22 230 20.036 690 21.188 1140 21.29 1590 23.055 2040 22 250 20.154 700 21.9 <										21.603
170 19.6 620 21.119 1070 21.279 1520 21.305 1970 22 180 19.681 630 21.144 1080 21.279 1530 21.306 1980 22 190 19.757 640 21.156 1090 21.28 1540 21.307 1990 22 200 19.829 650 21.161 1100 21.278 1550 23.555 2000 22 210 19.901 660 21.169 1110 21.28 1560 23.395 2010 22 220 19.953 670 21.174 1120 21.288 1570 23.27 2020 22 230 20.036 680 21.181 1130 21.287 1580 23.155 2030 22 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 22 250 20.154 700 21.19 <	-									21.59
180 19.681 630 21.144 1080 21.279 1530 21.306 1980 22 190 19.757 640 21.156 1090 21.28 1540 21.307 1990 22 200 19.829 650 21.161 1100 21.274 1550 23.555 2000 22 210 19.901 660 21.169 1110 21.28 1560 23.395 2010 22 220 19.953 670 21.174 1120 21.288 1560 23.395 2010 22 230 20.036 680 21.81 1130 21.287 1580 23.155 2030 22 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 22 250 20.144 700 21.199 1150 21.286 1600 22.988 2050 22 260 20.213 710 21.195										21.579
190 19.757 640 21.156 1090 21.28 1540 21.307 1990 22 200 19.829 650 21.161 1100 21.274 1550 23.555 2000 22 210 19.901 660 21.169 1110 21.28 1560 23.395 2010 22 220 19.953 670 21.174 1120 21.288 1570 23.27 2020 22 230 20.036 680 21.181 1130 21.287 1580 23.155 2030 22 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 22 250 20.154 700 21.195 1160 21.286 1600 22.958 2050 22 260 20.213 710 21.195 1160 21.288 1610 22.868 2060 22 270 20.267 720 21.199	-									21.568
200 19.829 650 21.161 1100 21.274 1550 23.555 2000 22 210 19.901 660 21.169 1110 21.28 1560 23.395 2010 22 220 19.953 670 21.174 1120 21.288 1570 23.27 2020 22 230 20.036 680 21.181 1130 21.287 1580 23.155 2030 22 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 22 250 20.154 700 21.19 1150 21.286 1600 22.958 2050 22 260 20.213 710 21.195 1160 21.288 1610 22.868 2060 22 270 20.267 720 21.199 1170 21.292 1620 22.785 2070 22 280 20.315 730 21.219										21.559
210 19.901 660 21.169 1110 21.28 1560 23.395 2010 22 220 19.953 670 21.174 1120 21.288 1570 23.27 2020 22 230 20.036 680 21.181 1130 21.287 1580 23.155 2030 22 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 22 250 20.154 700 21.19 1150 21.286 1600 22.958 2050 22 260 20.213 710 21.195 1160 21.286 1600 22.958 2050 22 270 20.267 720 21.199 1170 21.289 1620 22.785 2070 22 280 20.315 730 21.207 1180 21.29 1630 22.707 2080 22 290 20.37 740 21.212										21.547
220 19.953 670 21.174 1120 21.288 1570 23.27 2020 2: 230 20.036 680 21.181 1130 21.287 1580 23.155 2030 2: 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 2: 250 20.154 700 21.19 1150 21.286 1600 22.958 2050 2: 260 20.213 710 21.195 1160 21.288 1610 22.868 2060 2: 270 20.267 720 21.199 1170 21.292 1620 22.785 2070 22. 280 20.315 730 21.207 1180 21.292 1630 22.707 2080 22. 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 3 300 20.41 750 21.214	-									21.538
230 20.036 680 21.181 1130 21.287 1580 23.155 2030 2: 240 20.08 690 21.188 1140 21.29 1590 23.055 2040 2: 250 20.154 700 21.19 1150 21.286 1600 22.958 2050 2: 260 20.213 710 21.195 1160 21.288 1610 22.868 2060 2: 270 20.267 720 21.199 1170 21.292 1620 22.785 2070 2: 280 20.315 730 21.207 1180 21.29 1630 22.707 2080 2: 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 3: 310 20.461 760 21.214 1200 21.291 1650 22.436 2100 2: 320 20.501 770 21.219										21.529
240 20.08 690 21.188 1140 21.29 1590 23.055 2040 2: 250 20.154 700 21.19 1150 21.286 1600 22.958 2050 2: 260 20.213 710 21.195 1160 21.288 1610 22.868 2060 2: 270 20.267 720 21.199 1170 21.292 1620 22.785 2070 22 280 20.315 730 21.207 1180 21.29 1630 22.707 2080 2: 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 3 300 20.41 750 21.214 1200 21.291 1650 22.563 2100 2: 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 2: 320 20.501 770 21.219										21.523
250 20.154 700 21.19 1150 21.286 1600 22.958 2050 22 260 20.213 710 21.195 1160 21.288 1610 22.868 2060 22 270 20.267 720 21.199 1170 21.292 1620 22.785 2070 22 280 20.315 730 21.207 1180 21.29 1630 22.707 2080 22 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 2 300 20.41 750 21.214 1200 21.291 1650 22.563 2100 2 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 2 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 2 330 20.541 780 21.223 <	-									21.514
260 20.213 710 21.195 1160 21.288 1610 22.868 2060 2: 270 20.267 720 21.199 1170 21.292 1620 22.785 2070 2: 280 20.315 730 21.207 1180 21.29 1630 22.707 2080 2: 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 2. 300 20.41 750 21.214 1200 21.291 1650 22.563 2100 2: 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 2: 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 2: 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 2: 340 20.579 790 21.229										21.506
270 20.267 720 21.199 1170 21.292 1620 22.785 2070 22 280 20.315 730 21.207 1180 21.29 1630 22.707 2080 22 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 3 300 20.41 750 21.214 1200 21.291 1650 22.563 2100 22 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 22 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 22 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 22 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22 350 20.615 800 21.228										21.504
280 20.315 730 21.207 1180 21.29 1630 22.707 2080 22 290 20.37 740 21.212 1190 21.296 1640 22.632 2090 3 300 20.41 750 21.214 1200 21.291 1650 22.563 2100 22 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 22 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 22 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 21 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22 360 20.652 810 21.233										21.491
290 20.37 740 21.212 1190 21.296 1640 22.632 2090 3 300 20.41 750 21.214 1200 21.291 1650 22.563 2100 23 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 23 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 23 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 23 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 23 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 23 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 23 370 20.685 820 21.239										21.485
300 20.41 750 21.214 1200 21.291 1650 22.563 2100 22 310 20.461 760 21.218 1210 21.292 1660 22.499 2110 22 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 22 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 22 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 22 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 25 380 20.716 830 21.24										21.485
310 20.461 760 21.218 1210 21.292 1660 22.499 2110 22.375 320 20.501 770 21.219 1220 21.296 1670 22.436 2120 22.375 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 22.323 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22.323 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22.323 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 22.323 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 22.333 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22.323 390 20.744 840										21.48
320 20.501 770 21.219 1220 21.296 1670 22.436 2120 22.33 330 20.541 780 21.223 1230 21.293 1680 22.375 2130 22.33 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22.33 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22.33 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 23.33 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 23.33 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 23.33 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 23.33 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 23.33										21.471
330 20.541 780 21.223 1230 21.293 1680 22.375 2130 21 340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 22 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 22 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22										21.466
340 20.579 790 21.229 1240 21.293 1690 22.323 2140 22.323 350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22.323 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 22.323 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 22.323 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22.323 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22.323 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22.323 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22.012										21.462
350 20.615 800 21.228 1250 21.294 1700 22.267 2150 22 360 20.652 810 21.233 1260 21.296 1710 22.219 2160 22 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 22 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22										21.457
360 20.652 810 21.233 1260 21.296 1710 22.219 2160 22.219 370 20.685 820 21.239 1270 21.295 1720 22.175 2170 22.317 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22.317 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22.317 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22.317 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22.012										21.453
370 20.685 820 21.239 1270 21.295 1720 22.175 2170 22.3380 380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22.3380 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22.3380 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22.047 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22.012										21.446
380 20.716 830 21.24 1280 21.299 1730 22.126 2180 22.33 390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22.33 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22.047 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22.012										21.446
390 20.744 840 21.241 1290 21.295 1740 22.089 2190 22.047 400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22.047 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22.012										21.443
400 20.775 850 21.245 1300 21.3 1750 22.047 2200 22.047 410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22.012										21.439
410 20.804 860 21.247 1310 21.298 1760 22.012 2210 22										21.434
										21.431
42U 2U.8Z/ 8/U 21.231 132U 21.298 1//U 21.974 222U 2.										21.431
										21.427
										21.424 21.42



YATES PZ-17S

Data Set: S:\...\17S-1.aqt

Date: 03/01/17 Time: 08:46:37

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-17S
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 27.3 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-17S)

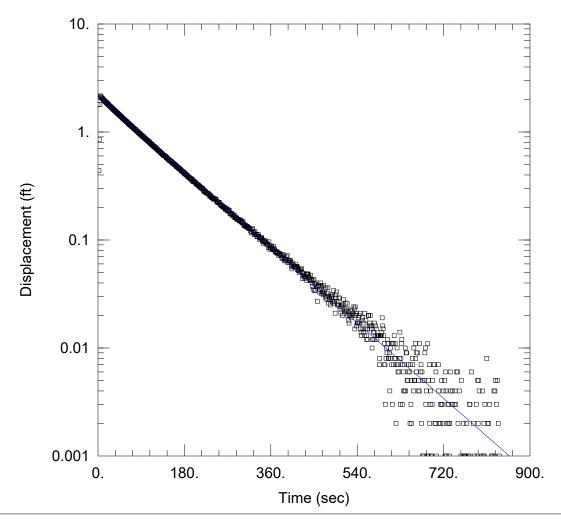
Initial Displacement: 2.37 ft Static Water Column Height: 27.3 ft

Total Well Penetration Depth: 27.15 ft Screen Length: 10. ft Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.9747 ft/day y0 = 2.434 ft



YATES PZ-17S

Data Set: S:\...\17S-2.aqt

Date: 03/01/17 Time: 08:47:43

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-17S
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 27.3 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-17S)

Initial Displacement: 1.8 ft

Static Water Column Height: 27.3 ft

Total Well Penetration Depth: 27.15 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

Casing Radius: 0.08333 ft

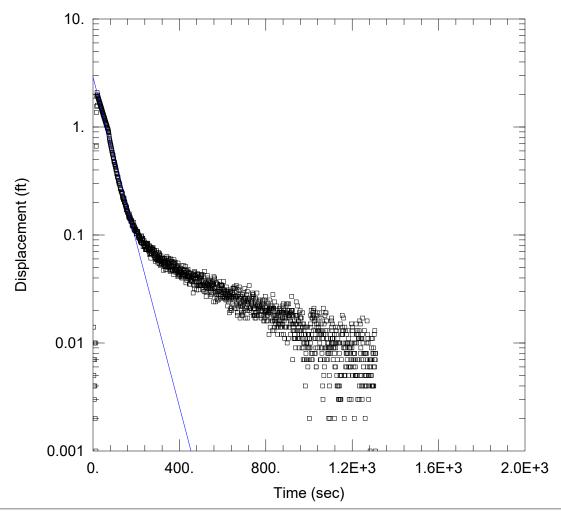
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.984 ft/day

y0 = 2.147 ft



YATES PZ-18I

Data Set: S:\...\18I-1.aqt

Date: 03/01/17 Time: 08:44:47

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-18I
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 65.27 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-18I)

Initial Displacement: 1.93 ft Static Water Column Height: 65.27 ft

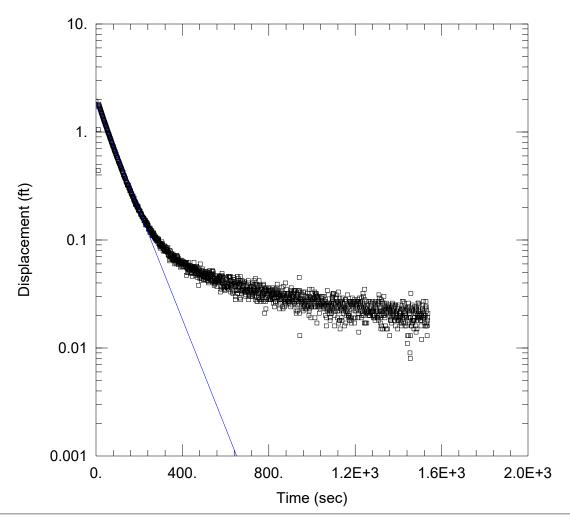
Total Well Penetration Depth: 65.3 ft Screen Length: 10. ft

Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 2.196 ft/day y0 = 2.899 ft



YATES PZ-18I

Data Set: S:\...\18I-2.aqt

Date: 03/01/17 Time: 08:43:34

PROJECT INFORMATION

Company: SCS Client: GPC Project: Yates Location: Yates Test Well: PZ-18I Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 65.27 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-18I)

Static Water Column Height: 65.27 ft Initial Displacement: 1.93 ft

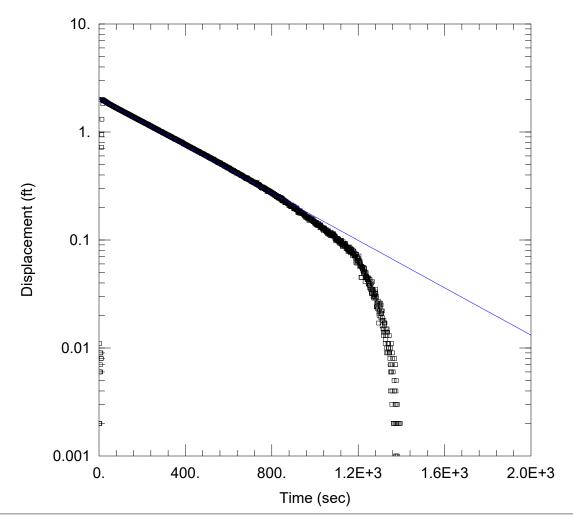
Total Well Penetration Depth: 65.3 ft Screen Length: 10. ft Casing Radius: 0.08333 ft

Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 1.449 ft/dayy0 = 1.896 ft



YATES PZ-18S

Data Set: S:\...\18S-1.aqt

Date: 03/01/17 Time: 08:51:07

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-18S
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 18.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-18S)

Initial Displacement: 2. ft

Static Water Column Height: 18.6 ft

Total Well Penetration Depth: 19.1 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

Casing Radius: 0.08333 ft

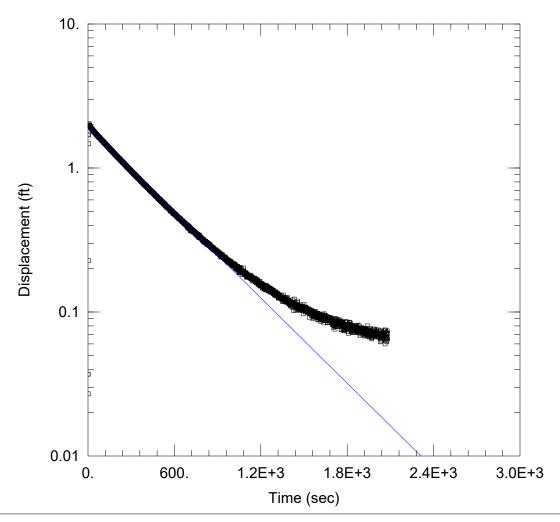
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.2674 ft/day

y0 = 2.053 ft



YATES PZ-18S

Data Set: S:\...\18S-2.aqt

Date: 03/01/17 Time: 08:53:39

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-18S
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 18.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-18S)

Initial Displacement: 1.7 ft

Static Water Column Height: 18.6 ft

Total Well Penetration Depth: 19.1 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

Casing Radius: 0.08333 ft

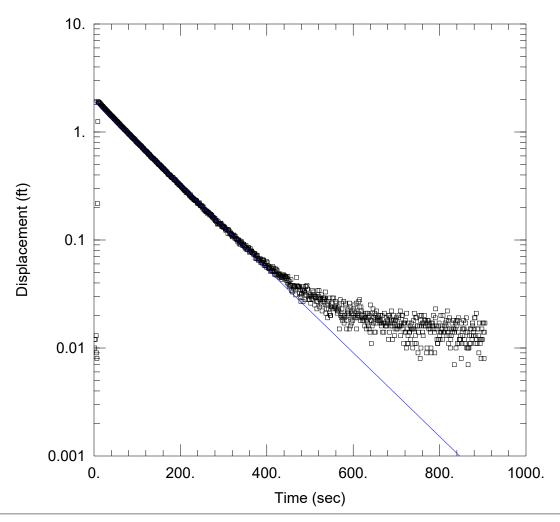
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.2407 ft/day

y0 = 1.916 ft



YATES PZ-19S

Data Set: S:\...\19S-2.aqt

Date: 03/01/17 Time: 08:56:25

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-19S
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 20.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-19S)

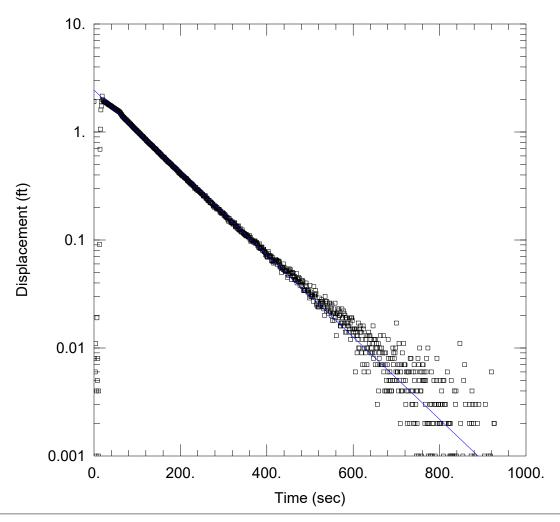
Initial Displacement: 1.87 ft Static Water Column Height: 20.6 ft

Total Well Penetration Depth: 21. ft Screen Length: 10. ft Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.9593 ft/day y0 = 1.922 ft



YATES PZ-20S

Data Set: S:\...\20S-1.aqt

Date: <u>03/01/17</u> Time: <u>08:57:38</u>

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-20S
Test Date: 11/20/2015

AQUIFER DATA

Saturated Thickness: 20.5 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-20S)

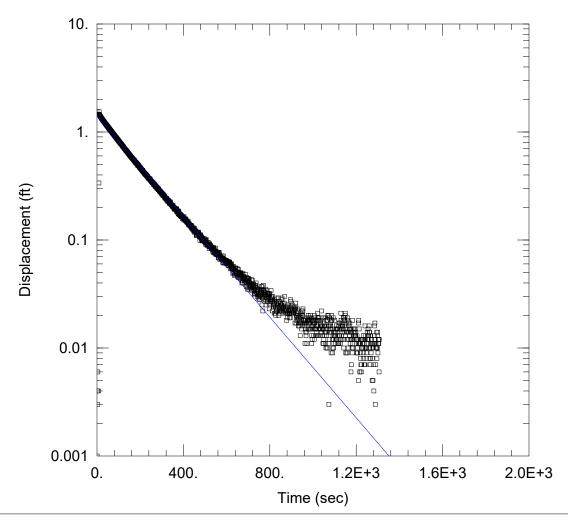
Initial Displacement: 1.91 ft Static Water Column Height: 20.5 ft

Total Well Penetration Depth: 39. ft Screen Length: 10. ft Well Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 1.028 ft/day y0 = 2.444 ft



YATES PZ-20S

Data Set: S:\...\20S-2.aqt

Date: 03/01/17 Time: 09:13:42

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-20S
Test Date: 1/13/16

AQUIFER DATA

Saturated Thickness: 20.5 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-20S)

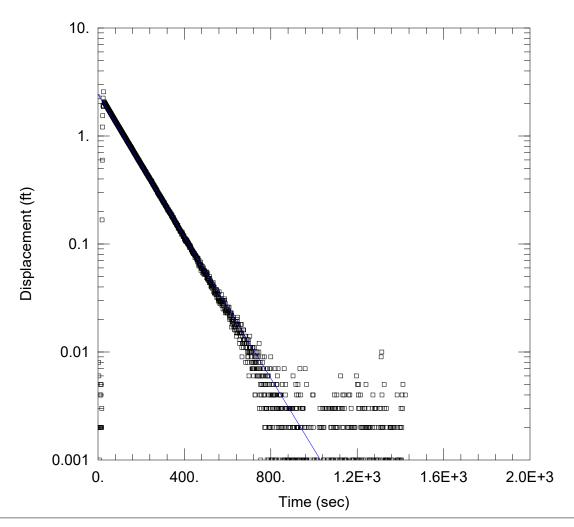
Initial Displacement: 1.54 ft Static Water Column Height: 20.5 ft

Total Well Penetration Depth: 39. ft Screen Length: 10. ft Casing Radius: 0.08333 ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.6286 ft/day y0 = 1.41 ft



YATES PZ-22S

Data Set: S:\...\22S-1.aqt

Date: 03/01/17 Time: 09:20:55

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-22S
Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 30.5 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-22S)

Initial Displacement: 2.3 ft

Static Water Column Height: 30.5 ft

Total Well Penetration Depth: 40.13 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

Casing Radius: 0.08333 ft

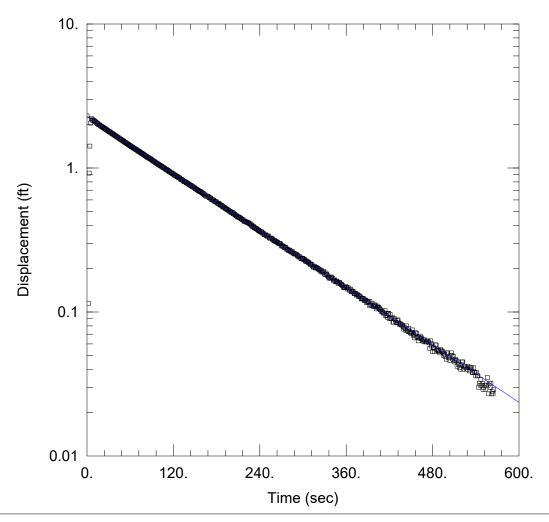
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.8904 ft/day

y0 = 2.434 ft



YATES PZ-22S

Data Set: S:\...\22S-2.aqt

Date: 03/01/17 Time: 09:22:57

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-22S
Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 30.5 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-22S)

Initial Displacement: 2.3 ft

Static Water Column Height: 30.5 ft

Total Well Penetration Depth: 30.13 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

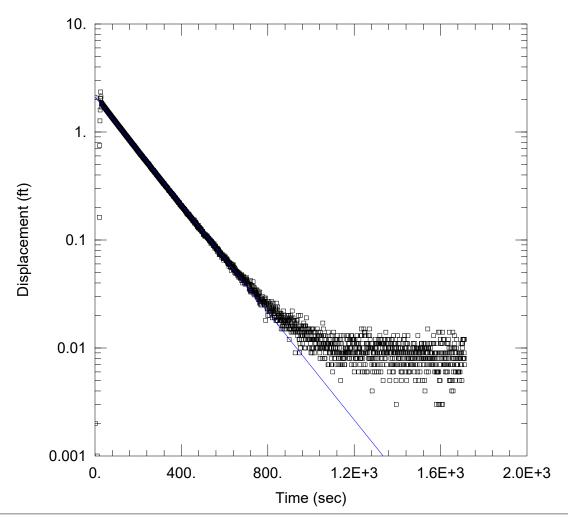
Casing Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.828 ft/day y0 = 2.28 ft



YATES PZ-23S

Data Set: S:\...\23S-1.aqt

Date: 03/01/17 Time: 09:24:54

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-23S
Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 23.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-23S)

Initial Displacement: 2.1 ft

Static Water Column Height: 23.6 ft

Total Well Penetration Depth: 23.3 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

Casing Radius: 0.08333 ft

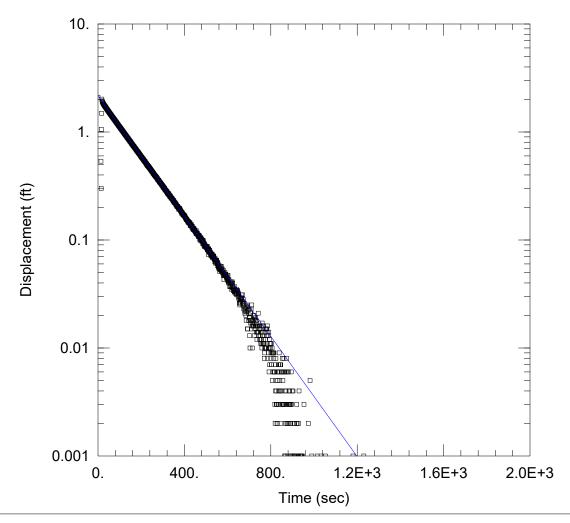
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.6054 ft/day

y0 = 2.108 ft



YATES PZ-23S

Data Set: S:\...\23S-2.aqt

Date: 03/01/17 Time: 09:25:50

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ 23S
Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 23.6 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-23S)

Initial Displacement: 2.1 ft

Static Water Column Height: 23.6 ft

Total Well Penetration Depth: 23.3 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

Casing Radius: 0.08333 ft

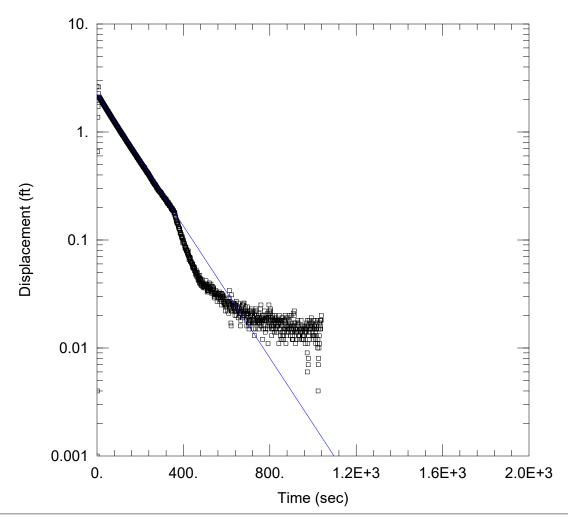
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.6763 ft/day

y0 = 2.157 ft



YATES PZ-24S

Data Set: S:\...\24S-1.aqt

Date: 03/01/17 Time: 09:28:07

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-24S
Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 29.37 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-24S)

Initial Displacement: 2.64 ft Stati

Total Well Penetration Depth: 29.4 ft

Casing Radius: 0.08333 ft

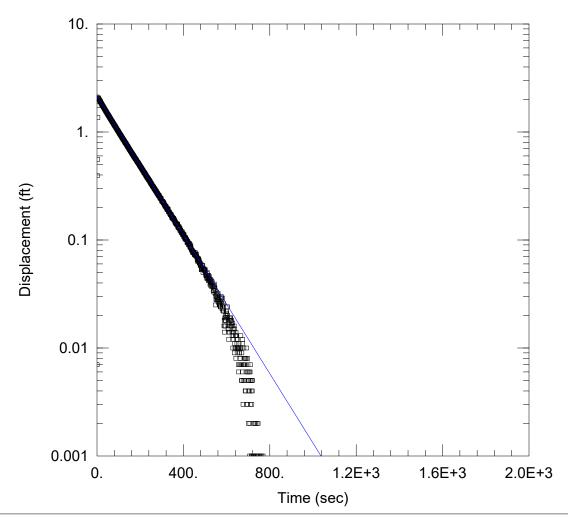
Static Water Column Height: 29.4 ft

Screen Length: 10. ft Well Radius: 0.08333 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.7924 ft/day y0 = 2.235 ft



YATES PZ-24S

Data Set: S:\...\24S-2.aqt

Date: 03/01/17 Time: 09:29:40

PROJECT INFORMATION

Company: SCS
Client: GPC
Project: Yates
Location: Yates
Test Well: PZ-24S
Test Date: 11/23/2015

AQUIFER DATA

Saturated Thickness: 29.37 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (Yates PZ-24S)

Initial Displacement: 2.11 ft

Static Water Column Height: 29.4 ft

Total Well Penetration Depth: 29.4 ft

Screen Length: 10. ft

Casing Radius: 0.08333 ft

Well Radius: 0.08333 ft

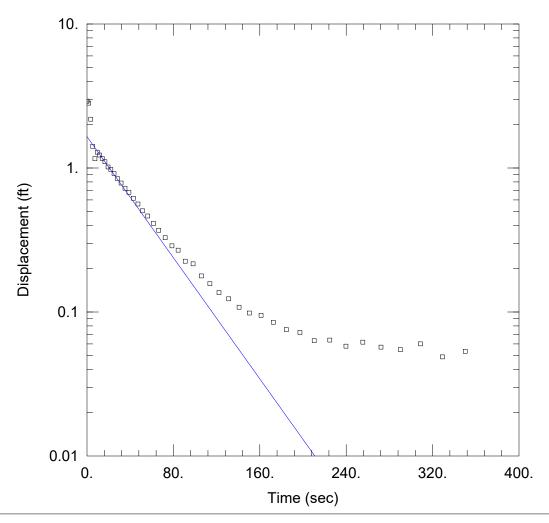
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.8348 ft/day

y0 = 2.161 ft



Data Set: P:\...\PZ-35 IN.aqt

Date: <u>03/15/17</u> Time: <u>14:12:51</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-35 IN
Test Date: 3/7/2017

AQUIFER DATA

Saturated Thickness: 36.58 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-35)

Initial Displacement: 2.91 ft

Total Well Penetration Depth: 36.58 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 36.58 ft

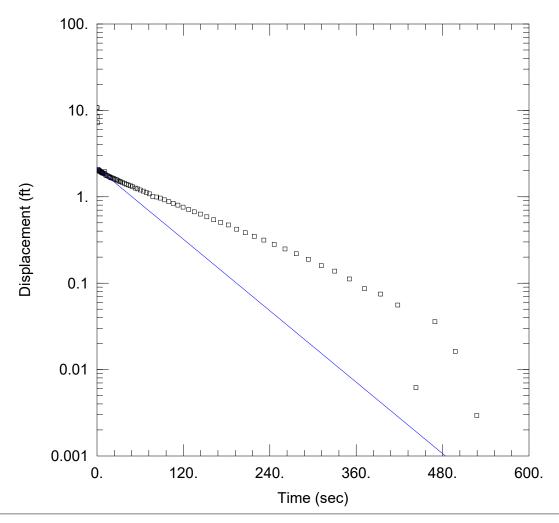
Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.003119 cm/sec y0 = 1.65 ft



Data Set: P:\...\PZ-35 OUT.aqt

Date: 03/15/17 Time: 14:13:10

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: PZ-35 OUT Test Date: 3/7/2017

AQUIFER DATA

Saturated Thickness: 36.58 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-35)

Initial Displacement: 10.76 ft

Total Well Penetration Depth: 36.58 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 36.58 ft

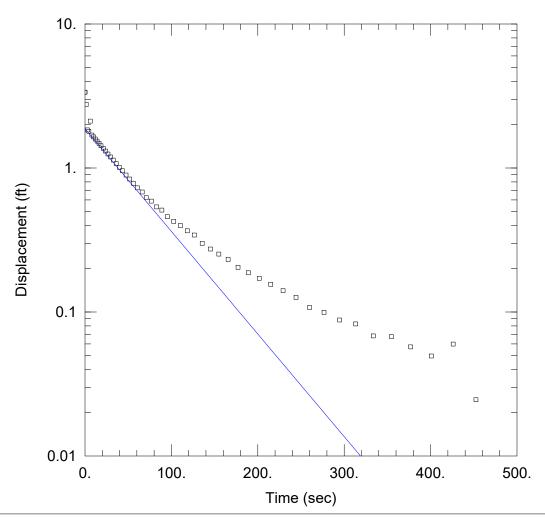
Screen Length: 10. ft Well Radius: 0.25 ft Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.002045 cm/secy0 = 2.163 ft



Data Set: P:\...\PZ-37 IN.aqt

Date: 03/15/17 Time: 14:13:25

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-37 IN
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 12.66 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-37)

Initial Displacement: 3.36 ft

Casing Radius: 0.0833 ft

Total Well Penetration Depth: 36.43 ft

Static Water Column Height: 35.43 ft

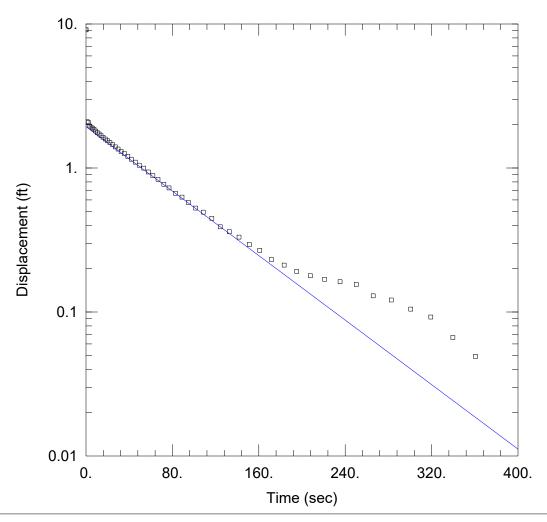
Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.002113 cm/sec y0 = 1.871 ft



Data Set: P:\...\PZ-37 OUT.aqt

Date: 03/15/17 Time: 14:13:36

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: PZ-37 OUT Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 12.66 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-37)

Initial Displacement: 9.11 ft

Total Well Penetration Depth: 35.43 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 35.43 ft

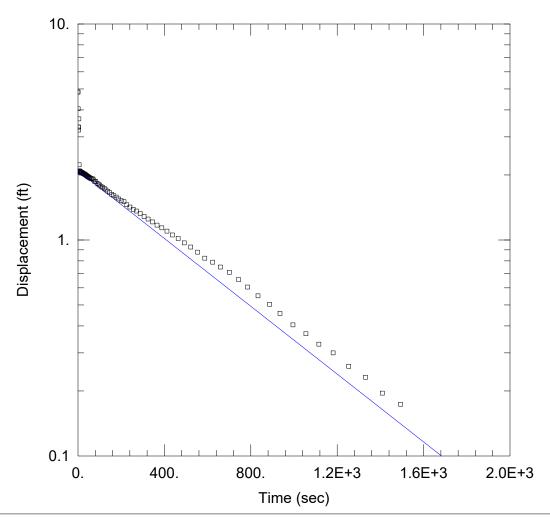
Screen Length: 10. ft Well Radius: 0.25 ft Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.001653 cm/secy0 = 1.938 ft



Data Set: P:\...\PZ-38 IN.aqt

Date: 03/15/17 Time: 14:13:49

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: PZ-38 IN Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 19.02 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-38)

Initial Displacement: 4.84 ft

Total Well Penetration Depth: 19.02 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 19.02 ft

Screen Length: 10. ft Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

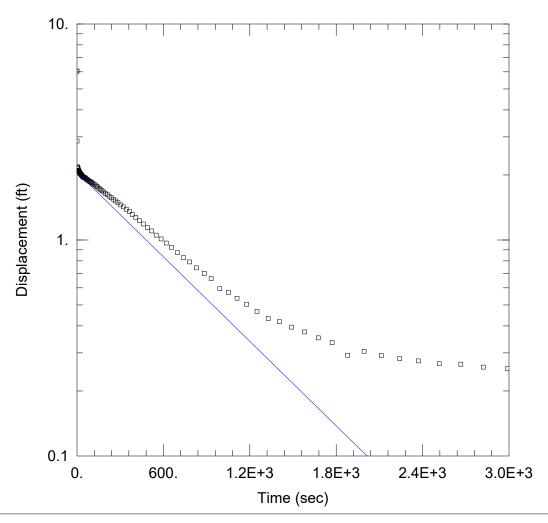
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0002079 cm/sec

y0 = 2.093 ft



Data Set: P:\...\PZ-38 OUT.aqt

Date: 03/15/17 Time: 14:14:09

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-38 OUT
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 19.02 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-38)

Initial Displacement: 6.05 ft

Total Well Penetration Depth: 19.02 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 19.02 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Cravel Back Borosity: 0

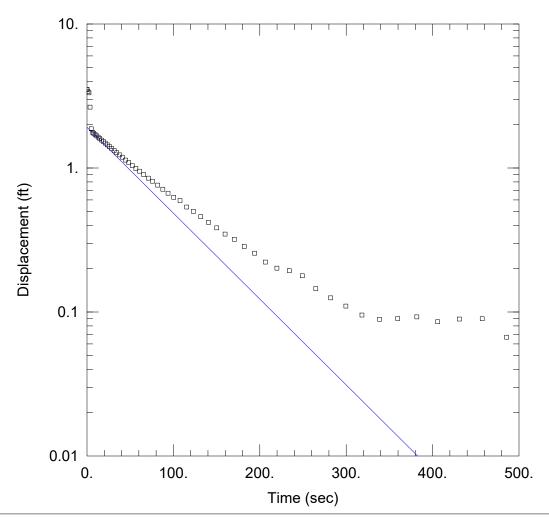
Gravel Pack Porosity: <u>0.3</u>

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0001727 cm/sec y0 = 2.056 ft



Data Set: P:\...\PZ-39 IN.aqt

Date: 03/15/17 Time: 14:14:25

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-39 IN
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 42.21 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-39)

Initial Displacement: 3.52 ft

Total Well Penetration Depth: 42.21 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 42.21 ft

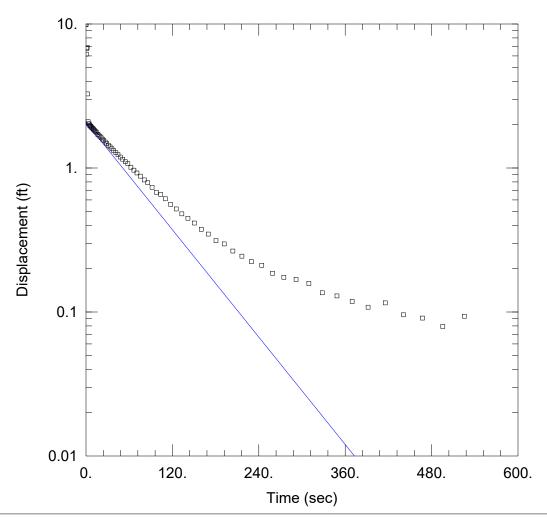
Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.00181 cm/sec y0 = 1.917 ft



Data Set: P:\...\PZ-39 OUT.aqt

Date: 03/15/17 Time: 14:14:44

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-39 OUT
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 42.21 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-39)

Initial Displacement: 9.91 ft

Total Well Penetration Depth: 42.21 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 42.21 ft

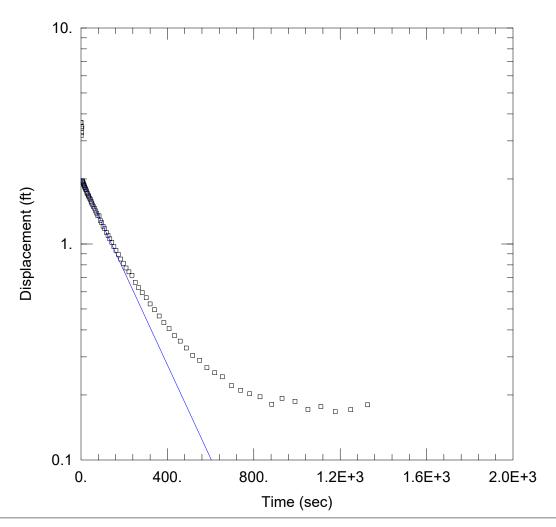
Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.001885 cm/sec y0 = 2.074 ft



Data Set: P:\...\PZ-40 IN.aqt

Date: <u>03/15/17</u> Time: <u>14:15:02</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-40 IN
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 20.13 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-40)

Initial Displacement: 3.66 ft

Total Well Penetration Depth: 20.13 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 20.13 ft

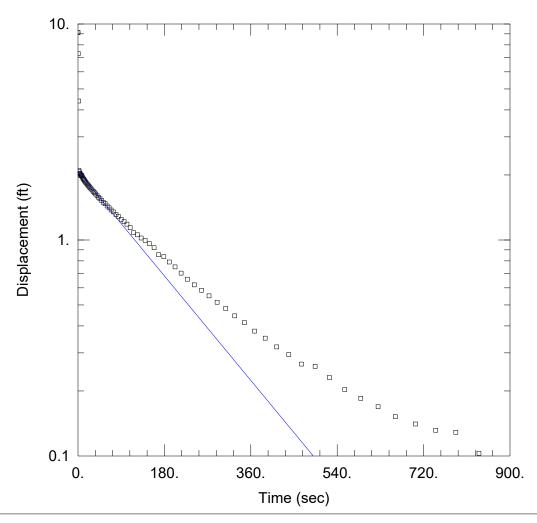
Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0005789 cm/sec y0 = 2.019 ft



Data Set: P:\...\PZ-40 OUT.aqt

Date: <u>03/15/17</u> Time: <u>14:15:18</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: PZ-40 OUT
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 20.13 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-40)

Initial Displacement: 9.12 ft

Total Well Penetration Depth: 20.13 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 20.13 ft

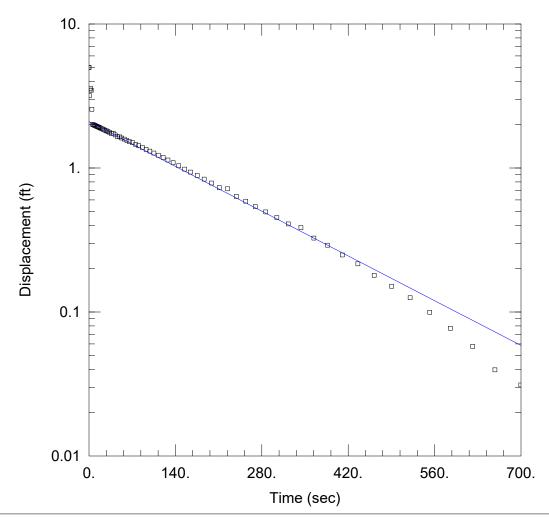
Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0007197 cm/sec y0 = 2.077 ft



Data Set: P:\...\PZ-48 IN.aqt

Date: 03/15/17 Time: 14:16:14

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: PZ-48 IN Test Date: 3/7/2017

AQUIFER DATA

Saturated Thickness: 34.93 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-48)

Initial Displacement: 4.99 ft

Total Well Penetration Depth: 34.93 ft

Static Water Column Height: 34.93 ft

Casing Radius: 0.0833 ft

Screen Length: 10. ft Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

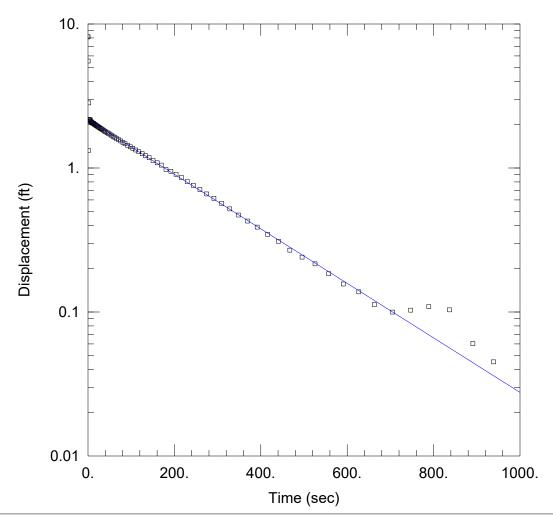
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0006539 cm/sec

y0 = 2.098 ft



Data Set: P:\...\PZ-48 OUT.aqt

Date: 03/15/17 Time: 14:16:27

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: PZ-48 OUT Test Date: 3/7/2017

AQUIFER DATA

Saturated Thickness: 34.93 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (PZ-48)

Initial Displacement: 8.14 ft

Total Well Penetration Depth: 34.93 ft

Casing Radius: 0.0833 ft

Static Water Column Height: 34.93 ft

Screen Length: 10. ft Well Radius: 0.25 ft Gravel Pack Porosity: 0.3

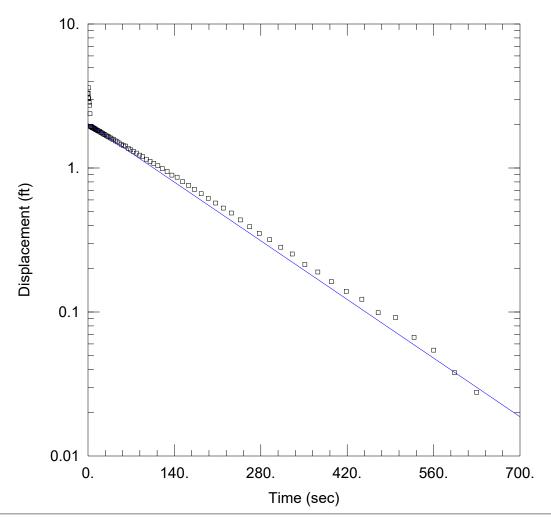
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0005577 cm/sec

y0 = 2.162 ft



Data Set: P:\...\YGWA-47 IN.aqt

Date: 03/15/17 Time: 14:16:43

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: YGWA-47 IN Test Date: 3/3/2017

AQUIFER DATA

Saturated Thickness: 27.53 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWA-47)

Initial Displacement: 3.31 ft

Static Water Column Height: 27.53 ft

Total Well Penetration Depth: 27.53 ft Casing Radius: 0.0833 ft

Screen Length: 10. ft Well Radius: 0.25 ft

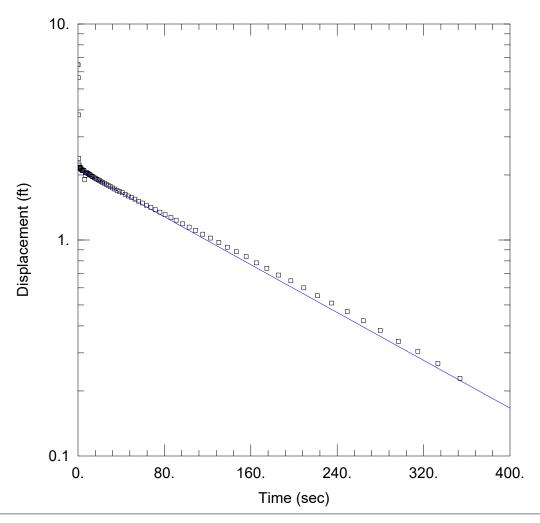
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0008235 cm/secy0 = 2.036 ft



Data Set: P:\...\YGWA-47 OUT.aqt

Date: 03/15/17 Time: 14:16:59

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: YGWA-47 OUT Test Date: 3/3/2017

AQUIFER DATA

Saturated Thickness: 27.53 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWA-47)

Initial Displacement: 6.47 ft

Static Water Column Height: 27.53 ft

Total Well Penetration Depth: 27.53 ft

Screen Length: 10. ft Well Radius: 0.25 ft

Casing Radius: 0.0833 ft

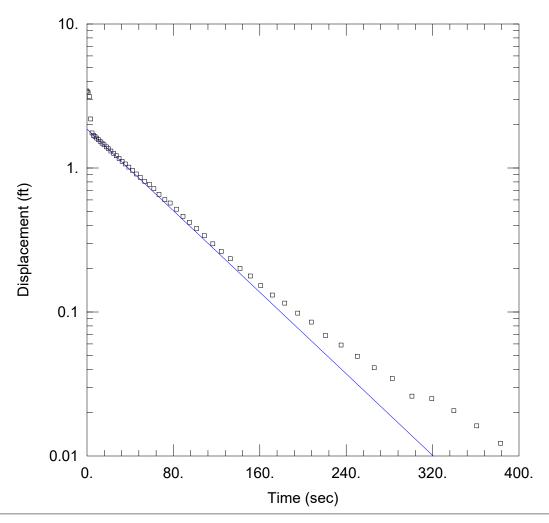
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0007836 cm/secy0 = 2.131 ft



Data Set: P:\...\YGWC-32I IN.aqt

Date: <u>03/15/17</u> Time: <u>14:19:20</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-32I IN
Test Date: 3/9/2017

AQUIFER DATA

Saturated Thickness: 19.03 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-32I)

Initial Displacement: 3.42 ft Static Water Column Height: 19.03 ft

Total Well Penetration Depth: 19.03 ft

Casing Radius: 0.0833 ft

Screen Length: 10. ft

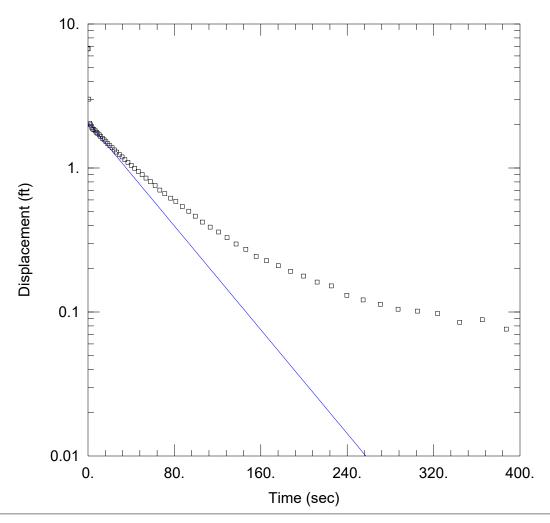
Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.001876 cm/sec y0 = 1.86 ft



Data Set: P:\...\YGWC-32I OUT.aqt

Date: <u>03/15/17</u> Time: <u>14:19:37</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-32I OUT

Test Date: 3/9/2017

AQUIFER DATA

Saturated Thickness: 19.03 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-32I)

Initial Displacement: 6.72 ft

Total Well Penetration Depth: 19.03 ft

Static Water Column Height: 19.03 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

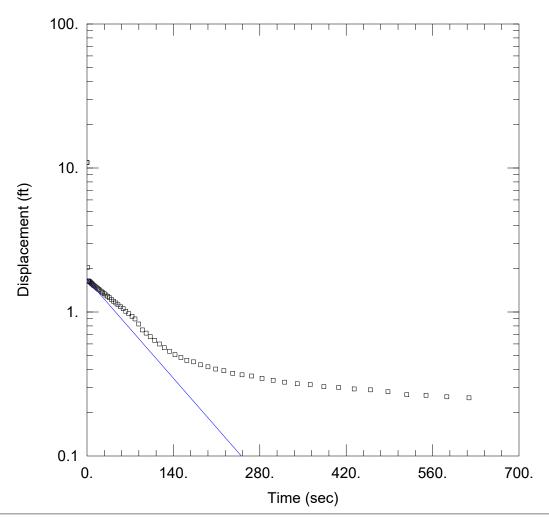
SOLUTION

Aquifer Model: Unconfined

Casing Radius: 0.0833 ft

Solution Method: Bouwer-Rice

K = 0.002387 cm/sec y0 = 2.079 ft



Data Set: P:\...\YGWC-32S IN.aqt

Date: 03/15/17 Time: 14:19:56

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-32S IN
Test Date: 3/9/2017

AQUIFER DATA

Saturated Thickness: 7.03 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-32S)

Initial Displacement: 10.91 ft

Static Water Column Height: 7.03 ft

Total Well Penetration Depth: 7.03 ft

Screen Length: 7.03 ft Well Radius: 0.25 ft

Casing Radius: 0.0833 ft

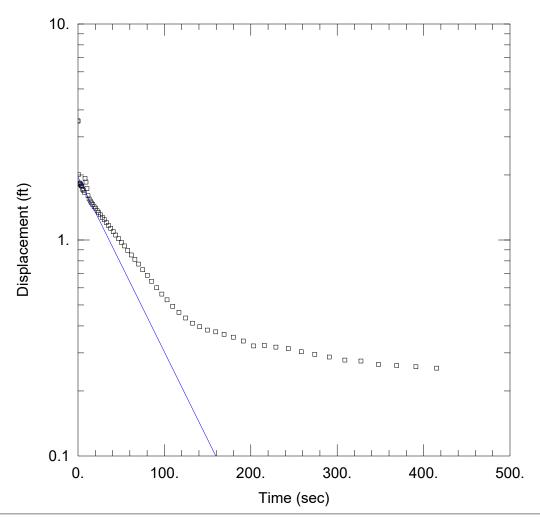
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.001447 cm/sec y0 = 1.683 ft



Data Set: P:\...\YGWC-32S OUT.aqt

Date: 03/15/17 Time: 14:20:09

PROJECT INFORMATION

Company: <u>ACC</u>
Project: <u>I054-104</u>
Location: Plant Yates

Test Well: YGWC-32S OUT

Test Date: 3/9/2017

AQUIFER DATA

Saturated Thickness: 7.03 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-32S)

Initial Displacement: 3.56 ft

Total Well Penetration Depth: 7.03 ft Screen Length: 7.03 ft Wall Parlings 0.0003 ft

Casing Radius: 0.0833 ft Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

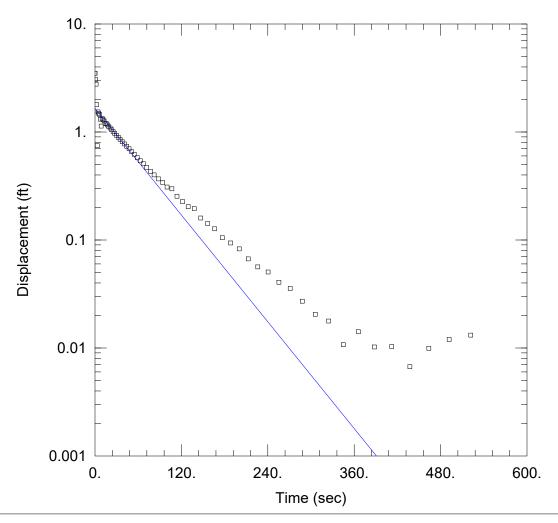
Gravel Pack Porosity: <u>0.3</u>

Static Water Column Height: 7.03 ft

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.00239 cm/sec y0 = 1.956 ft



Data Set: P:\...\YGWC-33S IN.aqt

Date: <u>03/15/17</u> Time: <u>14:20:26</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-33S IN
Test Date: 3/7/2017

AQUIFER DATA

Saturated Thickness: 30.12 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-33S)

Initial Displacement: 3.5 ft Static Water Column Height: 30.12 ft

Total Well Penetration Depth: 30.12 ft

Casing Radius: 0.0833 ft

Screen Length: 10. ft

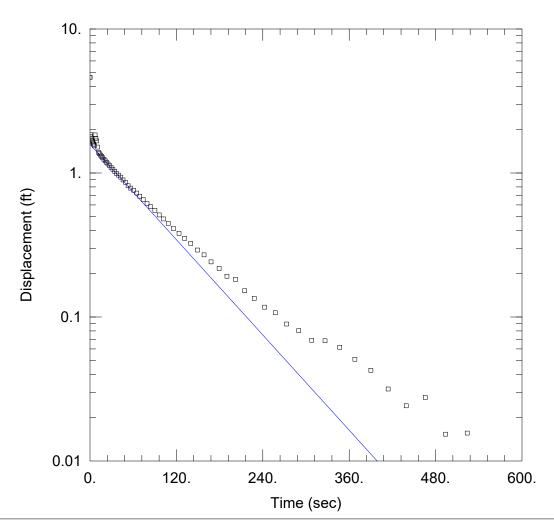
Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.002368 cm/sec y0 = 1.657 ft



Data Set: P:\...\YGWC-33S OUT.aqt

Date: 03/15/17 Time: 14:20:44

PROJECT INFORMATION

Company: <u>ACC</u>
Project: <u>I054-104</u>
Location: Plant Yates

Test Well: YGWC-33S OUT

Test Date: 3/7/2017

AQUIFER DATA

Saturated Thickness: 30.12 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-33S)

Initial Displacement: 4.62 ft Static Water Column Height: 30.12 ft

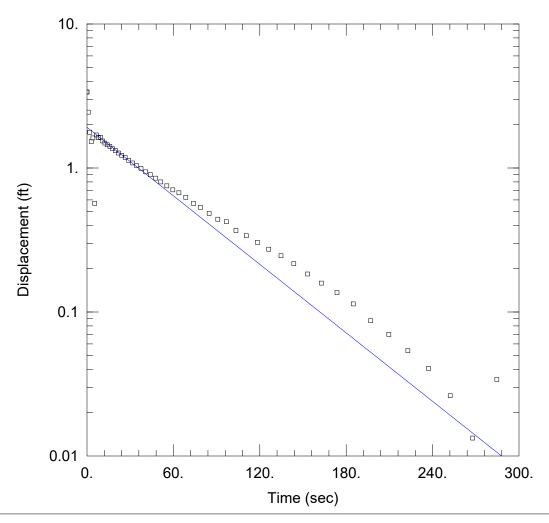
Total Well Penetration Depth: 30.12 ft
Casing Radius: 0.0833 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.001584 cm/sec y0 = 1.577 ft



Data Set: P:\...\YGWC-34I IN.aqt

Date: <u>03/15/17</u> Time: <u>14:21:01</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-341 IN
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 14.25 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-34I)

Initial Displacement: 3.37 ft Static Water Column Height: 14.25 ft

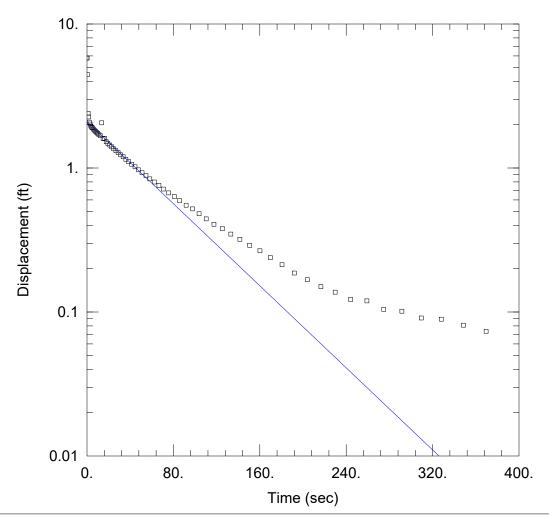
Total Well Penetration Depth: 14.25 ft Screen Length: 10. ft Screen Length: 0.0833 ft Well Radius: 0.25 ft Screen Length: 0.25 ft Screen

Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.001985 cm/sec y0 = 1.915 ft



Data Set: P:\...\YGWC-34I OUT.aqt

Date: <u>03/15/17</u> Time: <u>14:21:19</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-34I OUT
Test Date: 3/8/2017

AQUIFER DATA

Saturated Thickness: 14.25 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-34I)

Initial Displacement: 5.8 ft Static Water Column Height: 14.25 ft

Total Well Penetration Depth: 14.25 ft

Casing Radius: 0.0833 ft

Screen Length: 10. ft

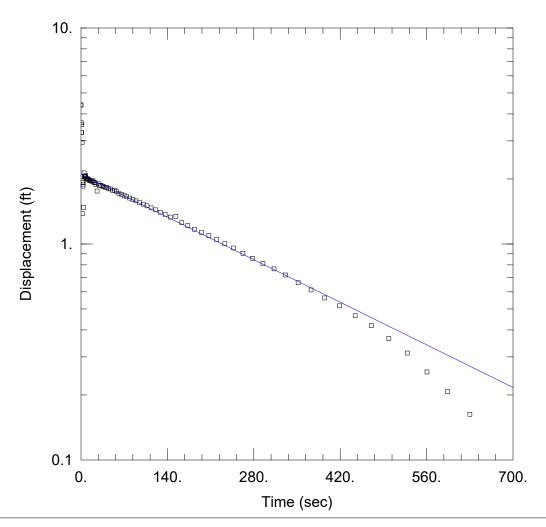
Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.001784 cm/sec y0 = 2.098 ft



Data Set: P:\...\YGWC-36 IN.aqt

Date: 03/15/17 Time: 14:21:36

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-36 IN
Test Date: 3/6/2017

AQUIFER DATA

Saturated Thickness: 45.56 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-36)

Initial Displacement: 4.4 ft

Ift Static Water Column Height: 45.56 ft

Total Well Penetration Depth: 45.56 ft Casing Radius: 0.0833 ft

Screen Length: 10. ft
Well Radius: 0.25 ft

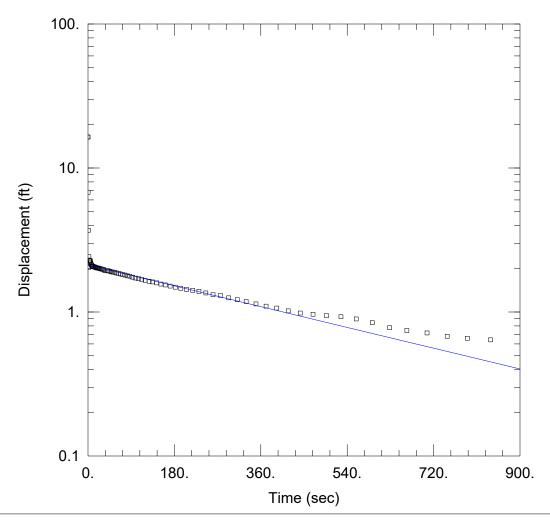
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0004323 cm/sec y0 = 2.097 ft



Data Set: P:\...\YGWC-36 OUT.aqt

Date: 03/15/17 Time: 14:21:55

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-36 OUT
Test Date: 3/6/2017

AQUIFER DATA

Saturated Thickness: 45.56 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-36)

SOLUTION

Initial Displacement: 16.41 ft

Static Water Column Height: 45.56 ft

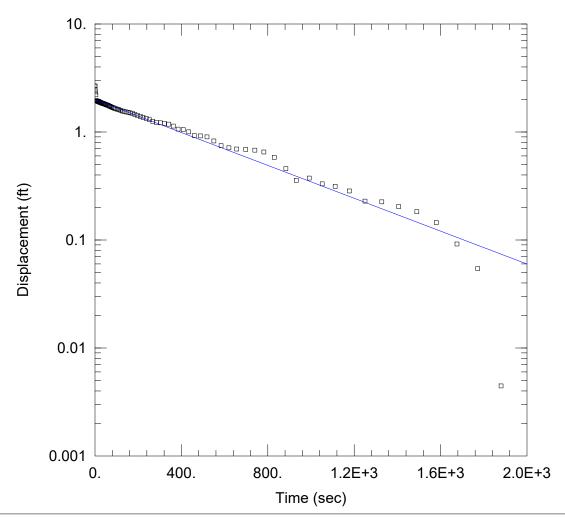
Total Well Penetration Depth: 45.56 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

Casing Radius: 0.0833 ft

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.000246 cm/sec y0 = 2.116 ft



Data Set: P:\...\YGWC-42 IN.aqt

Date: <u>03/15/17</u> Time: <u>14:22:11</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-42 IN
Test Date: 3/6/2017

AQUIFER DATA

Saturated Thickness: 31.41 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-42)

Initial Displacement: 2.67 ft

Static Water Column Height: 21.41 ft

Total Well Penetration Depth: 31.41 ft Casing Radius: 0.0833 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Cravel Back Parasity: 0

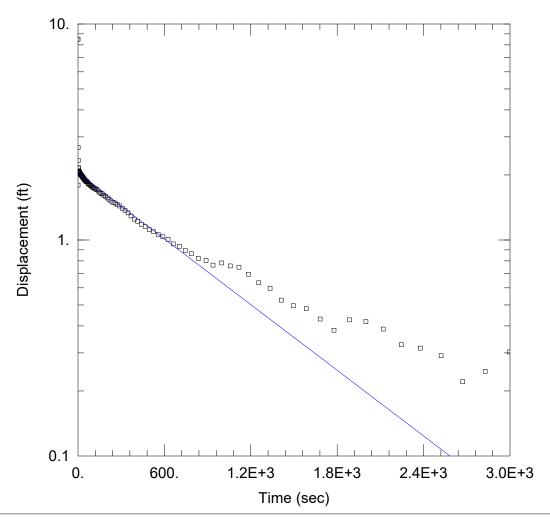
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0002203 cm/sec y0 = 1.992 ft



Data Set: P:\...\YGWC-42 OUT.aqt

Date: <u>03/15/17</u> Time: <u>14:22:47</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-42 OUT
Test Date: 3/6/2017

AQUIFER DATA

Saturated Thickness: 31.41 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-42)

Initial Displacement: 8.5 ft

31 41 ft Screen Len

Static Water Column Height: 31.41 ft

Total Well Penetration Depth: 31.41 ft

Screen Length: 10. ft Well Radius: 0.25 ft

Casing Radius: 0.0833 ft

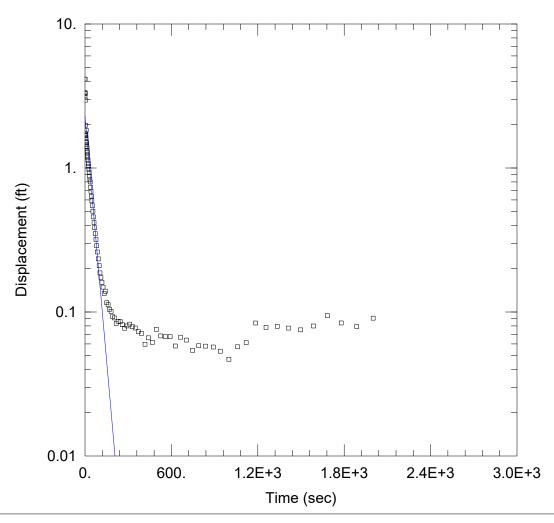
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0001467 cm/sec y0 = 2.036 ft



Data Set: P:\...\YGWC-43 IN.aqt

Date: 03/15/17 Time: 14:23:00

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-43 IN
Test Date: 3/2/2017

AQUIFER DATA

Saturated Thickness: 55.59 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-43)

Initial Displacement: 4.14 ft Static Water Column Height: 65.59 ft

Total Well Penetration Depth: 65.59 ft

Casing Radius: 0.0833 ft

Screen Length: 10. ft

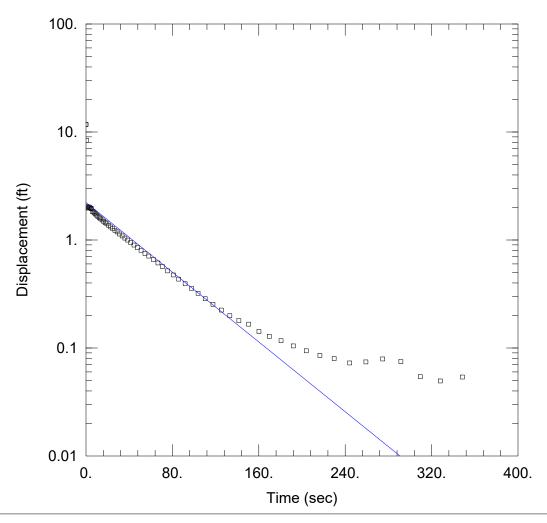
Well Radius: 0.25 ft

Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined Solution Method: Bouwer-Rice

K = 0.003663 cm/sec y0 = 2.293 ft



Data Set: P:\...\YGWC-43 OUT.aqt

Date: 03/15/17 Time: 14:23:17

PROJECT INFORMATION

Company: ACC Project: 1054-104 Location: Plant Yates Test Well: YGWC-43 OUT Test Date: 3/2/2017

AQUIFER DATA

Saturated Thickness: 65.59 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-43)

Initial Displacement: 11.74 ft

Static Water Column Height: 65.59 ft

Total Well Penetration Depth: 65.59 ft Casing Radius: 0.0833 ft

Screen Length: 10. ft Well Radius: 0.25 ft

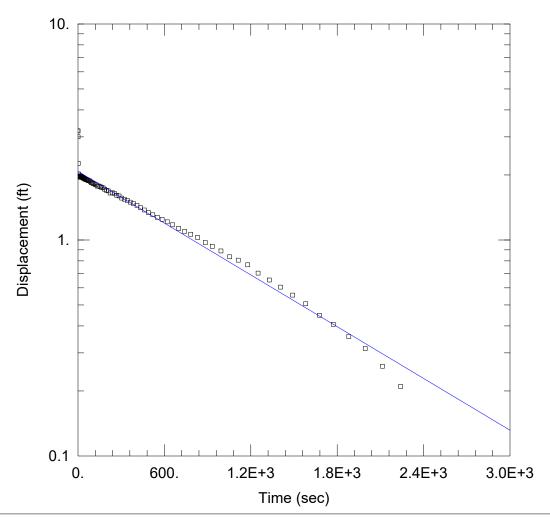
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.002614 cm/secy0 = 2.228 ft



Data Set: P:\...\YGWC-44 IN.aqt

Date: 03/15/17 Time: 14:23:30

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-44 IN
Test Date: 3/2/2017

AQUIFER DATA

Saturated Thickness: 39.39 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-44)

Initial Displacement: 3.2 ft

Static Water Column Height: 39.39 ft

Total Well Penetration Depth: 39.39 ft

Screen Length: 10. ft
Well Radius: 0.25 ft

Casing Radius: 0.0833 ft

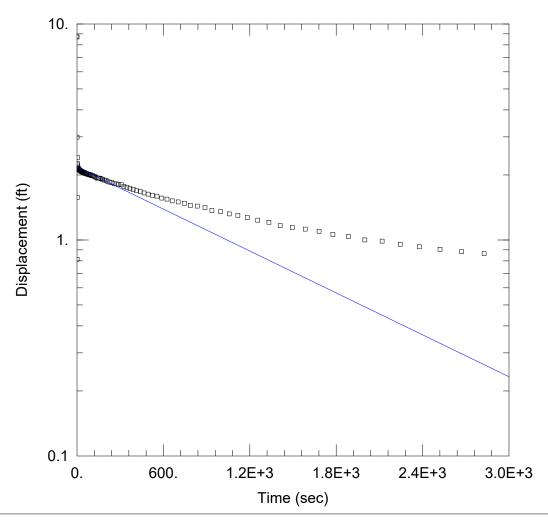
Gravel Pack Porosity: 0.3

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0001199 cm/sec y0 = 2.078 ft



Data Set: P:\...\YGWC-44 OUT.aqt

Date: 03/15/17 Time: 14:23:48

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-44 OUT
Test Date: 3/2/2017

AQUIFER DATA

Saturated Thickness: 39.39 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-44)

Initial Displacement: 8.71 ft

Static Water Column Height: 39.39 ft

Total Well Penetration Depth: 39.39 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

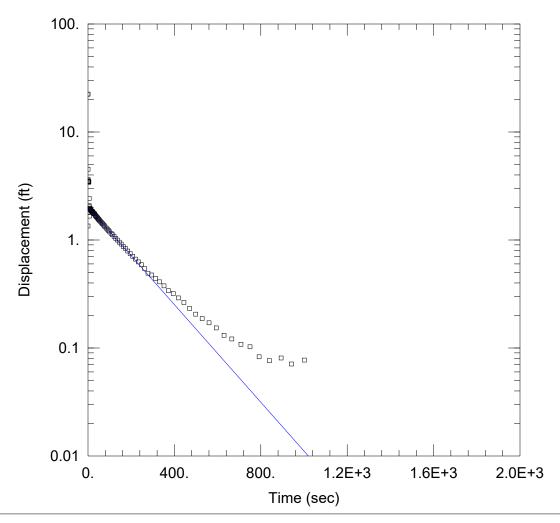
Casing Radius: 0.0833 ft

SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 9.707E-5 cm/sec y0 = 2.172 ft



Data Set: P:\...\YGWC-49 IN.aqt

Date: <u>03/15/17</u> Time: <u>14:25:07</u>

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-49 IN
Test Date: 3/6/2017

AQUIFER DATA

Saturated Thickness: 44.89 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-49)

Initial Displacement: 22.38 ft

Static Water Column Height: 44.89 ft

Total Well Penetration Depth: 44.89 ft

Screen Length: 10. ft Well Radius: 0.25 ft

Casing Radius: 0.0833 ft

Gravel Pack Porosity: 0.3

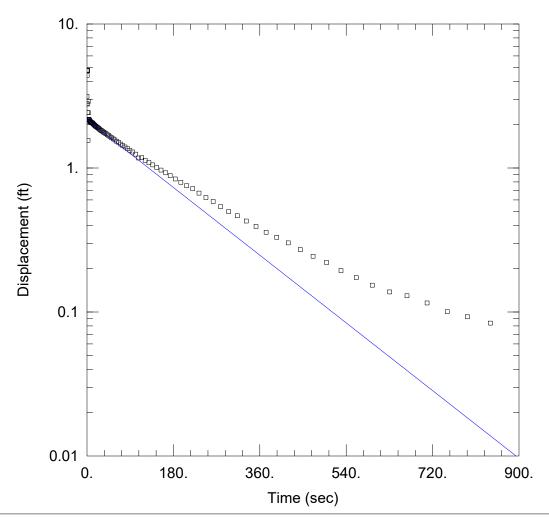
SOLUTION

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0006917 cm/sec

y0 = 2.015 ft



Data Set: P:\...\YGWC-49 OUT.aqt

Date: 03/15/17 Time: 14:25:20

PROJECT INFORMATION

Company: ACC
Project: 1054-104
Location: Plant Yates
Test Well: YGWC-49 OUT
Test Date: 3/6/2017

AQUIFER DATA

Saturated Thickness: 44.89 ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (YGWC-49)

SOLUTION

Initial Displacement: 4.74 ft

Static Water Column Height: 44.89 ft

Total Well Penetration Depth: 44.89 ft

Screen Length: 10. ft
Well Radius: 0.25 ft
Gravel Pack Porosity: 0.3

Casing Radius: 0.0833 ft

Aquifer Model: Unconfined

Solution Method: Bouwer-Rice

K = 0.0008 cm/sec y0 = 2.162 ft



October 27, 2015

Bart Smelser **Southern Company Services, Inc.** 299 Logan Martin Village Road Vincent, AL 35178 205-438-5893 direct

Subject: Laboratory Testing Results

Plant Yates Piezometers Geotechnical Investigation Cardno Project Number Z003000203

Mr. Smelser:

Cardno ATC has completed the soils testing for the Shelby Tube samples collected from the Plant Yates Piezometers location. These samples were collected by Southern Company Services, Inc. and delivered to the Cardno ATC laboratory in Alabaster, AL by members of Cardno staff. This work was conducted in accordance with the master agreement between Cardno ATC and Southern Company Affiliates, dated February 28, 2014, and detailed in the Work Authorization dated September 23, 2015.

The purpose of this letter is to report the results of the laboratory testing which are detailed in the following pages.

Cardno ATC sincerely appreciates the opportunity to work with you on this project. If you have any questions or if we may be of further service to you, please contact us.

Respectfully Submitted,

Cardno ATC

Brian A. White, CET Laboratory Supervisor Cardno ATC

Direct Line +1 205 624 1870

Email: gauen.alexander@cardno.com

Enclosures: laboratory report

Fred R. DeLeon, Jr., P.E., P.G.

Principal Engineer Cardno ATC

Direct Line +1 205 624 1876

Email: fred.deleon@cardno.com



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200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775 Fax: 205-733-8954

SUMMARY OF LABORATORY RESULTS

PAGE 1 OF 1

CLIENT Southern Company Services

PROJECT NAME Plant Yates Piezometers

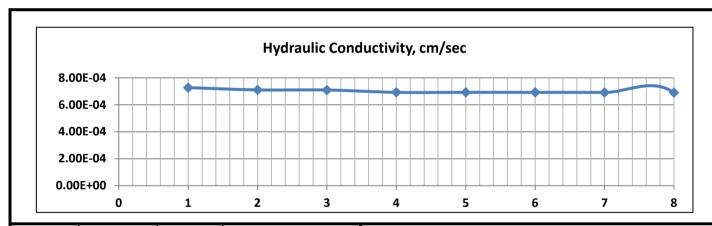
PROJECT NUMBER	Z003000	203		PI	ROJEC	T LOCA	TION	Newn	an, GA

Borehole	Depth	Liquid Limit	Plastic Limit	Plasticity Index	Maximum Size (mm)	%<#200 Sieve	Class- ification	Water Content (%)	Dry Density (pcf)	Specific Gravity	Void Ratio
PZ-17s	17.0	NP	NP	NP	4.75	21.2	SM-SC			2.665	
PZ-19s	17.0	NP	NP	NP	9.5	42.0	SM-SC			2.681	
PZ-20s	17.0	NP	NP	NP	4.75	28.9	SM-SC			2.665	
PZ-22s	7.0	NP	NP	NP	9.5	20.3	SM-SC			2.731	
PZ-22s	17.0	NP	NP	NP	9.5	28.0	SM-SC			2.717	
PZ-24s	17.0	NP	NP	NP	19	15.3	SM-SC			2.693	
PZ-24s	37.0	NP	NP	NP	4.75	22.0	SM-SC			2.701	
型 PZ-25s	33.0	NP	NP	NP	9.5	23.4	SM-SC			2.678	
PZ-25s	44.0	NP	NP	NP	19	22.3	SM-SC			2.682	
PZ-26s	17.0	37	27	10	4.75	57.9	ML			2.741	
₹ PZ-26s	27.0	NP	NP	NP	4.75	33.7	SM-SC			2.720	
₹ PZ-27s	17.0	39	30	9	4.75	73.5	ML			2.661	
PZ-27s	27.0	NP	NP	NP	2	45.0	SM-SC			2.673	
PZ-28s	17.0	NP	NP	NP	19	18.9	SM-SC			2.578	
PZ-30s	s 27.0 NP NP NP		4.75	16.6	SM-SC			2.710			
PZ-24s PZ-24s PZ-24s PZ-24s PZ-24s PZ-24s PZ-25s PZ-25s PZ-26s PZ-26s PZ-27s PZ-27s PZ-27s PZ-27s PZ-27s PZ-28s PZ-30s PZ-31s	7.0	NP	NP	NP	4.75	16.7	SM-SC			2.653	

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-17s (17'-19')
Sample Location :	PZ-17S (17'-19') UD-01	Date Sampled:	09/10/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial W	orking	Final Sample Conditions	
Wet Density, pcf	119.8	Pressure	s, psi	Wet Density, pcf	119.6
Dry Density, pcf	98.0	Chamber	92	Dry Density, pcf	98.1
Moisture Content, %	22.2	Head	79	Moisture Content, %	22.0
Void ratio, e	0.697	Tail	77	Void ratio, e	0.695
Porosity, n	0.411	Conso.	14	Porosity, n	0.410
Saturation, Percent	84.9	Soil Specifi	c Gravity	Saturation, Percent	84.2
Hydraulic Gradient, i	9.8	Gs	2.665	Hydraulic Gradient, i	7.1
Sample Length, Inches	5.668	Proctor Re	ferenced	Sample Length, Inches	5.650
Sample Volume, cc	584.4856	-	-	Sample Volume, cc	583.5545
B-value :	97	.0%	Sample Co	nsolidated During Saturation, %	0.32%



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.08	1.9180	7.27E-04	20
2	0.17	1.8429	7.10E-04	20
3	0.25	1.7692	7.10E-04	20
4	0.33	1.7053	6.92E-04	20
5	0.42	1.6385	6.92E-04	20
6	0.50	1.5746	6.92E-04	20
7	0.58	1.5134	6.92E-04	20
8	0.67	1.4550	6.91E-04	20
9	0.00			20

Hydraulic Conductivity, cm/sec 6.91E-04



GRAIN SIZE DISTRIBUTION



100

200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 1/23/8 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 HYDROMETER 100 95 90 85 80 75 (TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY,PLANT YATES PIEZOMETERS\G\NT\PLANT YATES PIEZOMETERS.GP 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5

	COBE	DIEC	GRA	VEL		SAND)	SILT OD CLAV									
	COBE	DLES	coarse	coarse fine coarse medium fine							SILT OR CLAY						
BORE	HOLE	DEPT	Н		Cla	ssification			LL	PL	PI	Сс					

GRAIN SIZE IN MILLIMETERS

0.1

0.01

10

. S:\	•	PZ-17S	17		S	ILTY SAND	NP	NP	NP				
11:25													
10/27/15													
- 1													
AB.GDT													
US LAB	В	OREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand	ı	%Silt	%C	Clay
STDU	•	PZ-17S	17	4.75	0.329	0.117		0.0	78.8		2	1.2	
GINT													
- 1													
AIN SIZE													

0.001

Cu

Cardno ATC

200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

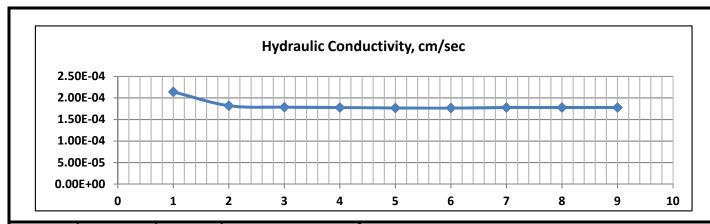
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:26 - S.\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY/PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GEV. 30 I N D E X 20 10 CL-ML (ML) (MH) 20 40 100 60 80 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-17S NP NP NP SILTY SAND (SM-SC) 17 21

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-19S (17'-19')
Sample Location :	PZ-19S (17'-19') UD-01	Date Sampled:	09/21/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial W	orking	Final Sample Conditions	
Wet Density, pcf	112.9	Pressure	s, psi	Wet Density, pcf	110.9
Dry Density, pcf	85.5	Chamber	87	Dry Density, pcf	85.5
Moisture Content, %	32.1	Head	78	Moisture Content, %	29.7
Void ratio, e	0.956	Tail	76	Void ratio, e	0.956
Porosity, n	0.489	Conso.	10	Porosity, n	0.489
Saturation, Percent	89.9	Soil Specifi	c Gravity	Saturation, Percent	83.3
Hydraulic Gradient, i	9.9	Gs	2.681	Hydraulic Gradient, i	8.4
Sample Length, Inches	5.613	Proctor Re	ferenced	Sample Length, Inches	5.613
Sample Volume, cc	582.1245	-	-	Sample Volume, cc	582.1245
B-value :	98	.0%	Sample Co	onsolidated During Saturation, %	0.00%



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.17	1.9513	2.14E-04	20
2	0.33	1.9180	1.82E-04	20
3	0.50	1.8804	1.79E-04	20
4	0.67	1.8429	1.78E-04	20
5	0.83	1.8068	1.77E-04	20
6	1.00	1.7706	1.77E-04	20
7	1.00	1.7692	1.78E-04	20
8	1.17	1.7331	1.78E-04	20
9	1.33	1.6983	1.78E-04	20

Hydraulic Conductivity, cm/sec 1.78E-04



GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 1/23/8 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 HYDROMETER 3 4 6 100 95 90 85 80 75 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY medium fine fine coarse coarse

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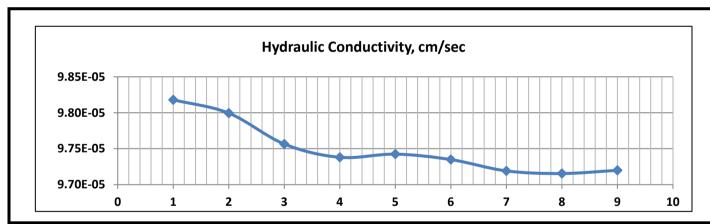
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:26 - S.\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY/PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GEV. 30 I N D E X 20 10 CL-ML (ML) (MH) 20 40 100 60 80 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-19S NP NP NP SILTY SAND (SM-SC) 17 42

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-20s (17'-19')
Sample Location :	PZ-20s (17'-19') UD-01	Date Sampled:	09/03/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial W	orking	Final Sample Conditions	
Wet Density, pcf	126.8	Pressure	s, psi	Wet Density, pcf	120.2
Dry Density, pcf	98.4	Chamber	93	Dry Density, pcf	98.8
Moisture Content, %	28.9	Head	79	Moisture Content, %	21.7
Void ratio, e	0.690	Tail	77	Void ratio, e	0.683
Porosity, n	0.408	Conso.	15	Porosity, n	0.406
Saturation, Percent	111.5	Soil Specifi	c Gravity	Saturation, Percent	84.5
Hydraulic Gradient, i	9.9	Gs	2.665	Hydraulic Gradient, i	8.2
Sample Length, Inches	5.585	Proctor Re	ferenced	Sample Length, Inches	5.535
Sample Volume, cc	569.1641	-	-	Sample Volume, cc	566.5934
B-value :	100	0.0%	Sample Co	nsolidated During Saturation, %	0.90%



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.33	1.9563	9.82E-05	20
2	0.67	1.9138	9.80E-05	20
3	1.00	1.8727	9.76E-05	20
4	1.33	1.8323	9.74E-05	20
5	1.67	1.7926	9.74E-05	20
6	2.00	1.7539	9.73E-05	20
7	2.33	1.7164	9.72E-05	20
8	2.67	1.6794	9.72E-05	20
9	3.00	1.6430	9.72E-05	20

Hydraulic Conductivity, cm/sec 9.72E-05



GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775 Fax: 205-733-8954

CLIENT Southern Company Services **PROJECT NAME** Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 1/23/8 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 HYDROMETER 100 95 90 85 80 75 GRAIN SIZE - GINT STD US LAB.GDT - 10/27/15 11:27 - S:\TRADITIONALBIRMINGHAM LAB\SOUTHERN COMPANYPLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GPJ 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY medium coarse fine coarse fine

Ž	BOREHOLE	DEPTH			Classification	on		LL	PL	PI	Сс	Cu
0.	● PZ-20s	17		S	ILTY SAND	(SM-SC)		NP	NP	NP		
7												
2												
- 10/2												
25.												
	BOREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand	i	%Silt	%(Clay
	PZ-20s	17	4.75	0.33	0.08		0.0	71.1		2	28.9	
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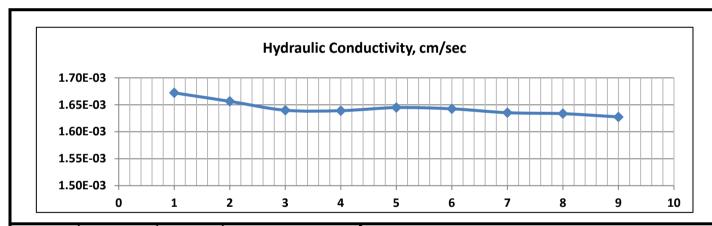
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 - S:\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANYPLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GP、 30 I N D E X 20 10 CL-ML (ML)(MH) 20 40 60 80 100 LIQUID LIMIT %M Fines **BOREHOLE DEPTH** LL PL Ы Classification ● PZ-20s NP NP NP SILTY SAND (SM-SC) 17 29 ATTERBERG LIMITS - GINT STD US LAB. GDT - 10/27/15 11:27

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-22S (7'-9')
Sample Location :	PZ-22S (7'-9') UD-01	Date Sampled:	09/17/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial W	orking	Final Sample Conditions					
Wet Density, pcf	110.9	Pressure	s, psi	Wet Density, pcf	110.3				
Dry Density, pcf	83.5	Chamber 83		Dry Density, pcf	83.6				
Moisture Content, %	32.8	Head 78 N		Moisture Content, %	31.9				
Void ratio, e	1.040	Tail	76	Void ratio, e	1.038				
Porosity, n	0.510	Conso.	6	Porosity, n	0.509				
Saturation, Percent	86.2	Soil Specifi	c Gravity	Saturation, Percent	84.0				
Hydraulic Gradient, i	9.9	Gs	2.731	Hydraulic Gradient, i	8.4				
Sample Length, Inches	5.618	Proctor Re	ferenced	Sample Length, Inches	5.600				
Sample Volume, cc	571.7477	-	-	Sample Volume, cc	570.8288				
B-value :	96	.0% Sample Cor		nsolidated During Saturation, %	0.32%				



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.02	1.9625	1.67E-03	20
2	0.03	1.9263	1.66E-03	20
3	0.05	1.8916	1.64E-03	20
4	0.07	1.8568	1.64E-03	20
5	0.08	1.8220	1.64E-03	20
6	0.10	1.7887	1.64E-03	20
7	0.12	1.7567	1.64E-03	20
8	0.13	1.7247	1.63E-03	20
9	0.15	1.6941	1.63E-03	20

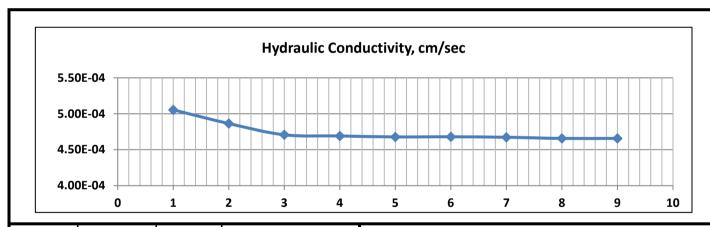
Hydraulic Conductivity, cm/sec 1.63E-03



ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-22s (17'-19')
Sample Location :	PZ-22s (17'-19') UD-02	Date Sampled:	09/17/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial W	orking	Final Sample Conditions					
Wet Density, pcf	111.1	Pressure	s, psi	Wet Density, pcf	110.3				
Dry Density, pcf	82.5	Chamber 88 I		Dry Density, pcf	82.4				
Moisture Content, %	34.7	Head 78 M		Moisture Content, %	33.8				
Void ratio, e	1.056	Tail	76	Void ratio, e	1.057				
Porosity, n	0.514	Conso.	11	Porosity, n	0.514				
Saturation, Percent	89.4	Soil Specifi	c Gravity	Saturation, Percent	86.9				
Hydraulic Gradient, i	9.8	Gs	2.717	Hydraulic Gradient, i	7.8				
Sample Length, Inches	5.645	Proctor Re	ferenced	Sample Length, Inches	5.648				
Sample Volume, cc	572.9644	-	-	Sample Volume, cc	573.1166				
B-value :	99	.0%	Samp	ole Swelled During Saturation, %	0.05%				



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.08	1.9444	5.05E-04	20
2	0.17	1.8943	4.86E-04	20
3	0.25	1.8485	4.71E-04	20
4	0.33	1.8012	4.69E-04	20
5	0.42	1.7553	4.68E-04	20
6	0.50	1.7100	4.68E-04	20
7	0.58	1.6663	4.67E-04	20
8	0.67	1.6246	4.66E-04	20
9	0.75	1.5829	4.66E-04	20

Hydraulic Conductivity, cm/sec 4.66E-04



GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 1/23/8 3 4 6 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 HYDROMETER 100 95 90 85 80 75 70 65 PERCENT FINER BY WEIGHT × 60 55 50 45 40 35 30 X 25 20 15 10 5 100 10 0.1 0.01 0.001

COPPLES	GRA	VEL		SAND)	SILT OR CLAY
COBBLES	coarse	fine	coarse	medium	fine	SILT OR CLAT

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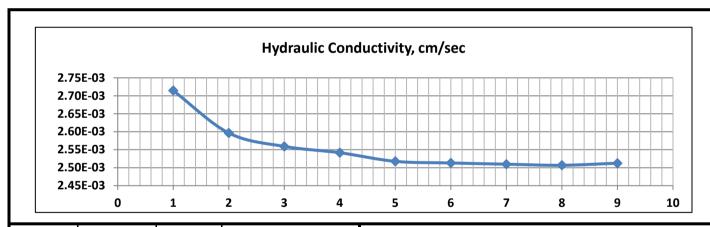
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:28 - S.\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY/PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GEV. 30 I N D E X 20 10 CL-ML (ML)(MH) 20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-22S NP NP NP SILTY SAND (SM-SC) 7 20 **▼** PZ-22S 17 NP NP NP 28 SILTY SAND (SM-SC)

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-24S (17'-19')
Sample Location :	PZ-24S (17'-19') UD-01	Date Sampled:	09/17/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	103.0	Pressure	s, psi	Wet Density, pcf	111.1
Dry Density, pcf	83.4	Chamber	90	Dry Density, pcf	83.4
Moisture Content, %	23.5	Head	79	Moisture Content, %	33.3
Void ratio, e	1.015	Tail	77	Void ratio, e	1.015
Porosity, n	0.504	Conso.	12	Porosity, n	0.504
Saturation, Percent	62.2	Soil Specifi	c Gravity	Saturation, Percent	88.2
Hydraulic Gradient, i	9.9	Gs	2.693	Hydraulic Gradient, i	7.7
Sample Length, Inches	5.587	Proctor Referenced		Sample Length, Inches	5.583
Sample Volume, cc	566.4718			Sample Volume, cc	566.2689
B-value :	96	.0% Sample Co		nsolidated During Saturation, %	0.07%



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.02	1.9388	2.71E-03	20
2	0.03	1.8846	2.60E-03	20
3	0.05	1.8318	2.56E-03	20
4	0.07	1.7803	2.54E-03	20
5	0.08	1.7317	2.52E-03	20
6	0.10	1.6830	2.51E-03	20
7	0.12	1.6357	2.51E-03	20
8	0.13	1.5899	2.51E-03	20
9	0.15	1.5440	2.51E-03	20

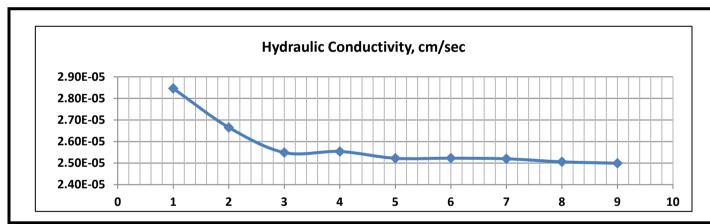
Hydraulic Conductivity, cm/sec 2.51E-03



ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-24S (37'-39')
Sample Location :	PZ-24S (37'-39') UD-02	Date Sampled:	09/16/15
Northing:	Easting:	Elevation:	-
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial W	orking	Final Sample Conditions	
Wet Density, pcf	109.6	Pressure	s, psi	Wet Density, pcf	112.3
Dry Density, pcf	90.6	Chamber	101	Dry Density, pcf	91.0
Moisture Content, %	21.0	Head	79	Moisture Content, %	23.3
Void ratio, e	0.860	Tail	77	Void ratio, e	0.851
Porosity, n	0.462	Conso.	23	Porosity, n	0.460
Saturation, Percent	66.0	Soil Specifi	c Gravity	Saturation, Percent	74.0
Hydraulic Gradient, i	9.6	Gs	2.701	Hydraulic Gradient, i	8.3
Sample Length, Inches	5.745	Proctor Re	ferenced	Sample Length, Inches	5.687
Sample Volume, cc	594.031			Sample Volume, cc	591.0023
B-value :	100	0.0% Sample Co		onsolidated During Saturation, %	1.01%



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	1.00	1.9611	2.85E-05	20
2	2.00	1.9277	2.67E-05	20
3	3.00	1.8971	2.55E-05	20
4	4.00	1.8638	2.55E-05	20
5	5.00	1.8332	2.52E-05	20
6	6.00	1.8015	2.52E-05	20
7	7.00	1.7706	2.52E-05	20
8	8.00	1.7414	2.51E-05	20
9	9.00	1.7122	2.50E-05	20

Hydraulic Conductivity, cm/sec 2.50E-05

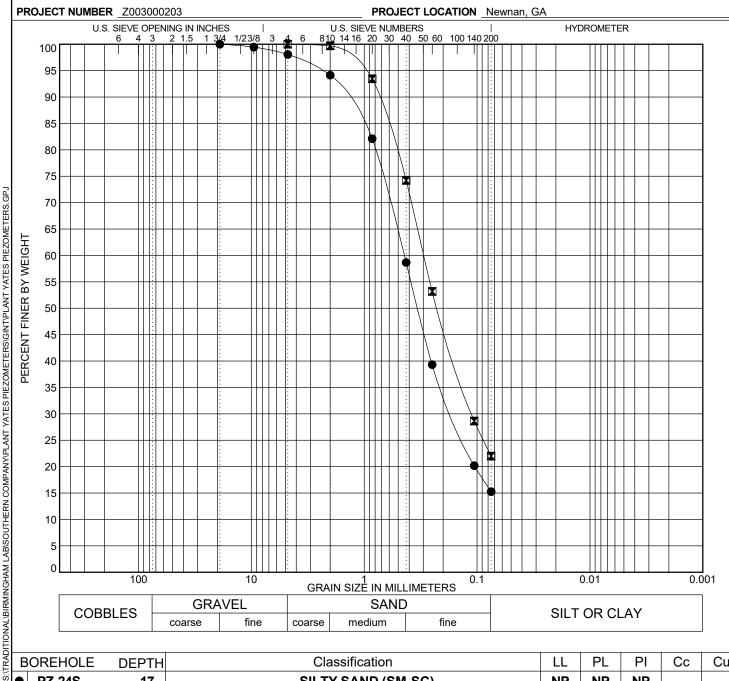


GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775 Fax: 205-733-8954

 CLIENT
 Southern Company Services
 PROJECT NAME
 Plant Yates Piezometers



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- S:\TRADITI(BOREHOLE	DEPTH		Classification					PL	PI	C	Cu
. S:∖.	• PZ-24S	17		S	ILTY SAND	(SM-SC)		NP	NP	NP		
11:28	▼ PZ-24S	37		S	ILTY SAND	(SM-SC)		NP	NP	NP		
10/27/15												
- 10/2												
GDT												
AIN SIZE - GINT STD US LAB.GDT	BOREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand	1	%Silt	%(Clay
E U	• PZ-24S	17	19	0.442	0.165		1.9	82.8		1	5.3	,
INTS	▼ PZ-24S	37	4.75	0.297	0.111		0.0	78.0		2	22.0	
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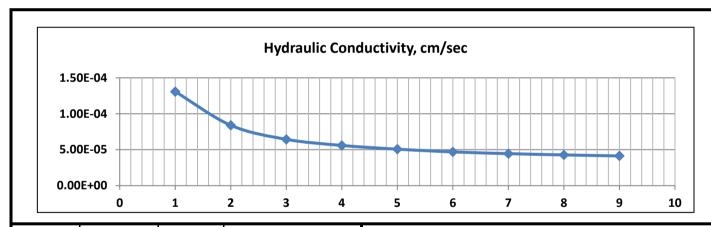
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:28 - S.\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY/PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GEV. 30 I N D E X 20 10 CL-ML (ML)(MH) 20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-24S NP NP NP SILTY SAND (SM-SC) 17 15 **▼** PZ-24S 37 NP NP NP 22 SILTY SAND (SM-SC)

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-25S (33'-35')
Sample Location :	PZ-25S (33'-35') UD-01	Date Sampled:	09/03/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	110.9	Pressure	s, psi	Wet Density, pcf	112.1
Dry Density, pcf	85.8	Chamber	98	Dry Density, pcf	86.1
Moisture Content, %	29.2	Head	79	Moisture Content, %	30.3
Void ratio, e	0.947	Tail	77	Void ratio, e	0.942
Porosity, n	0.486	Conso.	20	Porosity, n	0.485
Saturation, Percent	82.7	Soil Specifi	c Gravity	Saturation, Percent	86.1
Hydraulic Gradient, i	9.8	Gs	2.678	Hydraulic Gradient, i	7.7
Sample Length, Inches	5.635	Proctor Re	ferenced	Sample Length, Inches	5.600
Sample Volume, cc	575.4849			Sample Volume, cc	573.6865
B-value :	97	.0% Sample Co		nsolidated During Saturation, %	0.62%



Start	Cum.	Head	Hydra	ulic
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	1.00	1.8290	1.31E-04	20
2	2.00	1.7831	8.40E-05	20
3	3.00	1.7525	6.44E-05	20
4	4.00	1.7164	5.59E-05	20
5	5.00	1.6816	5.07E-05	20
6	6.00	1.6497	4.70E-05	20
7	7.00	1.6163	4.45E-05	20
8	8.00	1.5829	4.28E-05	20
9	9.00	1.5509	4.13E-05	20

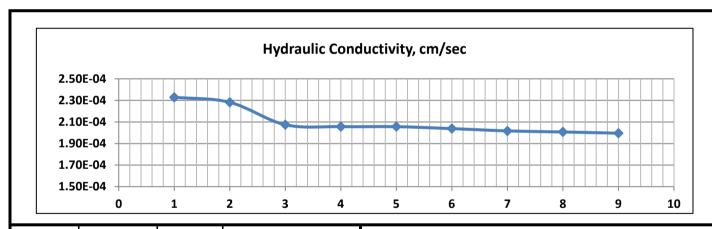
Hydraulic Conductivity, cm/sec 4.13E-05



ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-25S (44'-46')
Sample Location :	PZ-25S (44'-46') UD-02	Date Sampled:	09/03/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	118.4	Pressure	s, psi	Wet Density, pcf	119.4
Dry Density, pcf	97.9	Chamber	104	Dry Density, pcf	98.1
Moisture Content, %	21.0	Head	78	Moisture Content, %	21.7
Void ratio, e	0.710	Tail	76	Void ratio, e	0.706
Porosity, n	0.415	Conso.	27	Porosity, n	0.414
Saturation, Percent	79.2	Soil Specifi	c Gravity	Saturation, Percent	82.4
Hydraulic Gradient, i	9.9	Gs	2.682	Hydraulic Gradient, i	9.0
Sample Length, Inches	5.610	Proctor Re	ferenced	Sample Length, Inches	5.577
Sample Volume, cc	572.1864			Sample Volume, cc	570.4937
B-value :	97	.0% Sample Co		onsolidated During Saturation, %	0.59%



Start	Cum.	Head	Hydraulic		
Test @	Time	Loss, ∆h2	Conductivity, k		
t=0	Δt, min.	psi	(Permea	bility)	
0	0.00	2.0000	cm/sec	°C	
1	0.08	1.9736	2.33E-04	20	
2	0.17	1.9486	2.28E-04	20	
3	0.25	1.9302	2.07E-04	20	
4	0.33	1.9082	2.06E-04	20	
5	0.42	1.8860	2.06E-04	20	
6	0.50	1.8651	2.04E-04	20	
7	0.58	1.8451	2.02E-04	20	
8	0.67	1.8248	2.01E-04	20	
9	0.75	1.8051	2.00E-04	20	

Hydraulic Conductivity, cm/sec 2.00E-04



GRAIN SIZE DISTRIBUTION



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CLIENT Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES

6 4 3 2 1.5 1 3/4 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 HYDROMETER 3 1/23/8 100 95 90 85 I 80 75 GRAIN SIZE - GINT STD US LAB.GDT - 10/27/15 11:29 - S.\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY\PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS. GPJ 70 65 PERCENT FINER BY WEIGHT 60 55 × 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY medium fine fine coarse coarse

- 1	BOREHOLE	DEPTH		Classification					PL	PI	Сс	Cu
		33		S	ILTY SAND	(SM-SC)		NP	NP	NP		
Z-	PZ-25S	44		S	ILTY SAND	(SM-SC)		NP	NP	NP		
	BOREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand	1	%Silt	%(Clay
	PZ-25S	33	9.5	0.34	0.124		0.6	76.1		2	23.4	
		44	19	0.306	0.109		3.8	73.9		2	22.3	
			·									

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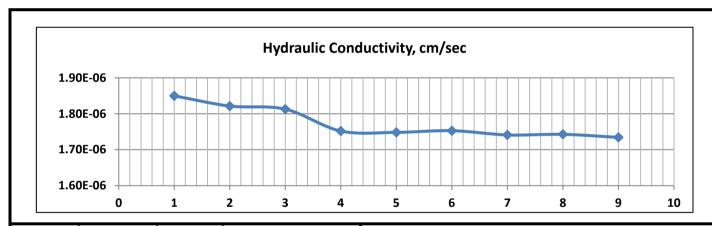
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:29 - S.'TRADITIONAL'BIRMINGHAM LABISOUTHERN COMPANYIPLANT YATES PIEZOMETERS'GINTIPLANT YATES PIEZOMETERS GR. 30 I N D E X 20 10 CL-ML (ML)(MH) 20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-25S NP NP NP SILTY SAND (SM-SC) 33 23 **▼** PZ-25S 44 NP NP NP 22 SILTY SAND (SM-SC)

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-26S (17'-19')
Sample Location :	PZ-26S (17'-19') UD-01	Date Sampled:	9/31/2015
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions	Initial Working		Final Sample Conditions		
Wet Density, pcf	122.6	Pressure	s, psi	Wet Density, pcf	115.8
Dry Density, pcf	94.9	Chamber	92	Dry Density, pcf	95.2
Moisture Content, %	29.3	Head	79	Moisture Content, %	21.7
Void ratio, e	0.803	Tail	77	Void ratio, e	0.797
Porosity, n	0.445	Conso.	14	Porosity, n	0.443
Saturation, Percent	99.9	Soil Specifi	c Gravity	Saturation, Percent	74.5
Hydraulic Gradient, i	9.8	Gs	2.741	Hydraulic Gradient, i	8.9
Sample Length, Inches	5.645	Proctor Re	ferenced	Sample Length, Inches	5.601
Sample Volume, cc	590.4383			Sample Volume, cc	588.1194
B-value :	100	0.0% Sample Co		nsolidated During Saturation, %	0.78%



Start	Cum.	Head	Hydraulic	
Test @	Time	Loss, ∆h2	Conductivity, k	
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	10.00	1.9741	1.85E-06	20
2	20.00	1.9494	1.82E-06	20
3	30.00	1.9249	1.81E-06	20
4	40.00	1.9038	1.75E-06	20
5	50.00	1.8807	1.75E-06	20
6	60.00	1.8574	1.75E-06	20
7	70.00	1.8357	1.74E-06	20
8	80.00	1.8131	1.74E-06	20
9	90.00	1.7920	1.73E-06	20

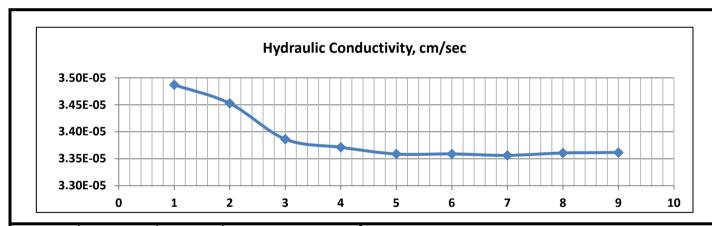
Hydraulic Conductivity, cm/sec 1.73E-06



ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-26S (27'-29')
Sample Location :	PZ-26S (27'-29') UD-02	Date Sampled:	9/31/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions	Initial Working		Final Sample Conditions		
Wet Density, pcf	120.1	Pressure	s, psi	Wet Density, pcf	119.2
Dry Density, pcf	92.5	Chamber	101	Dry Density, pcf	92.5
Moisture Content, %	29.8	Head	79	Moisture Content, %	28.8
Void ratio, e	0.834	Tail	77	Void ratio, e	0.834
Porosity, n	0.455	Conso.	23	Porosity, n	0.455
Saturation, Percent	97.1	Soil Specifi	c Gravity	Saturation, Percent	94.1
Hydraulic Gradient, i	9.8	Gs	2.720	Hydraulic Gradient, i	8.0
Sample Length, Inches	5.623	Proctor Re	ferenced	Sample Length, Inches	5.619
Sample Volume, cc	570.3925			Sample Volume, cc	570.1895
B-value :	100	0.0% Sample Co		nsolidated During Saturation, %	0.07%



Start	Cum.	Head	Hydraulic	
Test @	Time	Loss, ∆h2	Conductivity, k	
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	1.00	1.9538	3.49E-05	20
2	2.00	1.9096	3.45E-05	20
3	3.00	1.8685	3.39E-05	20
4	4.00	1.8273	3.37E-05	20
5	5.00	1.7873	3.36E-05	20
6	6.00	1.7475	3.36E-05	20
7	7.00	1.7089	3.36E-05	20
8	8.00	1.6705	3.36E-05	20
9	9.00	1.6332	3.36E-05	20

Hydraulic Conductivity, cm/sec 3.36E-05



GRAIN SIZE DISTRIBUTION



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PZ-26s

27

4.75

0.138

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Fax: 205-733-8954 PROJECT NAME Plant Yates Piezometers **CLIENT** Southern Company Services PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 1/23/8 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 **HYDROMETER** 100 95 90 85 80 75 S:\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY,PLANT YATES PIEZOMETERS\G\INT\PLANT YATES PIEZOMETERS.GR, 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY medium fine fine coarse coarse Classification Сс **BOREHOLE** LL PL Ы Cu **DEPTH** • **PZ-26s** 17 SANDY SILT (ML) 37 27 10 - 10/27/15 11:29 **PZ-26s** 27 \blacksquare SILTY SAND (SM-SC) NP NP NP US LAB.GDT **DEPTH BOREHOLE** D100 D60 D30 D10 %Gravel %Sand %Silt %Clay 42.1 57.9 GINT STD PZ-26s 17 4.75 0.081 0.0

0.0

66.3

33.7

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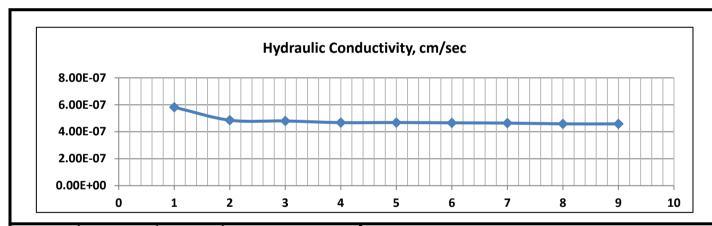
ATTERBERG LIMITS RESULTS

Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:29 - S.'TRADITIONAL'BIRMINGHAM LABISOUTHERN COMPANYIPLANT YATES PIEZOMETERS'GINTIPLANT YATES PIEZOMETERS GR. 30 I N D E X 20 10 CL-ML (ML) (MH) 20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-26s SANDY SILT (ML) 17 37 27 10 58 **▼** PZ-26s 27 NP NP NP 34 SILTY SAND (SM-SC)

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-27S (17'-19')
Sample Location :	PZ-27S (17'-19') UD-01	Date Sampled:	10/07/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions	Initial Working		Final Sample Conditions		
Wet Density, pcf	126.5	Pressure	s, psi	Wet Density, pcf	126.1
Dry Density, pcf	99.6	Chamber	93	Dry Density, pcf	99.8
Moisture Content, %	27.0	Head	79	Moisture Content, %	26.3
Void ratio, e	0.667	Tail	77	Void ratio, e	0.663
Porosity, n	0.400	Conso.	15	Porosity, n	0.399
Saturation, Percent	107.7	Soil Specifi	c Gravity	Saturation, Percent	105.7
Hydraulic Gradient, i	9.8	Gs	2.661	Hydraulic Gradient, i	9.6
Sample Length, Inches	5.635	Proctor Re	ferenced	Sample Length, Inches	5.603
Sample Volume, cc	578.7582			Sample Volume, cc	577.1055
B-value :	97	.0% Sample Co		onsolidated During Saturation, %	0.57%



Start	Cum.	Head	Hydraulic		
Test @	Time	Loss, ∆h2	Conductivity, k		
t=0	Δt, min.	psi	(Permea	bility)	
0	0.00	2.0000	cm/sec	°C	
1	10.00	1.9917	5.82E-07	20	
2	20.00	1.9861	4.86E-07	20	
3	30.00	1.9794	4.80E-07	20	
4	40.00	1.9733	4.68E-07	20	
5	50.00	1.9666	4.68E-07	20	
6	60.00	1.9602	4.66E-07	20	
7	70.00	1.9538	4.64E-07	20	
8	80.00	1.9480	4.58E-07	20	
9	90.00	1.9416	4.58E-07	20	

Hydraulic Conductivity, cm/sec 4.58E-07

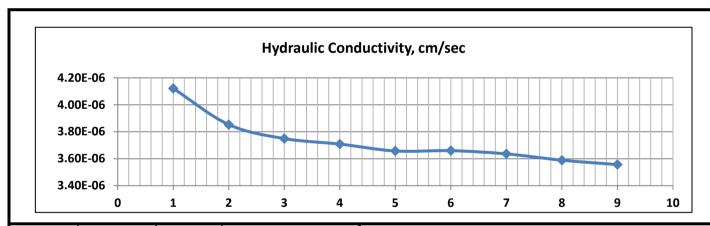


Measurement of Hydraulic Conductivity of Saturated Porous Materials Using <u>a Flexible Wall Permeameter</u>

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-27S (27'-29')
Sample Location :	PZ-27S (27'-29') UD-02	Date Sampled:	10/07/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	120.0	Pressure	s, psi	Wet Density, pcf	119.6
Dry Density, pcf	91.1	Chamber	98	Dry Density, pcf	91.3
Moisture Content, %	31.7	Head	79	Moisture Content, %	31.0
Void ratio, e	0.831	Tail	77	Void ratio, e	0.827
Porosity, n	0.454	Conso.	20	Porosity, n	0.453
Saturation, Percent	101.9	Soil Specifi	c Gravity	Saturation, Percent	100.2
Hydraulic Gradient, i	9.8	Gs	2.673	Hydraulic Gradient, i	8.8
Sample Length, Inches	5.657	Proctor Re	ferenced	Sample Length, Inches	5.629
Sample Volume, cc	576.1263			Sample Volume, cc	574.6934
B-value :	99	0.0% Sample Co		nsolidated During Saturation, %	0.49%



Start	Cum.	Head	Hydraulic	
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	5.00	1.9725	4.12E-06	20
2	10.00	1.9488	3.85E-06	20
3	15.00	1.9258	3.75E-06	20
4	20.00	1.9027	3.71E-06	20
5	25.00	1.8807	3.66E-06	20
6	30.00	1.8576	3.66E-06	20
7	35.00	1.8359	3.64E-06	20
8	40.00	1.8159	3.59E-06	20
9	45.00	1.7959	3.56E-06	20

Hydraulic Conductivity, cm/sec 3.56E-06



200 Wellington Manor Court Suite 100 Alabaster, Alabama 35007

GRAIN SIZE DISTRIBUTION



US LAB.GDT

GINT STD

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BOREHOLE

PZ-27s

PZ-27s

DEPTH

17

27

D100

4.75

2

D60

0.111

D30

D10

%Gravel

0.0

0.0

%Sand

26.5

55.0

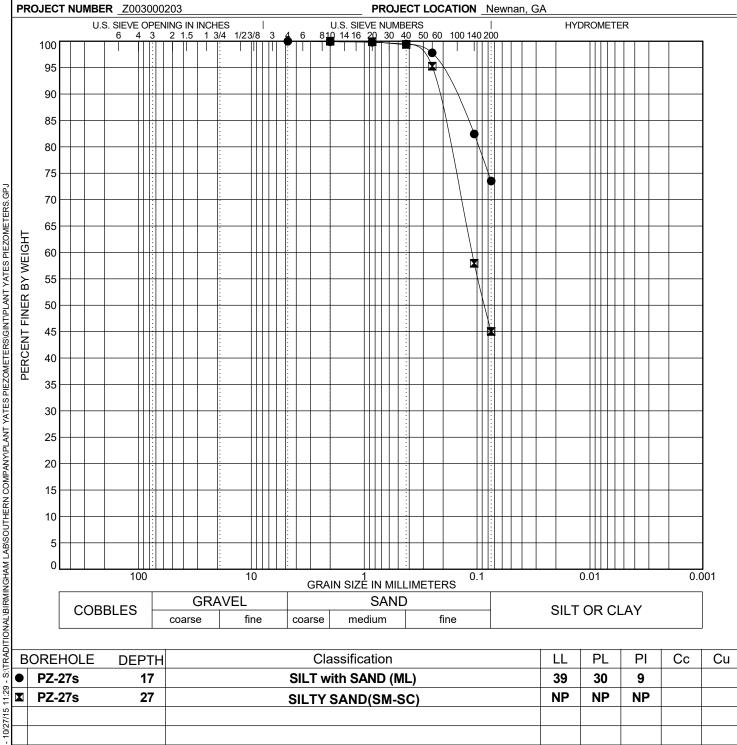
%Silt

73.5

45.0

200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775 Fax: 205-733-8954

 CLIENT
 Southern Company Services
 PROJECT NAME
 Plant Yates Piezometers



%Clay

Cardno ATC

200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

ATTERBERG LIMITS RESULTS

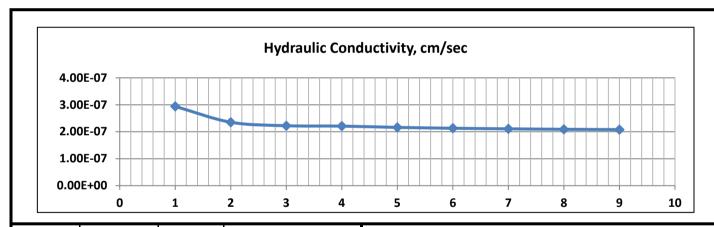
Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:29 - S.'TRADITIONAL'BIRMINGHAM LABISOUTHERN COMPANYIPLANT YATES PIEZOMETERS'GINTIPLANT YATES PIEZOMETERS GR. 30 I N D E X 20 10 CL-ML (ML)(MH)20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-27s 39 30 SILT with SAND (ML) 17 9 74 **▼** PZ-27s 27 NP NP NP 45 SILTY SAND(SM-SC)

Measurement of Hydraulic Conductivity of Saturated Porous Materials Using <u>a Flexible Wall Permeameter</u>

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-28S (17'-19')
Sample Location :	PZ-28S (17'-19') UD-01	Date Sampled:	No Date
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	124.6	Pressure	s, psi	Wet Density, pcf	126.0
Dry Density, pcf	100.8	Chamber	93	Dry Density, pcf	100.9
Moisture Content, %	23.6	Head	79	Moisture Content, %	24.9
Void ratio, e	0.597	Tail	77	Void ratio, e	0.594
Porosity, n	0.374	Conso.	15	Porosity, n	0.373
Saturation, Percent	102.2	Soil Specifi	c Gravity	Saturation, Percent	108.0
Hydraulic Gradient, i	9.9	Gs	2.578	Hydraulic Gradient, i	9.8
Sample Length, Inches	5.606	Proctor Re	ferenced	Sample Length, Inches	5.584
Sample Volume, cc	568.1281			Sample Volume, cc	567.0091
B-value :	99	9.0% Sample Co		nsolidated During Saturation, %	0.39%



Start	Cum.	Head	Hydraulic	
Test @	Time	Loss, ∆h2	Conducti	vity, k
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	10.00	1.9958	2.94E-07	20
2	20.00	1.9933	2.35E-07	20
3	30.00	1.9905	2.22E-07	20
4	40.00	1.9875	2.21E-07	20
5	50.00	1.9847	2.16E-07	20
6	60.00	1.9819	2.13E-07	20
7	70.00	1.9791	2.11E-07	20
8	80.00	1.9764	2.09E-07	20
9	90.00	1.9736	2.08E-07	20

Hydraulic Conductivity, cm/sec 2.08E-07



200 Wellington Manor Court Suite 100 Alabaster, Alabama 35007

GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

Fax: 205-733-8954 **PROJECT NAME** Plant Yates Piezometers **CLIENT** Southern Company Services PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 **HYDROMETER** 1/23/8 3 100 95 90 85 80 75 S:\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY\PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GP、 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY medium fine fine coarse coarse Classification Сс **BOREHOLE** LL PL Ы Cu **DEPTH PZ-28s** 17 SILTY SAND (SM-SC) NP NP NP US LAB.GDT - 10/27/15 11:30 -**DEPTH BOREHOLE** D100 D60 D30 D10 %Gravel %Sand %Silt %Clay PZ-28s 18.9 GINT STD 17 19 0.352 0.128 5.7 75.4

200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

ATTERBERG LIMITS RESULTS

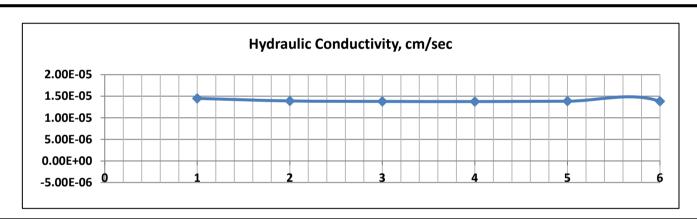
Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:30 - S./TRADITIONAL/BIRMINGHAM LAB/SOUTHERN COMPANY/PLANT YATES PIEZOMETERS/GINT/PLANT YATES PIEZOMETERS.GBV. 30 I N D E X 20 10 CL-ML (ML) (MH)20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-28s NP NP NP SILTY SAND (SM-SC) 17 19

Measurement of Hydraulic Conductivity of Saturated Porous Materials Using <u>a Flexible Wall Permeameter</u>

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-30S (27'-29')
Sample Location :	PZ-30S (27'-29') UD-01	Date Sampled:	09/23/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	122.4	Pressure	s, psi	Wet Density, pcf	124.0
Dry Density, pcf	99.7	Chamber	101	Dry Density, pcf	100.5
Moisture Content, %	22.7	Head	79	Moisture Content, %	23.5
Void ratio, e	0.696	Tail	77	Void ratio, e	0.683
Porosity, n	0.410	Conso.	23	Porosity, n	0.406
Saturation, Percent	88.5	Soil Specifi	c Gravity	Saturation, Percent	93.1
Hydraulic Gradient, i	10.0	Gs	2.710	Hydraulic Gradient, i	7.5
Sample Length, Inches	5.538	Proctor Re	ferenced	Sample Length, Inches	5.450
Sample Volume, cc	580.1959			Sample Volume, cc	575.5133
B-value :	100	0.0% Sample Co		nsolidated During Saturation, %	1.59%



Start	Cum.	Head	Hydraulic		
Test @	Time	Loss, ∆h2	Conducti	ivity, k	
t=0	Δt, min.	psi	(Permea	bility)	
0	0.00	2.0000	cm/sec	°C	
1	5.00	1.8985	1.45E-05	20	
2	10.00	1.8098	1.39E-05	20	
3	15.00	1.7236	1.38E-05	20	
4	20.00	1.6410	1.38E-05	20	
5	25.00	1.5596	1.38E-05	20	
6	30.00	1.4842	1.38E-05	20	
7	0.00			20	
8	0.00			20	
9	0.00			20	

Hydraulic Conductivity, cm/sec 1.38E-05



200 Wellington Manor Court Suite 100 Alabaster, Alabama 35007

GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775 Fay: 205-733-8954

Fax: 205-733-8954 **CLIENT** Southern Company Services **PROJECT NAME** Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE OPENING IN INCHES 6 4 3 2 1.5 1 3/4 1/23/8 U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 **HYDROMETER** 100 95 90 85 80 75 S:\TRADITIONAL\BIRMINGHAM LAB\SOUTHERN COMPANY\PLANT YATES PIEZOMETERS\GINT\PLANT YATES PIEZOMETERS.GP. 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY medium coarse fine coarse fine **BOREHOLE** Classification PL Сс Cu **DEPTH** ы PZ-30s SILTY SAND (SM-SC) NP NP NP 27

	FZ-303	21		3	ILII SAND	(3141-3C)		INF	IAL	IAL	
В	OREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand		%Silt	%Clay
•	PZ-30s	27	4.75	0.296	0.13		0.0	83.4		1	6.6
\dashv											

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ATTERBERG LIMITS RESULTS

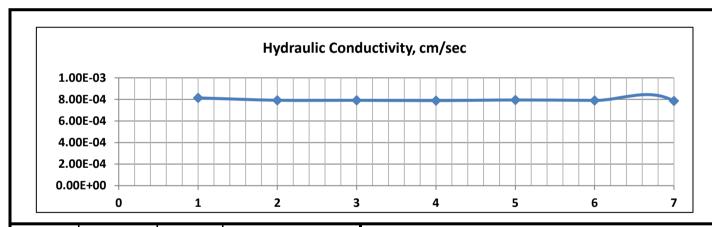
Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:30 - S./TRADITIONAL/BIRMINGHAM LAB/SOUTHERN COMPANY/PLANT YATES PIEZOMETERS/GINT/PLANT YATES PIEZOMETERS.GBV. 30 I N D E X 20 10 CL-ML (ML)(MH)20 40 60 80 100 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-30s NP NP NP SILTY SAND (SM-SC) 27 17

Measurement of Hydraulic Conductivity of Saturated Porous Materials Using <u>a Flexible Wall Permeameter</u>

ASTM D 5084-10

Project :	Plant Yates Piezometers	Project Number:	Z003000203
Client :	Southern Company Services, INC	Sample Number :	PZ-31S (44'-46')
Sample Location :	PZ-31S (44'-46')	Date Sampled:	09/03/15
Northing:	Easting:	Elevation:	
Sample Preparation:	Shelby Tube Pushed	Permeant Liquid:	De-Aired Tap Water

Initial Sample Conditions		Initial Working		Final Sample Conditions	
Wet Density, pcf	99.9	Pressure	s, psi	Wet Density, pcf	108.3
Dry Density, pcf	79.1	Chamber	83	Dry Density, pcf	79.2
Moisture Content, %	26.3	Head	79	Moisture Content, %	36.7
Void ratio, e	1.092	Tail	77	Void ratio, e	1.089
Porosity, n	0.522	Conso.	5	Porosity, n	0.521
Saturation, Percent	63.8	Soil Specifi	c Gravity	Saturation, Percent	89.3
Hydraulic Gradient, i	9.9	Gs	2.653	Hydraulic Gradient, i	7.2
Sample Length, Inches	5.617	Proctor Referenced		Sample Length, Inches	5.598
Sample Volume, cc	583.669			Sample Volume, cc	582.6785
B-value :	98	3.0% Sample Co		nsolidated During Saturation, %	0.34%



Start	Cum.	Head	Hydraulic	
Test @	Time	Loss, ∆h2	Conductivity, k	
t=0	Δt, min.	psi	(Permea	bility)
0	0.00	2.0000	cm/sec	°C
1	0.08	1.9099	8.14E-04	20
2	0.17	1.8284	7.92E-04	20
3	0.25	1.7484	7.92E-04	20
4	0.33	1.6727	7.89E-04	20
5	0.42	1.5971	7.95E-04	20
6	0.50	1.5290	7.90E-04	20
7	0.58	1.4653	7.85E-04	20
8	0.67			20
9	0.75			20

Hydraulic Conductivity, cm/sec 7.85E-04



200 Wellington Manor Court Suite 100 Alabaster, Alabama 35007

GRAIN SIZE DISTRIBUTION



200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

Fax: 205-733-8954 **CLIENT** Southern Company Services **PROJECT NAME** Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA U.S. SIEVE NUMBERS | 810 14 16 20 30 40 50 60 100 140 200 U.S. SIEVE OPENING IN INCHES | 6 4 3 2 1.5 1 3/4 1/23/8 HYDROMETER 100 95 90 85 80 75 GRAIN SIZE - GINT STD US LAB.GDT - 10/27/15 11:30 - S:TRADITIONALBIRMINGHAM LABISOUTHERN COMPANYIPLANT YATES PIEZOMETERSIGINTIPLANT YATES PIEZOMETERS. GPJ 70 65 PERCENT FINER BY WEIGHT 60 55 50 45 40 35 30 25 20 15 10 5 10 0.1 0.01 0.001 GRAIN SIZE IN MILLIMETERS **GRAVEL SAND COBBLES** SILT OR CLAY fine medium fine coarse coarse BOREHOLE 11 ΡI ΡI Cc DEDTH Classification

	OKEHULE	DEPTH			Ciassilicati	on		LL	PL	PI		Cu
Ó	PZ-31s	7		SILTY SAND (SM-SC)					NP	NP		
3												
E	BOREHOLE	DEPTH	D100	D60	D30	D10	%Gravel	%Sand	1	%Silt	%(Clay
	PZ-31s	7	4.75	0.344	0.141		0.0	83.3		1	6.7	
								·				
-												

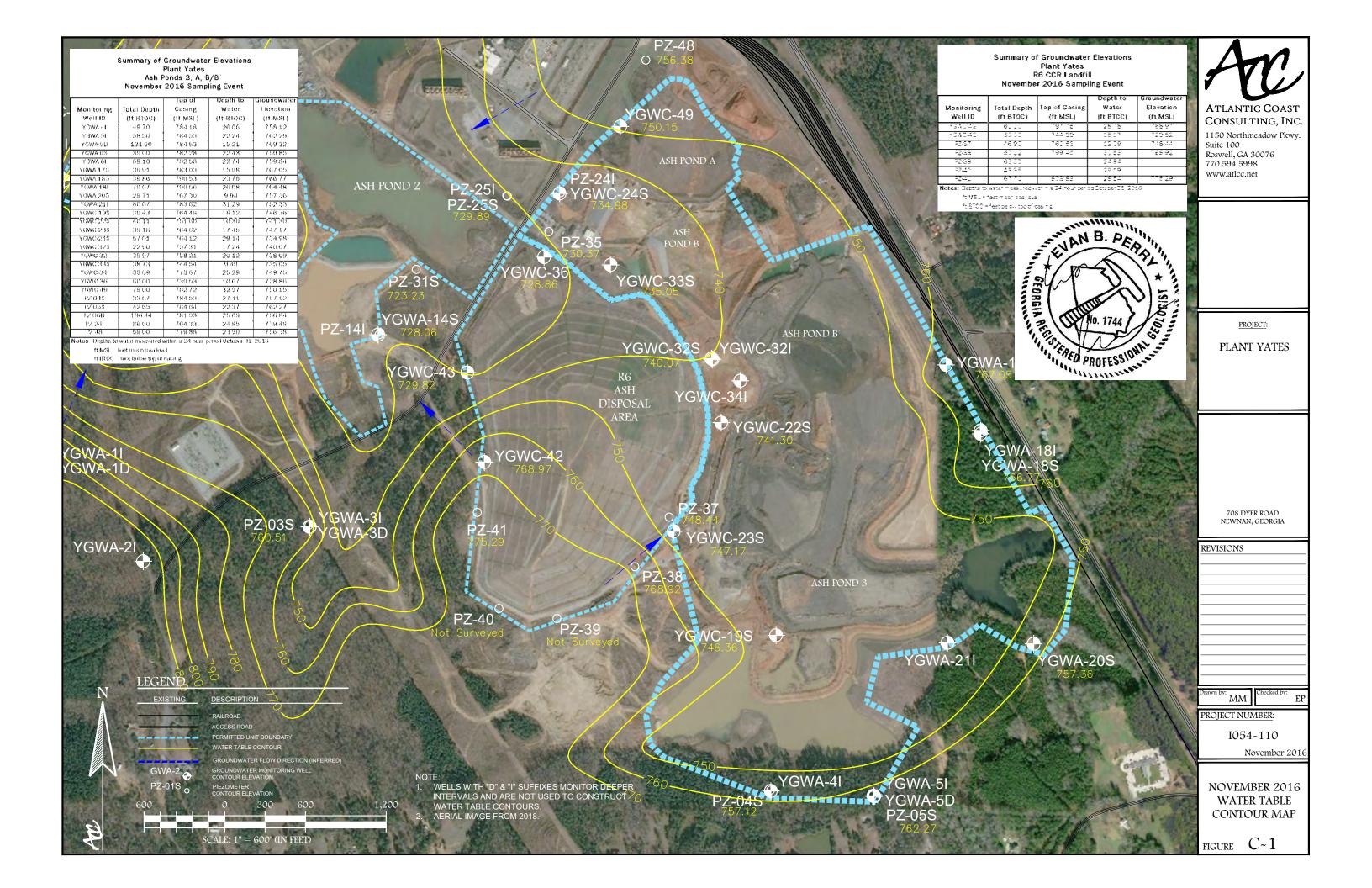
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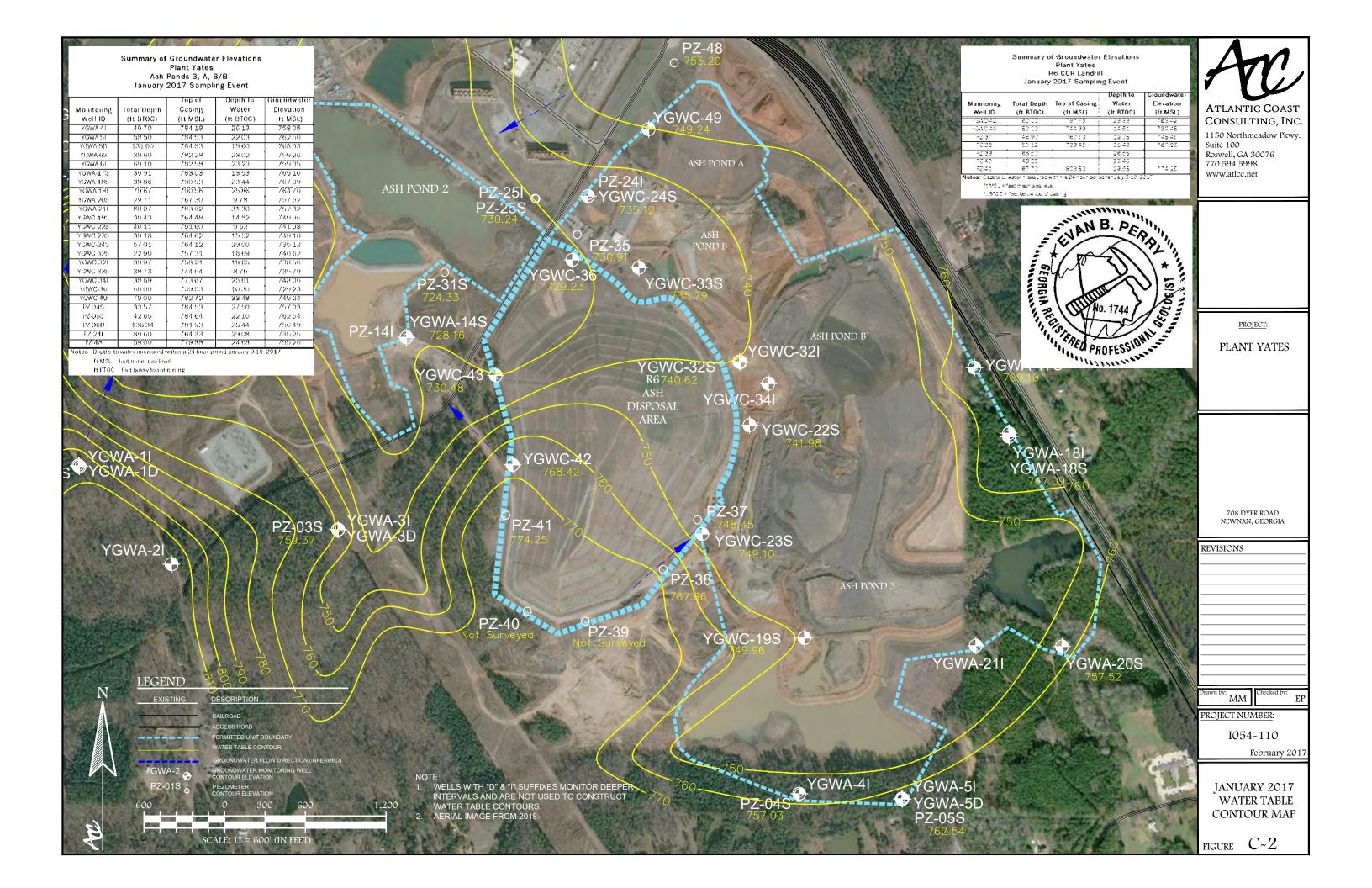
200 Wellington Court, Suite 100 Alabaster, Alabama 35007 Office: 205-738-8775

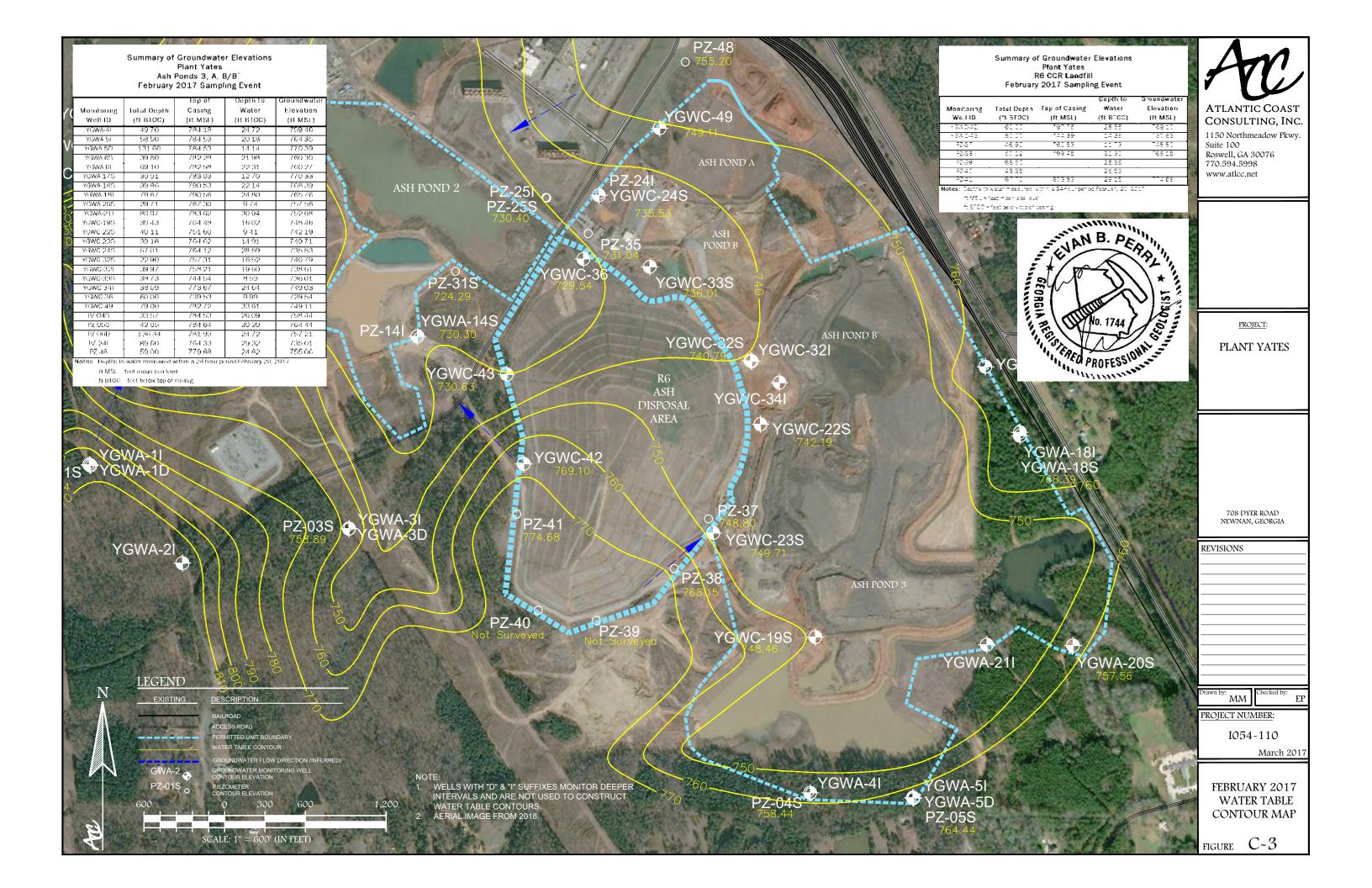
ATTERBERG LIMITS RESULTS

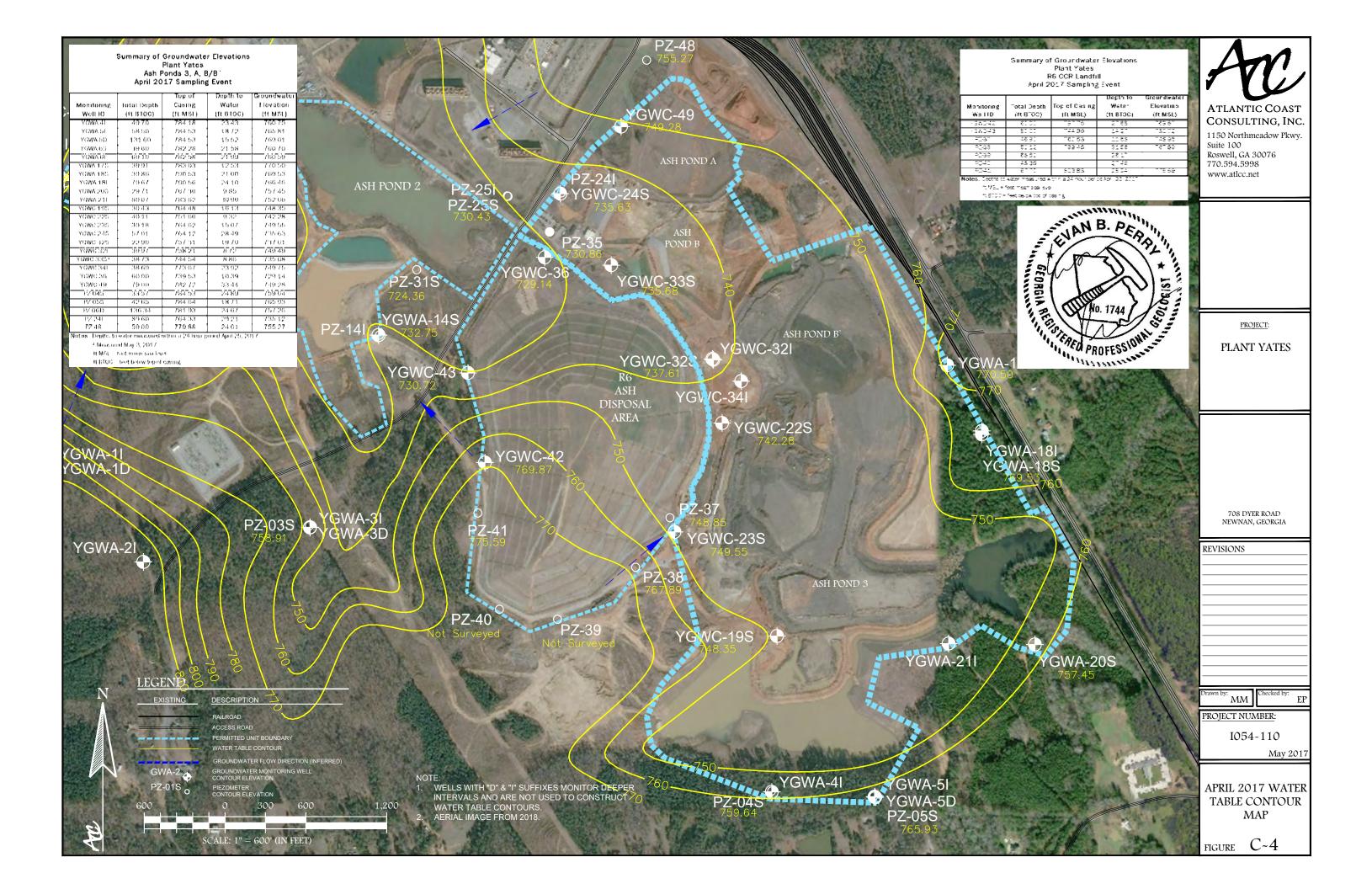
Fax: 205-733-8954 **CLIENT** Southern Company Services PROJECT NAME Plant Yates Piezometers PROJECT NUMBER Z003000203 PROJECT LOCATION Newnan, GA 60 (CL) (CH) 50 LASTICITY 40 ATTERBERG LIMITS - GINT STD US LAB.GDT - 10/27/15 11:30 - S./TRADITIONAL/BIRMINGHAM LAB/SOUTHERN COMPANY/PLANT YATES PIEZOMETERS/GINT/PLANT YATES PIEZOMETERS.GBV. 30 I N D E X 20 10 CL-ML (ML)(MH) 20 40 60 100 80 LIQUID LIMIT **BOREHOLE DEPTH** LL PL Ы %M Fines Classification ● PZ-31s NP NP NP SILTY SAND (SM-SC) 7 17

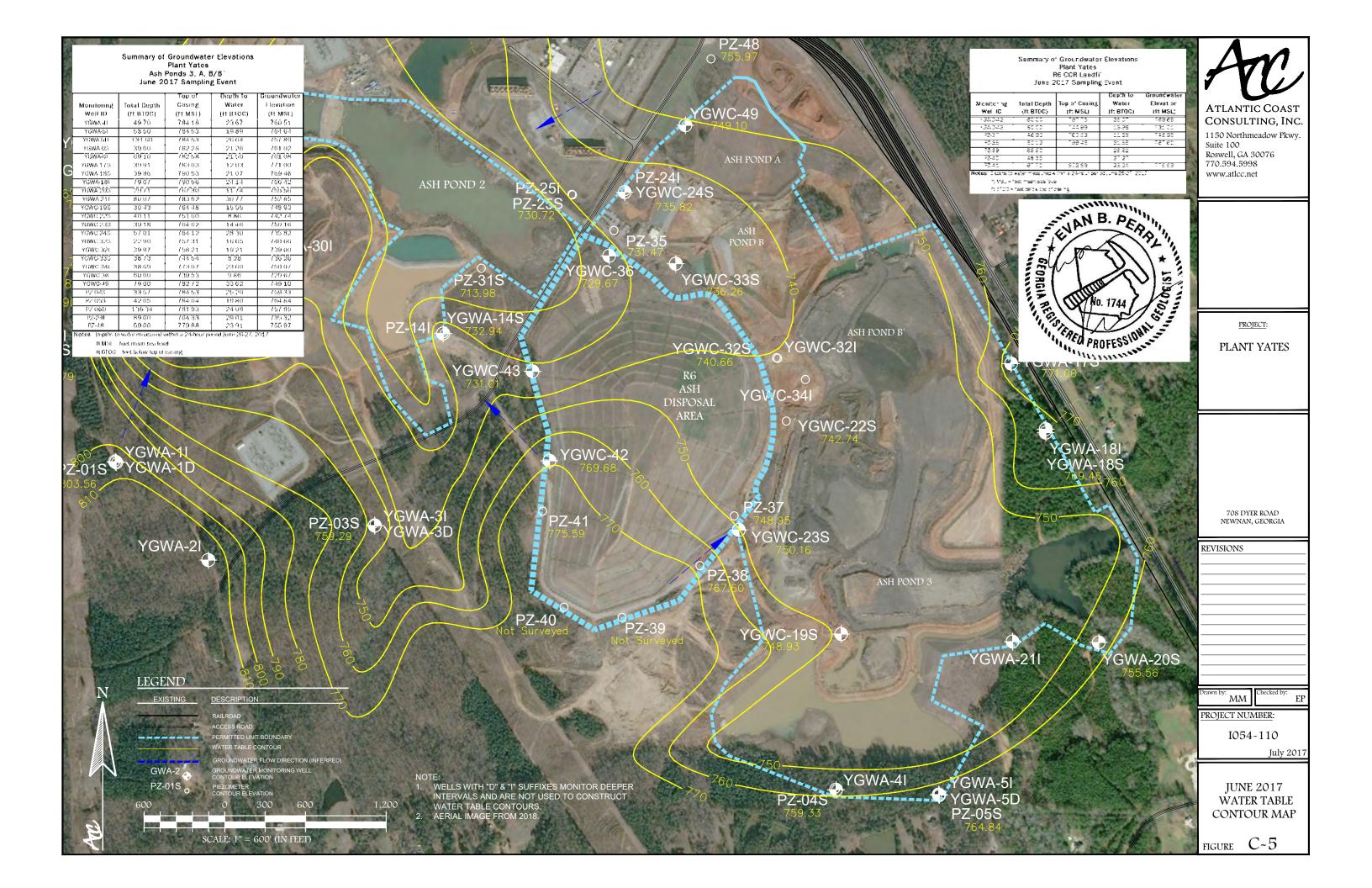
APPENDIX D HISTORICAL POTENTIOMETRIC MAPS (2016 - 2018) AND WATER LEVEL DATA

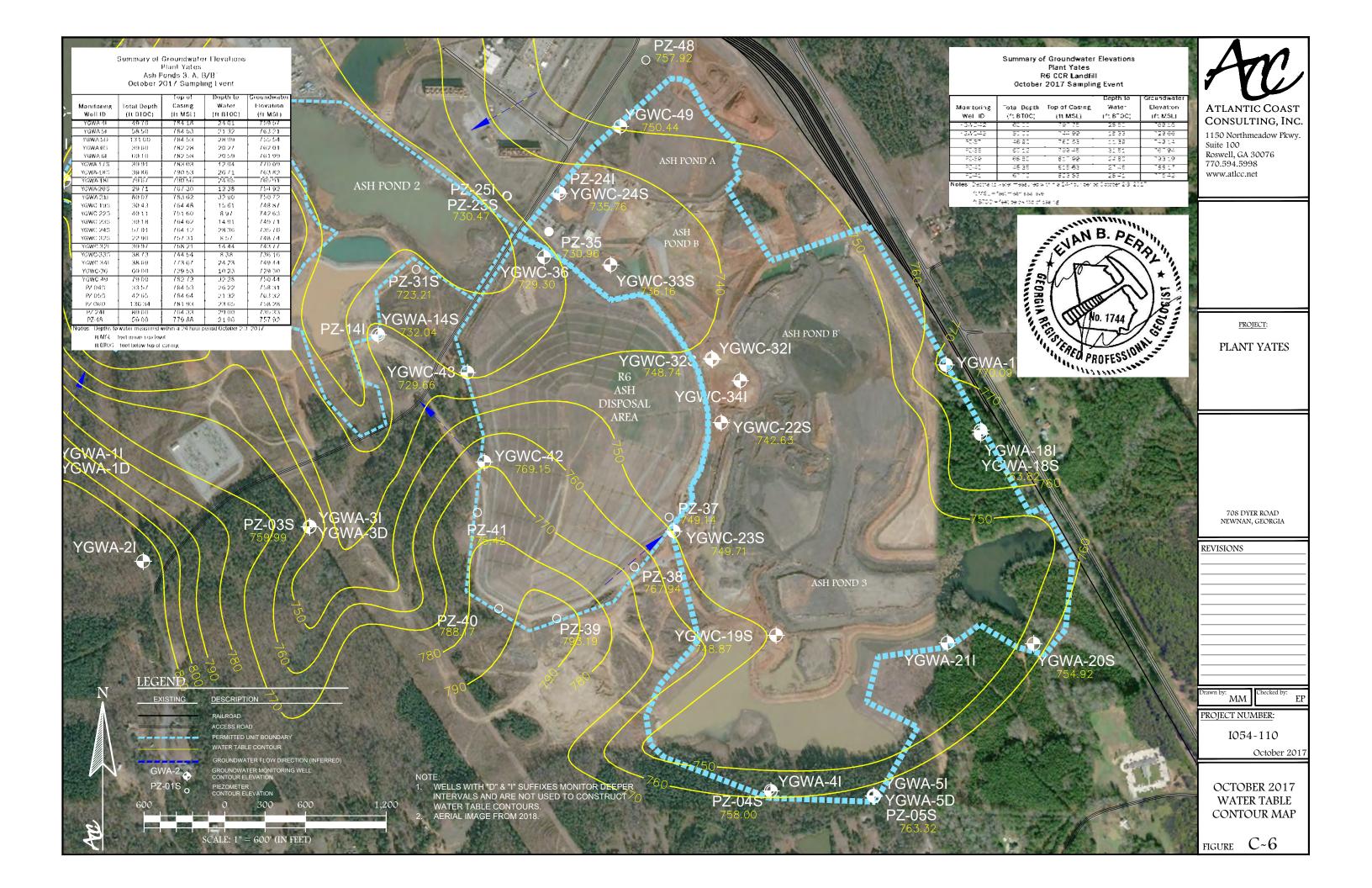


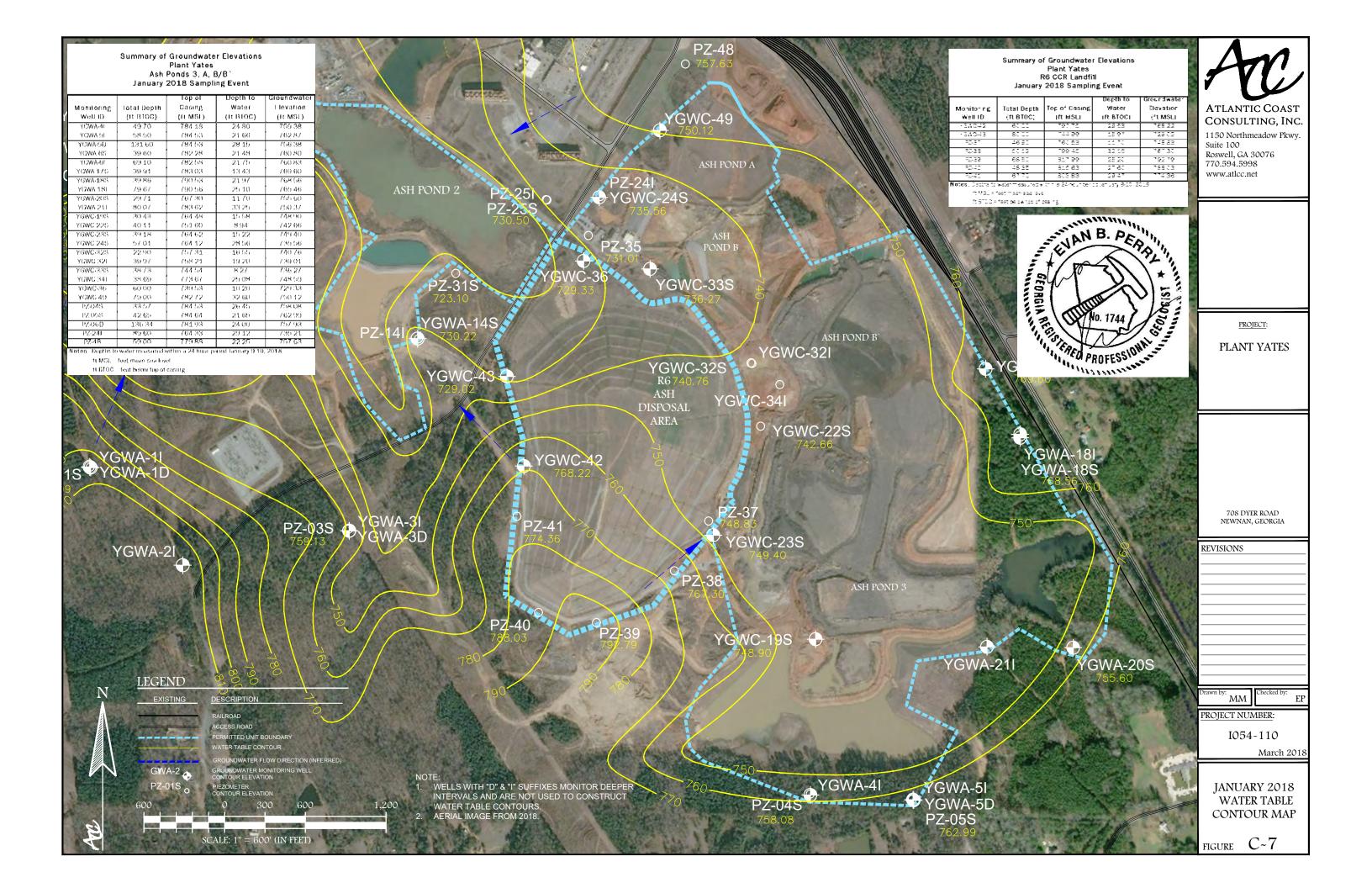


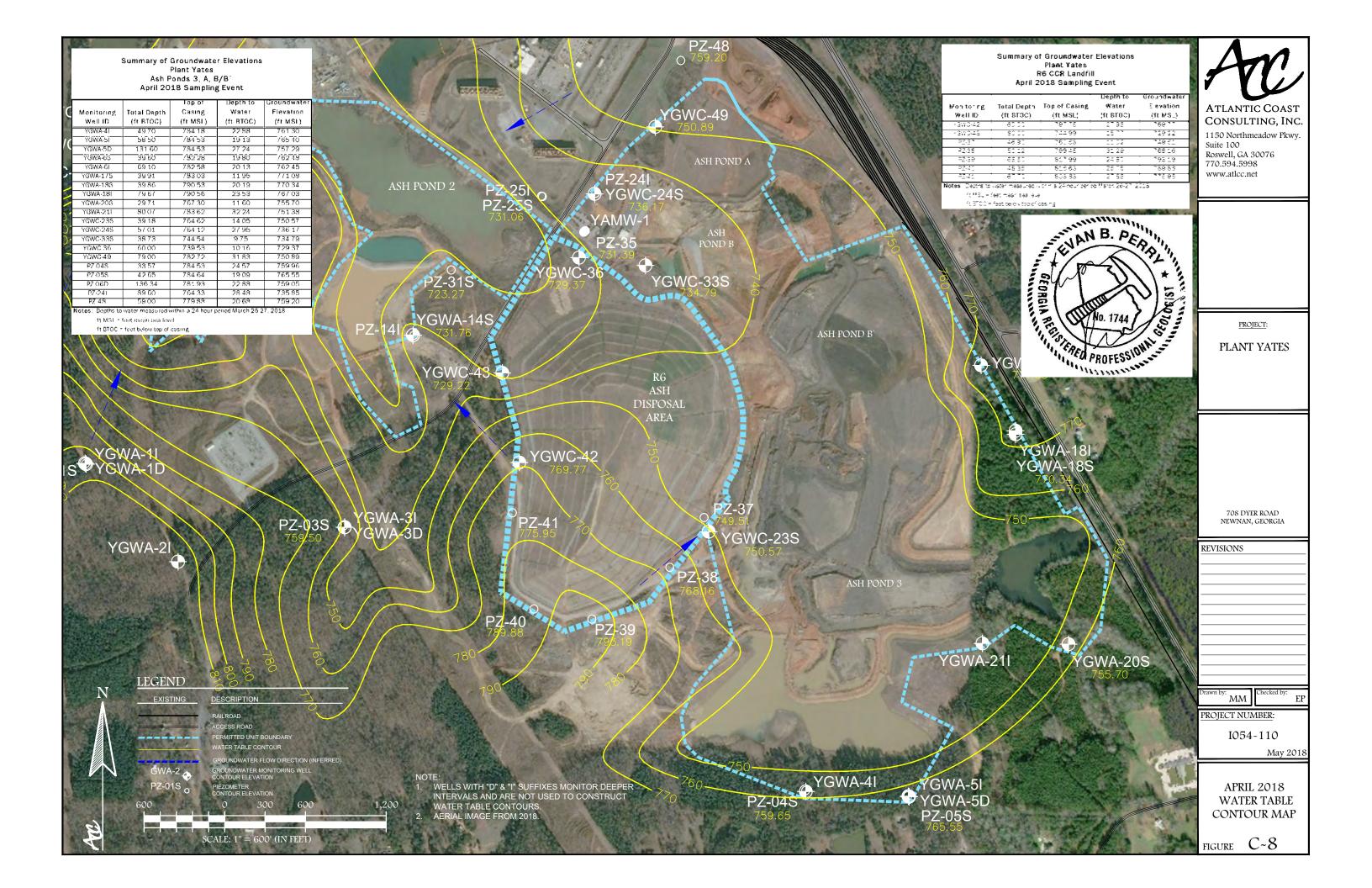


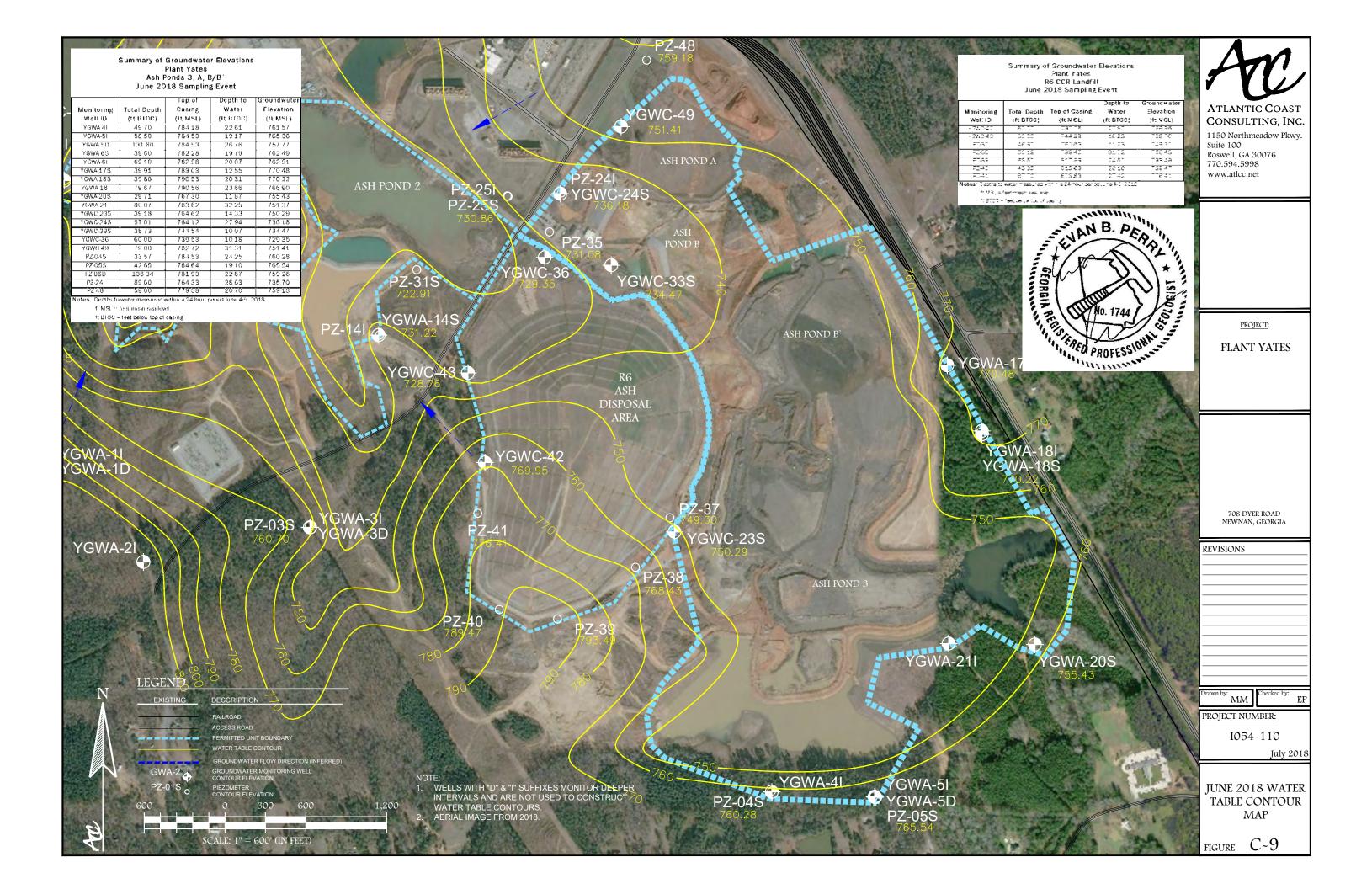


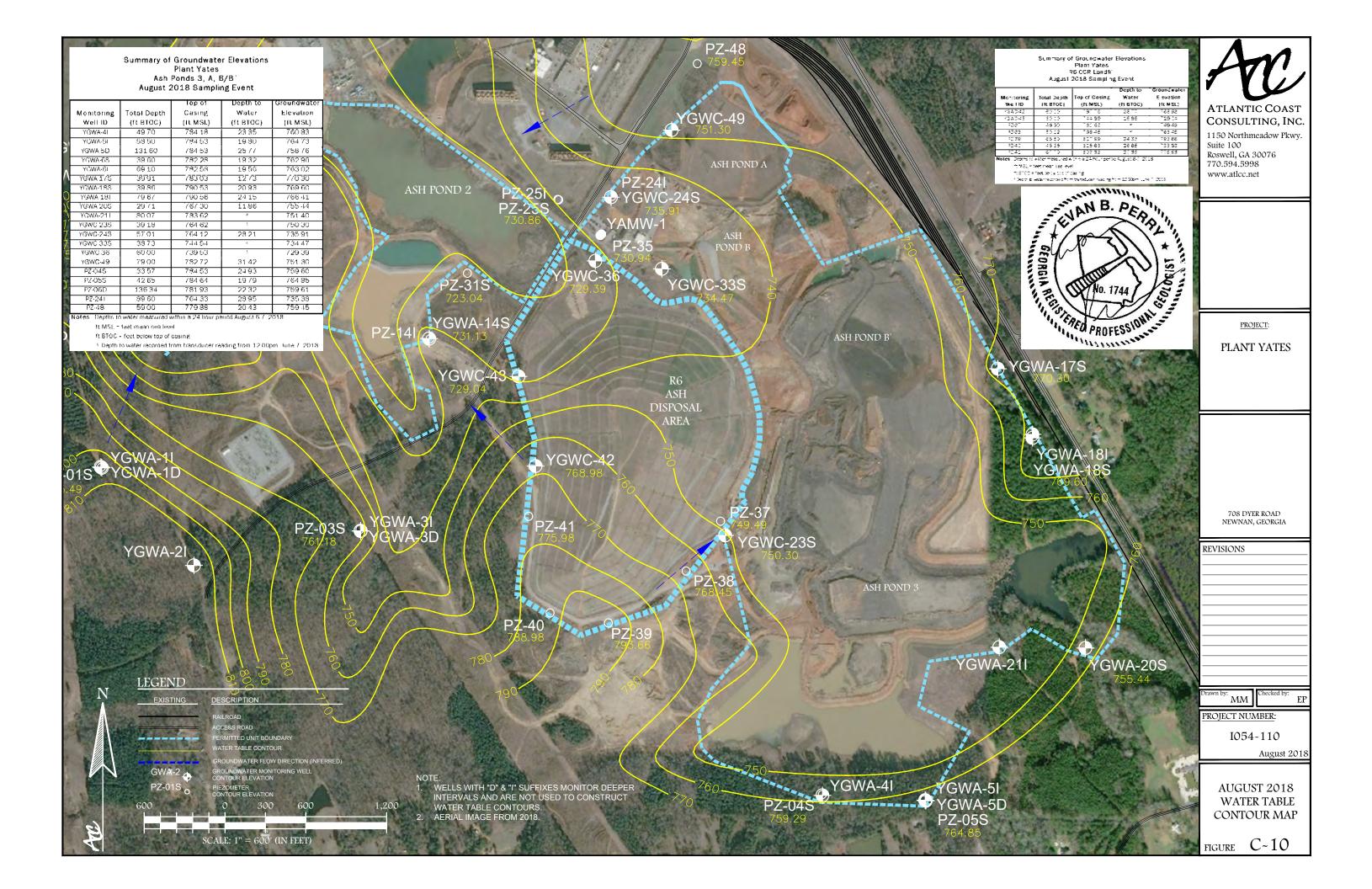


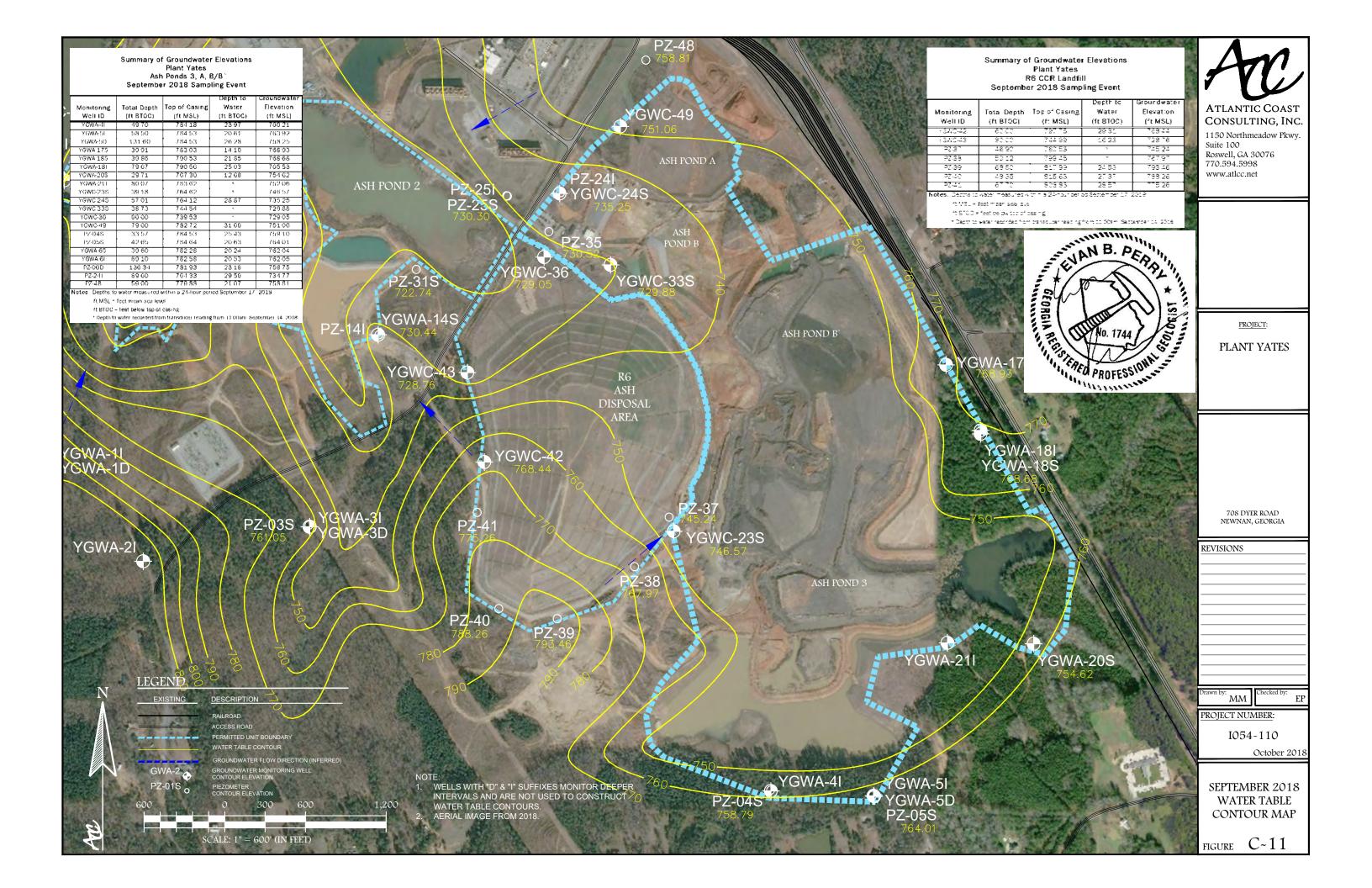


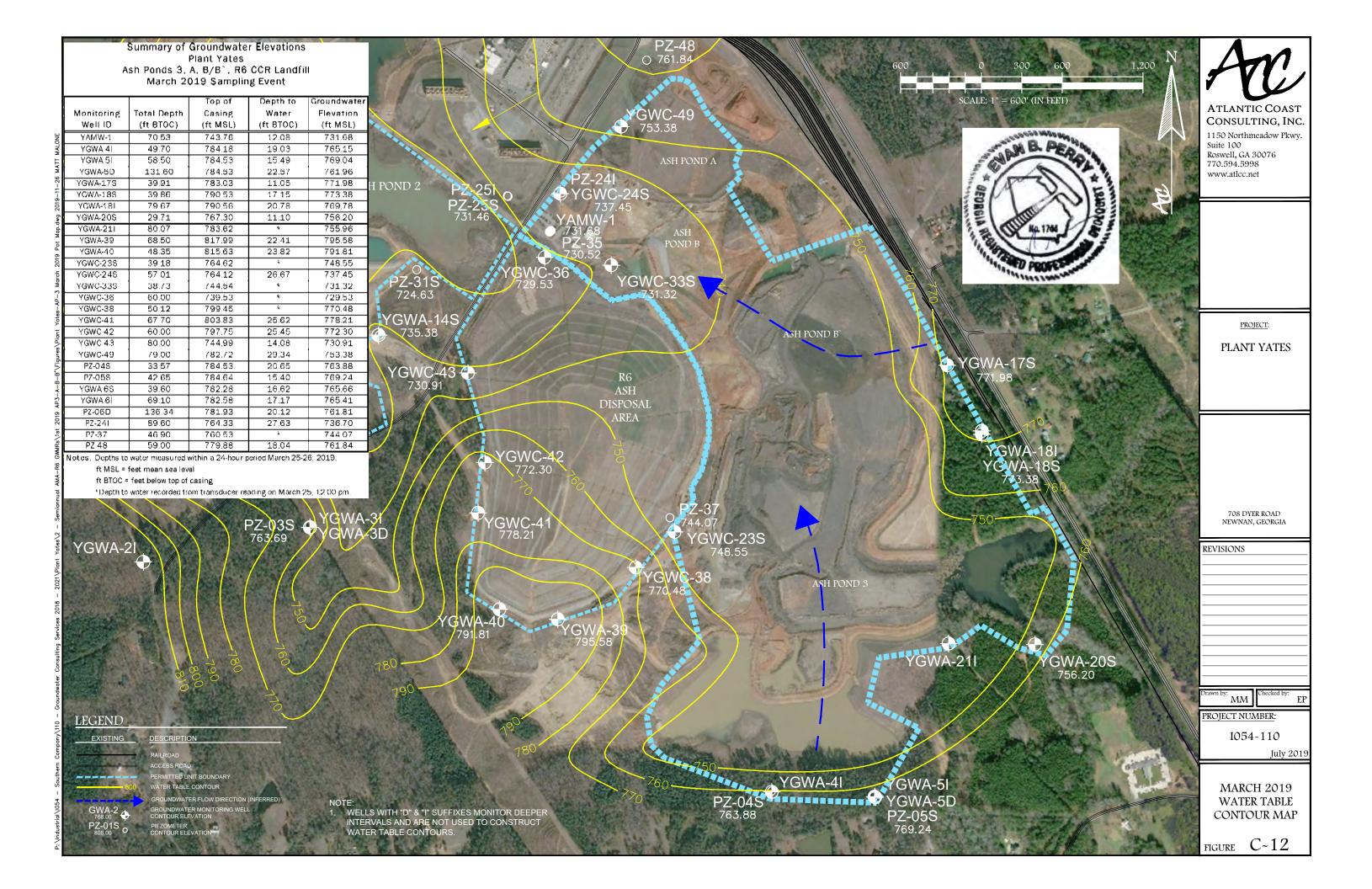


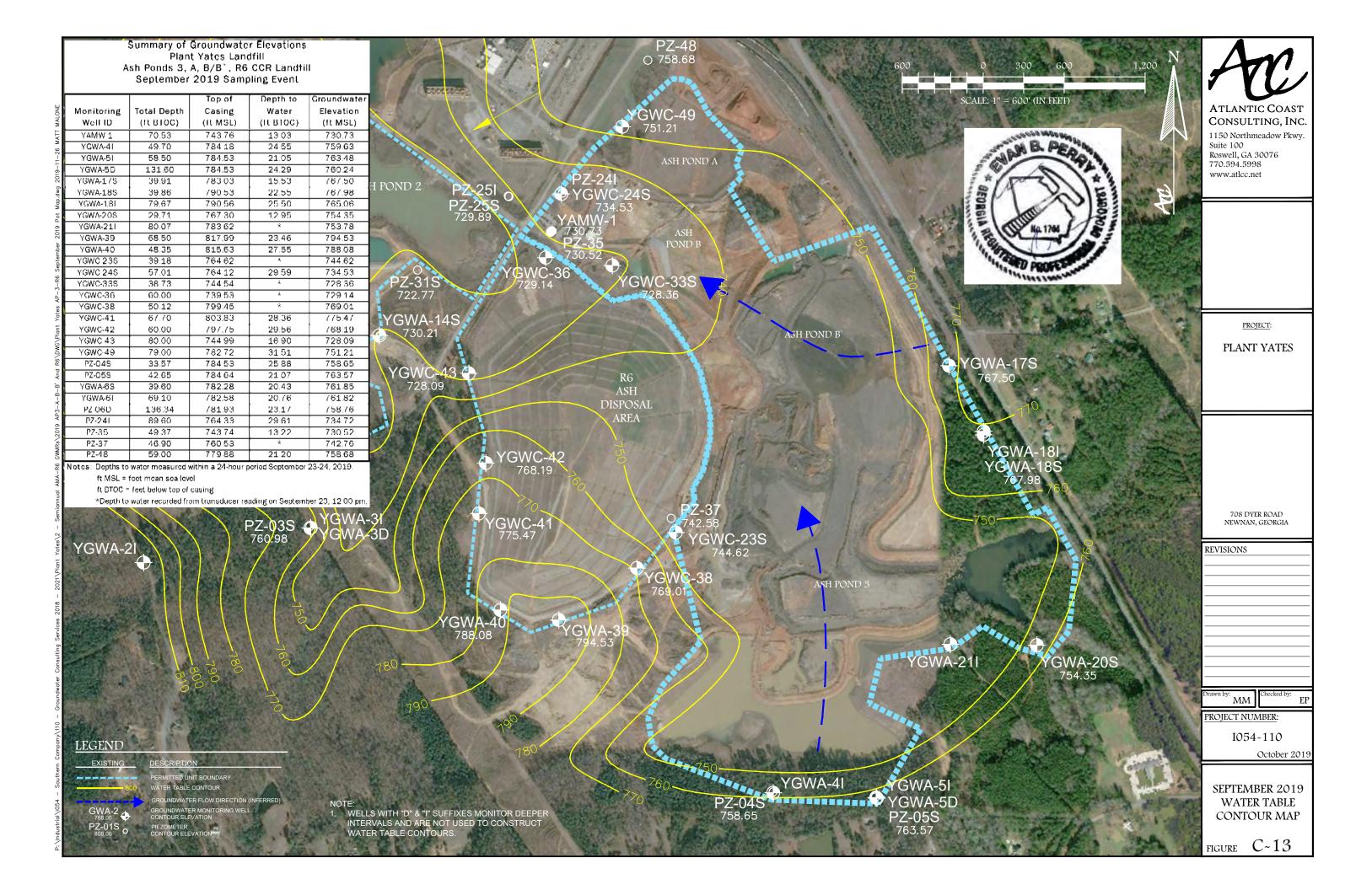


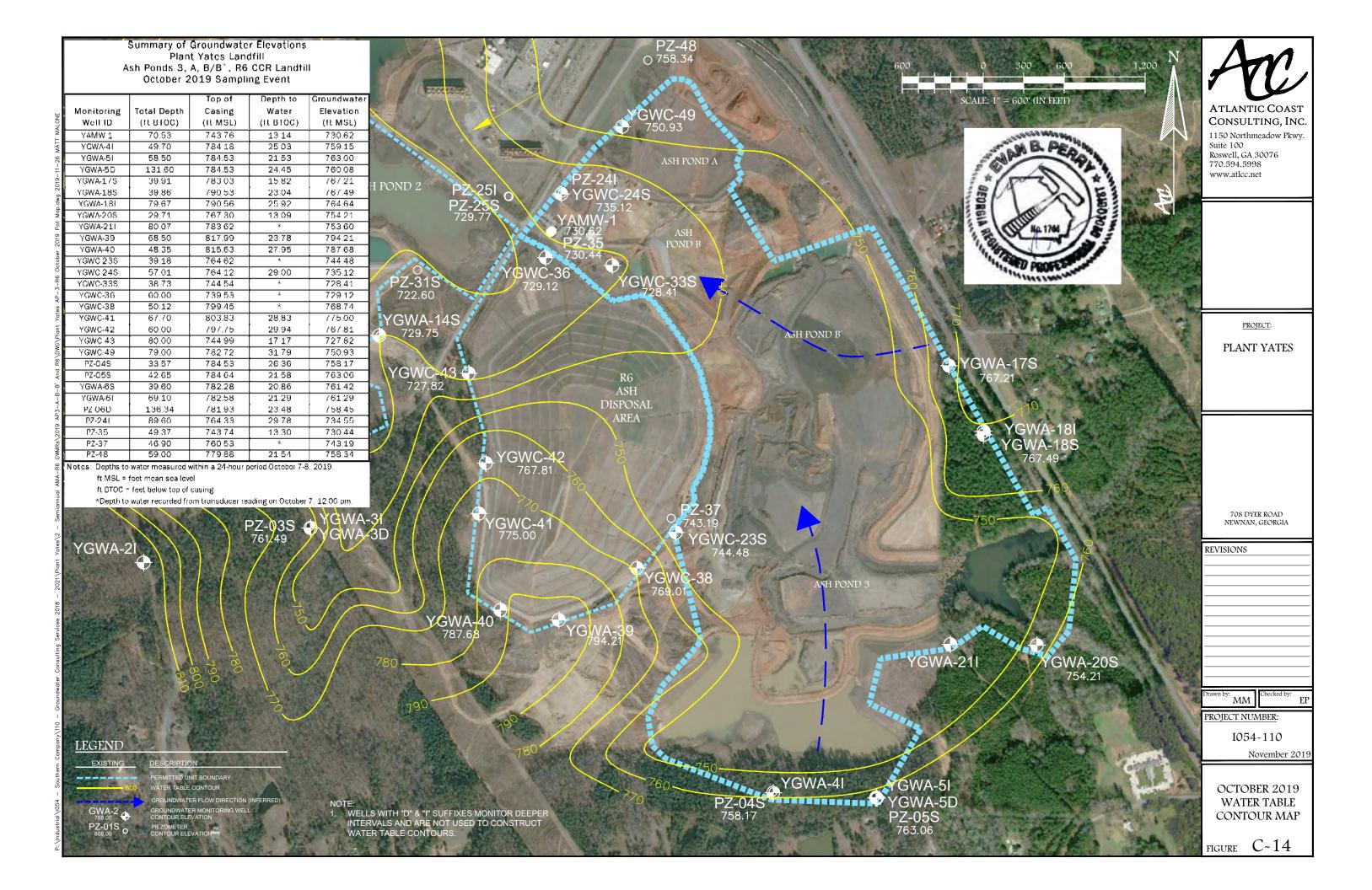


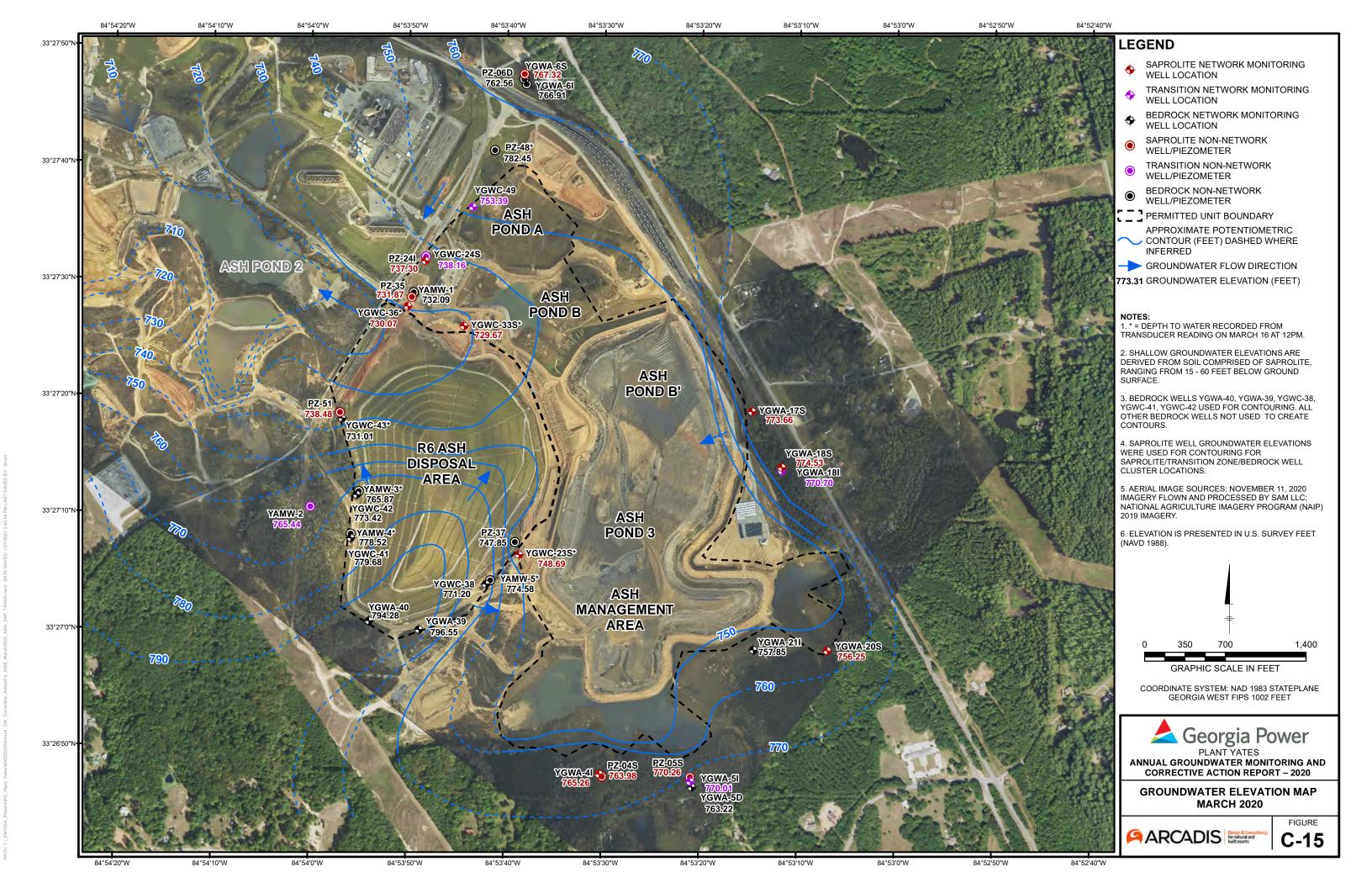












84°53'30"W

84°53'20"W

84°53'10"W

84°53'0"W

84°52'40"W

84°53'40"W

84°54'10"W

84°54'20"W

84°54'0"W

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
PZ-04S	33.57	784.53	27.41	757.12	10/31/2016
PZ-04S	33.57	784.53	27.50	757.03	1/9/2017
PZ-04S	33.57	784.53	26.09	758.44	2/20/2017
PZ-04S	33.57	784.53	24.89	759.64	4/25/2017
PZ-04S	33.57	784.53	25.20	759.33	6/26/2017
PZ-04S	33.57	784.53	26.22	758.31	10/2/2017
PZ-04S	33.57	784.53	26.45	758.08	1/9/2018
PZ-04S	33.57	784.53	24.57	759.96	3/26/2018
PZ-04S	33.57	784.53	24.25	760.28	6/4/2018
PZ-04S	33.57	784.53	24.93	759.60	8/6/2018
PZ-04S	33.57	784.53	25.43	759.10	9/17/2018
PZ-04S	33.57	784.53	21.45	763.08	2/25/2019
PZ-04S	33.57	784.53	20.65	763.88	3/25/2019
PZ-04S	33.57	784.53	24.59	759.94	8/19/2019
PZ-04S	33.57	784.53	25.88	758.65	9/23/2019
PZ-04S	33.57	784.53	26.36	758.17	10/7/2019
PZ-04S	33.57	784.25	24.18	760.07	2/10/2020
PZ-04S	33.57	784.25	20.27	763.98	3/16/2020
PZ-04S	33.57	784.25	24.95	759.30	9/21/2020
PZ-05S	42.65	784.64	22.37	762.27	10/31/2016
PZ-05S	42.65	784.64	22.10	762.54	1/9/2017
PZ-05S	42.65	784.64	20.20	764.44	2/20/2017
PZ-05S	42.65	784.64	18.71	765.93	4/25/2017
PZ-05S	42.65	784.64	19.80	764.84	6/26/2017
PZ-05S	42.65	784.64	21.32	763.32	10/2/2017
PZ-05S	42.65	784.64	21.65	762.99	1/9/2018
PZ-05S	42.65	784.64	19.09	765.55	3/26/2018
PZ-05S	42.65	784.64	19.10	765.54	6/4/2018
PZ-05S	42.65	784.64	19.79	764.85	8/6/2018
PZ-05S	42.65	784.64	20.63	764.01	9/17/2018
PZ-05S	42.65	784.64	15.62	769.02	2/25/2019
PZ-05S	42.65	784.64	15.40	769.24	3/25/2019
PZ-05S	42.65	784.64	19.69	764.95	8/19/2019
PZ-05S	42.65	784.64	21.07	763.57	9/23/2019
PZ-05S	42.65	784.64	21.58	763.06	10/7/2019
PZ-05S	42.65	784.64	18.25	766.39	2/10/2020
PZ-05S	42.65	784.64	14.38	770.26	3/16/2020
PZ-05S	42.65	784.64	19.81	764.83	9/21/2020

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

Monitoring Well ID	Total Depth (ft BTOC)	Top of Casing (ft MSL)	Depth to Water (ft BTOC)	Groundwater Elevation (ft MSL)	Date
PZ-06D	136.34	781.93	25.09	756.84	10/31/2016
PZ-06D	136.34	781.93	25.44	756.49	1/9/2017
PZ-06D	136.34	781.93	24.72	757.21	2/20/2017
PZ-06D	136.34	781.93	24.67	757.26	4/25/2017
PZ-06D	136.34	781.93	24.08	757.85	6/26/2017
PZ-06D	136.34	781.93	23.65	758.28	10/2/2017
PZ-06D	136.34	781.93	24.00	757.93	1/9/2018
PZ-06D	136.34	781.93	22.88	759.05	3/26/2018
PZ-06D	136.34	781.93	22.67	759.26	6/4/2018
PZ-06D	136.34	781.93	22.32	759.61	8/6/2018
PZ-06D	136.34	781.93	23.18	758.75	9/17/2018
PZ-06D	136.34	781.93	20.37	761.56	2/25/2019
PZ-06D	136.34	781.93	20.12	761.81	3/25/2019
PZ-06D	136.34	781.93	22.16	759.77	8/19/2019
PZ-06D	136.34	781.93	23.17	758.76	9/23/2019
PZ-06D	136.34	781.93	23.48	758.45	10/7/2019
PZ-06D	136.34	782.02	21.81	760.21	2/10/2020
PZ-06D	136.34	782.02	19.46	762.56	3/16/2020
PZ-06D	136.34	782.02	21.43	760.59	9/21/2020
PZ-24I	89.60	764.33	24.85	739.48	10/31/2016
PZ-24I	89.60	764.33	29.08	735.25	1/9/2017
PZ-24I	89.60	764.33	29.32	735.01	2/20/2017
PZ-24I	89.60	764.33	29.21	735.12	4/25/2017
PZ-24I	89.60	764.33	29.01	735.32	6/26/2017
PZ-24I	89.60	764.33	29.00	735.33	10/2/2017
PZ-24I	89.60	764.33	29.12	735.21	1/9/2018
PZ-24I	89.60	764.33	28.48	735.85	3/26/2018
PZ-24I	89.60	764.33	28.63	735.70	6/4/2018
PZ-24I	89.60	764.33	28.95	735.38	8/6/2018
PZ-24I	89.60	764.33	29.56	734.77	9/17/2018
PZ-24I	89.60	764.33	27.49	736.84	2/25/2019
PZ-24I	89.60	764.33	27.63	736.70	3/25/2019
PZ-24I	89.60	764.33	29.04	735.29	8/19/2019
PZ-24I	89.60	764.33	29.61	734.72	9/23/2019
PZ-24I	89.60	764.33	29.78	734.55	10/7/2019
PZ-24I	89.60	764.33	28.29	736.04	2/11/2020
PZ-24I	89.60	764.33	27.03	737.30	3/16/2020
PZ-24IA	89.53	764.33	29.13	735.20	9/21/2020

- 1. * = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
PZ-35	49.37	743.74	11.81	731.93	2/25/2019
PZ-35	49.37	743.74	13.22	730.52	3/25/2019
PZ-35	49.37	743.74	12.94	730.80	8/19/2019
PZ-35	49.37	743.74	13.22	730.52	9/23/2019
PZ-35	49.37	743.74	13.30	730.44	10/7/2019
PZ-35	49.37	743.81	12.14	731.67	2/11/2020
PZ-35	49.37	743.81	11.94	731.87	3/16/2020
PZ-35	49.37	743.81	15.44	728.37	9/21/2020
PZ-37	46.90	760.53	12.09	748.44	10/31/2016
PZ-37	46.90	760.53	12.08	748.45	1/9/2017
PZ-37	46.90	760.53	11.73	748.80	2/20/2017
PZ-37	46.90	760.53	11.58	748.95	4/25/2017
PZ-37	46.90	760.53	11.58	748.95	6/26/2017
PZ-37	46.90	760.53	11.39	749.14	10/2/2017
PZ-37	46.90	760.53	11.70	748.83	1/9/2018
PZ-37	46.90	760.53	11.02	749.51	3/26/2018
PZ-37	46.90	760.53	11.23	749.30	6/4/2018
PZ-37	46.90	760.53	*	749.49	7/7/2018
PZ-37	46.90	760.53	*	745.24	9/17/2018
PZ-37	46.90	760.53	*	744.22	2/25/2019
PZ-37	46.90	760.53	*	744.07	3/25/2019
PZ-37	46.90	760.53	*	744.07	8/19/2019
PZ-37	46.90	760.53	*	742.58	9/23/2019
PZ-37	46.90	760.53	*	743.19	10/7/2019
PZ-37	46.90	760.78	14.13*	746.65	2/11/2020
PZ-37	46.90	760.78	12.68*	748.10	3/16/2020
PZ-37	46.90	760.78	13.79	746.40	9/21/2020
PZ-48	59.00	779.88	23.50	756.38	10/31/2016
PZ-48	59.00	779.88	24.68	755.20	1/9/2017
PZ-48	59.00	779.88	24.82	755.06	2/20/2017
PZ-48	59.00	779.88	24.61	755.27	4/25/2017
PZ-48	59.00	779.88	23.92	755.96	6/26/2017
PZ-48	59.00	779.88	21.96	757.92	10/2/2017
PZ-48	59.00	779.88	22.25	757.63	1/9/2018
PZ-48	59.00	779.88	20.68	759.20	3/26/2018
PZ-48	59.00	779.88	20.70	759.18	6/4/2018
PZ-48	59.00	779.88	20.43	759.45	8/6/2018
PZ-48	59.00	779.88	21.07	758.81	9/17/2018

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
PZ-48	59.00	779.88	18.14	761.74	2/25/2019
PZ-48	59.00	779.88	18.04	761.84	3/25/2019
PZ-48	59.00	779.88	20.23	759.65	8/19/2019
PZ-48	59.00	779.88	21.20	758.68	9/23/2019
PZ-48	59.00	779.88	21.54	758.34	10/7/2019
PZ-48	59.00	779.83	19.43	760.40	2/10/2020
PZ-48	59.00	779.83	17.38	762.45	3/16/2020
PZ-48	59.00	799.83	19.94	779.89	9/21/2020
PZ-51	36.32	744.30	6.69	737.61	2/11/2020
PZ-51	36.32	744.30	5.82	738.48	3/16/2020
PZ-51	36.32	744.30	6.94	737.36	9/21/2020
YAMW-1	69.66	743.76	11.54	732.22	2/25/2019
YAMW-1	69.66	743.76	12.08	731.68	3/25/2019
YAMW-1	69.66	743.76	12.74	731.02	8/19/2019
YAMW-1	69.66	743.76	13.03	730.73	9/23/2019
YAMW-1	69.66	743.76	13.14	730.62	10/7/2019
YAMW-1	69.66	743.83	11.99	731.84	2/11/2020
YAMW-1	69.66	743.83	11.74	732.09	3/16/2020
YAMW-1	69.66	743.83	15.14	728.69	9/21/2020
YAMW-2	46.48	781.04	18.60	762.44	2/11/2020
YAMW-2	46.48	781.04	15.60	765.44	3/16/2020
YAMW-2	46.48	781.04	22.18	758.86	9/21/2020
YAMW-3	91.44	796.05	35.36	760.69	2/10/2020
YAMW-3	91.44	796.05	30.18	765.87	3/16/2020
YAMW-3	91.44	796.05	35.28	760.77	9/21/2020
YAMW-4	96.55	805.59	31.19	774.40	2/11/2020
YAMW-4	96.55	805.59	27.07	778.52	3/16/2020
YAMW-4	96.55	805.59	30.61	774.98	9/21/2020
YAMW-5	90.34	788.90	31.73	757.17	2/11/2020
YAMW-5	90.34	788.90	14.32	774.58	3/16/2020
YAMW-5	90.34	788.90	12.84	776.06	9/21/2020
YGWA-17S	39.91	783.03	15.98	767.05	10/31/2016
YGWA-17S	39.91	783.03	13.93	769.10	1/9/2017
YGWA-17S	39.91	783.03	12.70	770.33	2/20/2017
YGWA-17S	39.91	783.03	12.53	770.50	4/25/2017
YGWA-17S	39.91	783.03	12.03	771.00	6/26/2017
YGWA-17S	39.91	783.03	12.94	770.09	10/2/2017
YGWA-17S	39.91	783.03	13.43	769.60	1/9/2018

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWA-17S	39.91	783.03	11.95	771.08	3/26/2018
YGWA-17S	39.91	783.03	12.55	770.48	6/4/2018
YGWA-17S	39.91	783.03	12.73	770.30	8/6/2018
YGWA-17S	39.91	783.03	14.10	768.93	9/17/2018
YGWA-17S	39.91	783.03	10.02	773.01	2/25/2019
YGWA-17S	39.91	783.03	11.05	771.98	3/25/2019
YGWA-17S	39.91	783.03	14.63	768.40	8/19/2019
YGWA-17S	39.91	783.03	15.53	767.50	9/23/2019
YGWA-17S	39.91	783.03	15.82	767.21	10/7/2019
YGWA-17S	39.91	783.05	10.49	772.56	2/10/2020
YGWA-17S	39.91	783.05	9.39	773.66	3/16/2020
YGWA-17S	39.91	783.05	12.62	770.43	9/21/2020
YGWA-18I	79.67	790.56	26.08	764.48	10/31/2016
YGWA-18I	79.67	790.56	25.86	764.70	1/9/2017
YGWA-18I	79.67	790.56	24.80	765.76	2/20/2017
YGWA-18I	79.67	790.56	24.10	766.46	4/25/2017
YGWA-18I	79.67	790.56	24.14	766.42	6/26/2017
YGWA-18I	79.67	790.56	24.65	765.91	10/2/2017
YGWA-18I	79.67	790.56	25.10	765.46	1/9/2018
YGWA-18I	79.67	790.56	23.53	767.03	3/26/2018
YGWA-18I	79.67	790.56	23.66	766.90	6/4/2018
YGWA-18I	79.67	790.56	24.15	766.41	8/6/2018
YGWA-18I	79.67	790.56	25.03	765.53	9/17/2018
YGWA-18I	79.67	790.56	20.92	769.64	2/25/2019
YGWA-18I	79.67	790.56	20.78	769.78	3/25/2019
YGWA-18I	79.67	790.56	24.37	766.19	8/19/2019
YGWA-18I	79.67	790.56	25.50	765.06	9/23/2019
YGWA-18I	79.67	790.56	25.92	764.64	10/7/2019
YGWA-18I	79.67	790.57	22.57	768.00	2/10/2020
YGWA-18I	79.67	790.57	19.87	770.70	3/16/2020
YGWA-18I	79.67	790.57	23.59	766.98	9/21/2020
YGWA-18S	39.86	790.53	23.76	766.77	10/31/2016
YGWA-18S	39.86	790.53	23.44	767.09	1/9/2017
YGWA-18S	39.86	790.53	22.14	768.39	2/20/2017
YGWA-18S	39.86	790.53	21.00	769.53	4/25/2017
YGWA-18S	39.86	790.53	21.07	769.46	6/26/2017
YGWA-18S	39.86	790.53	26.71	763.82	10/2/2017
YGWA-18S	39.86	790.53	21.97	768.56	1/9/2018

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- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWA-18S	39.86	790.53	20.19	770.34	3/26/2018
YGWA-18S	39.86	790.53	20.31	770.22	6/4/2018
YGWA-18S	39.86	790.53	20.93	769.60	8/6/2018
YGWA-18S	39.86	790.53	21.85	768.68	9/17/2018
YGWA-18S	39.86	790.53	17.35	773.18	2/25/2019
YGWA-18S	39.86	790.53	17.15	773.38	3/25/2019
YGWA-18S	39.86	790.53	21.22	769.31	8/19/2019
YGWA-18S	39.86	790.53	22.55	767.98	9/23/2019
YGWA-18S	39.86	790.53	23.04	767.49	10/7/2019
YGWA-18S	39.86	790.57	19.43	771.14	2/10/2020
YGWA-18S	39.86	790.57	16.04	774.53	3/16/2020
YGWA-18S	39.86	790.57	20.39	770.18	9/21/2020
YGWA-20S	29.71	767.30	9.94	757.36	10/31/2016
YGWA-20S	29.71	767.30	9.78	757.52	1/9/2017
YGWA-20S	29.71	767.30	9.74	757.56	2/20/2017
YGWA-20S	29.71	767.30	9.85	757.45	4/25/2017
YGWA-20S	29.71	767.30	11.74	755.56	6/26/2017
YGWA-20S	29.71	767.30	12.38	754.92	10/2/2017
YGWA-20S	29.71	767.30	11.70	755.60	1/9/2018
YGWA-20S	29.71	767.30	11.60	755.70	3/26/2018
YGWA-20S	29.71	767.30	11.87	755.43	6/4/2018
YGWA-20S	29.71	767.30	11.86	755.44	8/6/2018
YGWA-20S	29.71	767.30	12.68	754.62	9/17/2018
YGWA-20S	29.71	767.30	11.01	756.29	2/25/2019
YGWA-20S	29.71	767.30	11.10	756.20	3/25/2019
YGWA-20S	29.71	767.30	12.46	754.84	8/19/2019
YGWA-20S	29.71	767.30	12.95	754.35	9/23/2019
YGWA-20S	29.71	767.30	13.09	754.21	10/7/2019
YGWA-20S	29.71	767.12	11.05	756.07	2/10/2020
YGWA-20S	29.71	767.12	10.87	756.25	3/16/2020
YGWA-20S	29.71	767.12	11.44	755.68	9/21/2020
YGWA-21I	80.07	783.62	31.29	752.33	10/31/2016
YGWA-21I	80.07	783.62	31.30	752.32	1/9/2017
YGWA-21I	80.07	783.62	30.94	752.68	2/20/2017
YGWA-21I	80.07	783.62	30.96	752.66	4/25/2017
YGWA-21I	80.07	783.62	30.77	752.85	6/26/2017
YGWA-21I	80.07	783.62	32.90	750.72	10/2/2017
YGWA-21I	80.07	783.62	33.25	750.37	1/9/2018

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- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWA-21I	80.07	783.62	32.24	751.38	3/26/2018
YGWA-21I	80.07	783.62	32.25	751.37	6/4/2018
YGWA-21I	80.07	783.62	*	751.40	7/7/2018
YGWA-21I	80.07	783.62	*	752.06	9/14/2018
YGWA-21I	80.07	783.62	*	755.51	2/25/2019
YGWA-21I	80.07	783.62	*	755.96	3/25/2019
YGWA-21I	80.07	783.62	*	755.96	8/19/2019
YGWA-21I	80.07	783.62	*	753.78	9/23/2019
YGWA-21I	80.07	783.62	*	753.60	10/7/2019
YGWA-21I	80.07	783.70	27.52*	756.18	2/11/2020
YGWA-21I	80.07	783.70	25.77*	757.93	3/16/2020
YGWA-21I	80.07	783.70	31.29	756.10	9/21/2020
YGWA-39	68.50	817.99	24.94	793.05	10/31/2016
YGWA-39	68.50	817.99	26.58	791.41	1/9/2017
YGWA-39	68.50	817.99	26.38	791.61	2/20/2017
YGWA-39	68.50	817.99	26.17	791.82	4/25/2017
YGWA-39	68.50	817.99	25.82	792.17	6/26/2017
YGWA-39	68.50	817.99	24.80	793.19	10/2/2017
YGWA-39	68.50	817.99	25.20	792.79	1/9/2018
YGWA-39	68.50	817.99	24.80	793.19	3/26/2018
YGWA-39	68.50	817.99	24.50	793.49	6/4/2018
YGWA-39	68.50	817.99	24.33	793.66	8/6/2018
YGWA-39	68.50	817.99	24.53	793.46	9/17/2018
YGWA-39	68.50	817.99	24.15	793.84	2/25/2019
YGWA-39	68.50	817.99	22.41	795.58	3/25/2019
YGWA-39	68.50	817.99	22.88	795.11	8/19/2019
YGWA-39	68.50	817.99	23.46	794.53	9/23/2019
YGWA-39	68.50	817.99	23.78	794.21	10/7/2019
YGWA-39	68.50	818.19	23.45	794.74	2/11/2020
YGWA-39	68.50	818.19	21.64	796.55	3/16/2020
YGWA-39	68.50	818.19	21.81	796.38	9/21/2020
YGWA-40	48.35	815.63	29.19	786.44	10/31/2016
YGWA-40	48.35	815.63	29.48	786.15	1/9/2017
YGWA-40	48.35	815.63	28.53	787.10	2/20/2017
YGWA-40	48.35	815.63	27.46	788.17	4/25/2017
YGWA-40	48.35	815.63	27.27	788.36	6/26/2017
YGWA-40	48.35	815.63	27.46	788.17	10/2/2017
YGWA-40	48.35	815.63	27.60	788.03	1/9/2018

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- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWA-40	48.35	815.63	25.75	789.88	3/26/2018
YGWA-40	48.35	815.63	26.16	789.47	6/4/2018
YGWA-40	48.35	815.63	26.65	788.98	8/6/2018
YGWA-40	48.35	815.63	27.37	788.26	9/17/2018
YGWA-40	48.35	815.63	23.89	791.74	2/25/2019
YGWA-40	48.35	815.63	23.82	791.81	3/25/2019
YGWA-40	48.35	815.63	26.54	789.09	8/19/2019
YGWA-40	48.35	815.63	27.55	788.08	9/23/2019
YGWA-40	48.35	815.63	27.95	787.68	10/7/2019
YGWA-40	48.35	815.73	24.90	790.83	2/11/2020
YGWA-40	48.35	815.73	21.45	794.28	3/16/2020
YGWA-40	48.35	815.73	25.44	790.29	9/21/2020
YGWA-4I	49.70	784.18	26.06	758.12	10/31/2016
YGWA-4I	49.70	784.18	26.13	758.05	1/9/2017
YGWA-4I	49.70	784.18	24.72	759.46	2/20/2017
YGWA-4I	49.70	784.18	23.43	760.75	4/25/2017
YGWA-4I	49.70	784.18	23.67	760.51	6/26/2017
YGWA-4I	49.70	784.18	24.61	759.57	10/2/2017
YGWA-4I	49.70	784.18	24.80	759.38	1/9/2018
YGWA-4I	49.70	784.18	22.88	761.30	3/26/2018
YGWA-4I	49.70	784.18	22.61	761.57	6/4/2018
YGWA-4I	49.70	784.18	23.35	760.83	8/6/2018
YGWA-4I	49.70	784.18	23.97	760.21	9/17/2018
YGWA-4I	49.70	784.18	19.79	764.39	2/25/2019
YGWA-4I	49.70	784.18	19.03	765.15	3/25/2019
YGWA-4I	49.70	784.18	23.17	761.01	8/19/2019
YGWA-4I	49.70	784.18	24.55	759.63	9/23/2019
YGWA-4I	49.70	784.18	25.03	759.15	10/7/2019
YGWA-4I	49.70	784.21	22.66	761.55	2/10/2020
YGWA-4I	49.70	784.21	18.95	765.26	3/16/2020
YGWA-4I	49.70	784.21	23.45	760.76	9/21/2020
YGWA-5D	131.60	784.53	15.21	769.32	10/31/2016
YGWA-5D	131.60	784.53	15.60	768.93	1/9/2017
YGWA-5D	131.60	784.53	14.14	770.39	2/20/2017
YGWA-5D	131.60	784.53	15.52	769.01	4/25/2017
YGWA-5D	131.60	784.53	26.64	757.89	6/26/2017
YGWA-5D	131.60	784.53	28.99	755.54	10/2/2017
YGWA-5D	131.60	784.53	28.15	756.38	1/9/2018

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWA-5D	131.60	784.53	27.24	757.29	3/26/2018
YGWA-5D	131.60	784.53	26.76	757.77	6/4/2018
YGWA-5D	131.60	784.53	25.77	758.76	8/6/2018
YGWA-5D	131.60	784.53	26.28	758.25	9/17/2018
YGWA-5D	131.60	784.53	23.91	760.62	2/25/2019
YGWA-5D	131.60	784.53	22.57	761.96	3/25/2019
YGWA-5D	131.60	784.53	23.32	761.21	8/19/2019
YGWA-5D	131.60	784.53	24.29	760.24	9/23/2019
YGWA-5D	131.60	784.53	24.45	760.08	10/7/2019
YGWA-5D	131.60	784.53	22.81	761.72	2/10/2020
YGWA-5D	131.60	784.53	21.31	763.22	3/16/2020
YGWA-5D	131.60	784.53	22.51	762.02	9/21/2020
YGWA-5I	58.50	784.53	22.24	762.29	10/31/2016
YGWA-5I	58.50	784.53	22.03	762.50	1/9/2017
YGWA-5I	58.50	784.53	20.18	764.35	2/20/2017
YGWA-5I	58.50	784.53	18.72	765.81	4/25/2017
YGWA-5I	58.50	784.53	19.89	764.64	6/26/2017
YGWA-5I	58.50	784.53	21.32	763.21	10/2/2017
YGWA-5I	58.50	784.53	21.66	762.87	1/9/2018
YGWA-5I	58.50	784.53	19.13	765.40	3/26/2018
YGWA-5I	58.50	784.53	19.17	765.36	6/4/2018
YGWA-5I	58.50	784.53	19.80	764.73	8/6/2018
YGWA-5I	58.50	784.53	20.61	763.92	9/17/2018
YGWA-5I	58.50	784.53	15.75	768.78	2/25/2019
YGWA-5I	58.50	784.53	15.49	769.04	3/25/2019
YGWA-5I	58.50	784.53	19.68	764.85	8/19/2019
YGWA-5I	58.50	784.53	21.05	763.48	9/23/2019
YGWA-5I	58.50	784.53	21.53	763.00	10/7/2019
YGWA-5I	58.50	784.54	18.33	766.21	2/10/2020
YGWA-5I	58.50	784.54	14.53	770.01	3/16/2020
YGWA-5I	58.50	784.54	19.82	764.72	9/21/2020
YGWA-6I	69.10	782.58	22.74	759.84	10/31/2016
YGWA-6I	69.10	782.58	23.23	759.35	1/9/2017
YGWA-6I	69.10	782.58	22.31	760.27	2/20/2017
YGWA-6I	69.10	782.58	21.99	760.59	4/25/2017
YGWA-6I	69.10	782.58	21.50	761.08	6/26/2017
YGWA-6I	69.10	782.58	20.59	761.99	10/2/2017
YGWA-6I	69.10	782.58	21.75	760.83	1/9/2018

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- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWA-6I	69.10	782.58	20.13	762.45	3/26/2018
YGWA-6I	69.10	782.58	20.07	762.51	6/4/2018
YGWA-6I	69.10	782.58	19.56	763.02	8/6/2018
YGWA-6I	69.10	782.58	20.53	762.05	9/17/2018
YGWA-6I	69.10	782.58	17.12	765.46	2/25/2019
YGWA-6I	69.10	782.58	17.17	765.41	3/25/2019
YGWA-6I	69.10	782.58	19.61	762.97	8/19/2019
YGWA-6I	69.10	782.58	20.76	761.82	9/23/2019
YGWA-6I	69.10	782.58	21.29	761.29	10/7/2019
YGWA-6I	69.10	782.73	17.74	764.99	2/10/2020
YGWA-6I	69.10	782.73	15.82	766.91	3/16/2020
YGWA-6I	69.10	782.73	18.48	764.25	9/21/2020
YGWA-6S	39.60	782.28	22.43	759.85	10/31/2016
YGWA-6S	39.60	782.28	23.02	759.26	1/9/2017
YGWA-6S	39.60	782.28	21.98	760.30	2/20/2017
YGWA-6S	39.60	782.28	21.58	760.70	4/25/2017
YGWA-6S	39.60	782.28	21.26	761.02	6/26/2017
YGWA-6S	39.60	782.28	20.27	762.01	10/2/2017
YGWA-6S	39.60	782.28	21.48	760.80	1/9/2018
YGWA-6S	39.60	782.28	19.80	762.48	3/26/2018
YGWA-6S	39.60	782.28	19.79	762.49	6/4/2018
YGWA-6S	39.60	782.28	19.32	762.96	8/6/2018
YGWA-6S	39.60	782.28	20.24	762.04	9/17/2018
YGWA-6S	39.60	782.28	16.66	765.62	2/25/2019
YGWA-6S	39.60	782.28	16.62	765.66	3/25/2019
YGWA-6S	39.60	782.28	19.27	763.01	8/19/2019
YGWA-6S	39.60	782.28	20.43	761.85	9/23/2019
YGWA-6S	39.60	782.28	20.86	761.42	10/7/2019
YGWA-6S	39.60	782.47	17.40	765.07	2/10/2020
YGWA-6S	39.60	782.47	15.15	767.32	3/16/2020
YGWA-6S	39.60	782.47	18.22	764.25	9/21/2020
YGWC-19S	30.43	764.48	18.12	746.36	10/31/2016
YGWC-19S	30.43	764.48	14.52	749.96	1/9/2017
YGWC-19S	30.43	764.48	16.02	748.46	2/20/2017
YGWC-19S	30.43	764.48	16.13	748.35	4/25/2017
YGWC-19S	30.43	764.48	15.55	748.93	6/26/2017
YGWC-19S	30.43	764.48	15.61	748.87	10/2/2017
YGWC-19S	30.43	764.48	15.58	748.90	1/9/2018

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWC-22S	40.11	751.60	10.30	741.30	10/31/2016
YGWC-22S	40.11	751.60	9.62	741.98	1/9/2017
YGWC-22S	40.11	751.60	9.41	742.19	2/20/2017
YGWC-22S	40.11	751.60	9.32	742.28	4/25/2017
YGWC-22S	40.11	751.60	8.86	742.74	6/26/2017
YGWC-22S	40.11	751.60	8.97	742.63	10/2/2017
YGWC-22S	40.11	751.60	8.94	742.66	1/9/2018
YGWC-23S	39.18	764.62	17.45	747.17	10/31/2016
YGWC-23S	39.18	764.62	15.52	749.10	1/9/2017
YGWC-23S	39.18	764.62	14.91	749.71	2/20/2017
YGWC-23S	39.18	764.62	15.07	749.55	4/25/2017
YGWC-23S	39.18	764.62	14.46	750.16	6/26/2017
YGWC-23S	39.18	764.62	14.91	749.71	10/2/2017
YGWC-23S	39.18	764.62	15.22	749.40	1/9/2018
YGWC-23S	39.18	764.62	14.05	750.57	3/26/2018
YGWC-23S	39.18	764.62	14.33	750.29	6/4/2018
YGWC-23S	39.18	764.62	*	750.30	7/7/2018
YGWC-23S	39.18	764.62	*	746.57	9/14/2018
YGWC-23S	39.18	764.62	*	749.43	2/25/2019
YGWC-23S	39.18	764.62	*	748.55	3/25/2019
YGWC-23S	39.18	764.62	*	748.55	8/19/2019
YGWC-23S	39.18	764.62	*	744.62	9/23/2019
YGWC-23S	39.18	764.62	*	744.48	10/7/2019
YGWC-23S	39.18	764.91	17.18*	747.73	2/11/2020
YGWC-23S	39.18	764.91	15.93*	748.98	3/16/2020
YGWC-23S	39.18	794.91	17.61	747.44	9/21/2020
YGWC-24S	57.01	764.12	29.14	734.98	10/31/2016
YGWC-24S	57.01	764.12	29.00	735.12	1/9/2017
YGWC-24S	57.01	764.12	28.59	735.53	2/20/2017
YGWC-24S	57.01	764.12	28.49	735.63	4/25/2017
YGWC-24S	57.01	764.12	28.30	735.82	6/26/2017
YGWC-24S	57.01	764.12	28.36	735.76	10/2/2017
YGWC-24S	57.01	764.12	28.56	735.56	1/9/2018
YGWC-24S	57.01	764.12	27.95	736.17	3/26/2018
YGWC-24S	57.01	764.12	27.94	736.18	6/4/2018
YGWC-24S	57.01	764.12	28.21	735.91	8/6/2018
YGWC-24S	57.01	764.12	28.87	735.25	9/17/2018
YGWC-24S	57.01	764.12	26.56	737.56	2/25/2019

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

				Groundwater	
Monitoring	Total Depth	Top of Casing	Depth to Water	Elevation	
Well ID	(ft BTOC)	(ft MSL)	(ft BTOC)	(ft MSL)	Date
YGWC-24S	57.01	764.12	26.67	737.45	3/25/2019
YGWC-24S	57.01	764.12	28.23	735.89	8/19/2019
YGWC-24S	57.01	764.12	29.59	734.53	9/23/2019
YGWC-24S	57.01	764.12	29.00	735.12	10/7/2019
YGWC-24S	57.01	764.12	27.53	736.59	2/11/2020
YGWC-24S	57.01	764.12	25.96	738.16	3/16/2020
YGWC-24SA	57.00	765.00	28.77	736.23	9/21/2020
YGWC-32I	39.97	758.21	20.12	738.09	10/31/2016
YGWC-32I	39.97	758.21	19.65	738.56	1/9/2017
YGWC-32I	39.97	758.21	19.60	738.61	2/20/2017
YGWC-32I	39.97	758.21	8.72	749.49	4/25/2017
YGWC-32I	39.97	758.21	19.21	739.00	6/26/2017
YGWC-32I	39.97	758.21	14.44	743.77	10/2/2017
YGWC-32I	39.97	758.21	19.20	739.01	1/9/2018
YGWC-32S	22.90	757.31	17.24	740.07	10/31/2016
YGWC-32S	22.90	757.31	16.69	740.62	1/9/2017
YGWC-32S	22.90	757.31	16.52	740.79	2/20/2017
YGWC-32S	22.90	757.31	19.70	737.61	4/25/2017
YGWC-32S	22.90	757.31	16.65	740.66	6/26/2017
YGWC-32S	22.90	757.31	8.57	748.74	10/2/2017
YGWC-32S	22.90	757.31	16.55	740.76	1/9/2018
YGWC-33S	38.73	744.54	9.49	735.05	10/31/2016
YGWC-33S	38.73	744.54	8.75	735.79	1/9/2017
YGWC-33S	38.73	744.54	8.53	736.01	2/20/2017
YGWC-33S	38.73	744.54	8.86	735.68	5/3/2017
YGWC-33S	38.73	744.54	8.28	736.26	6/26/2017
YGWC-33S	38.73	744.54	8.38	736.16	10/2/2017
YGWC-33S	38.73	744.54	8.27	736.27	1/9/2018
YGWC-33S	38.73	744.54	9.75	734.79	3/26/2018
YGWC-33S	38.73	744.54	10.07	734.47	6/4/2018
YGWC-33S	38.73	744.54	*	734.47	7/7/2018
YGWC-33S	38.73	744.54	*	729.88	9/14/2018
YGWC-33S	38.73	744.54	*	732.07	2/25/2019
YGWC-33S	38.73	744.54	*	731.32	3/25/2019
YGWC-33S	38.73	744.54	*	731.32	8/19/2019
YGWC-33S	38.73	744.54	*	728.36	9/23/2019
YGWC-33S	38.73	744.54	*	728.41	10/7/2019
YGWC-33S	38.73	744.54	15.41*	729.13	2/11/2020

- 1. \star = indicates depth to water recorded from transducer.
- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

Groundwater **Monitoring Total Depth Top of Casing Depth to Water** Elevation Well ID (ft BTOC) (ft MSL) (ft BTOC) (ft MSL) Date 3/16/2020 YGWC-33S 38.73 744.54 14.87* 729.67 YGWC-34I 38.69 773.67 25.29 749.75 10/31/2016 YGWC-34I 38.69 773.67 25.61 748.06 1/9/2017 YGWC-34I 38.69 773.67 24.64 749.03 2/20/2017 38.69 773.67 23.92 749.75 YGWC-34I 4/25/2017 YGWC-34I 38.69 773.67 23.60 750.07 6/26/2017 YGWC-34I 38.69 773.67 24.23 749.44 10/2/2017 YGWC-34I 38.69 773.67 25.08 748.59 1/9/2018 YGWC-36 60.00 10.67 728.86 739.53 10/31/2016 YGWC-36 60.00 739.53 10.30 729.23 1/9/2017 60.00 9.99 YGWC-36 739.53 729.54 2/20/2017 10.39 4/25/2017 YGWC-36 60.00 739.53 729.14 YGWC-36 60.00 739.53 9.86 729.67 6/26/2017 YGWC-36 60.00 739.53 10.23 729.30 10/2/2017 YGWC-36 60.00 739.53 10.20 729.33 1/9/2018 60.00 739.53 10.16 729.37 YGWC-36 3/26/2018 YGWC-36 60.00 739.53 10.18 729.35 6/4/2018 60.00 739.53 729.39 7/7/2018 YGWC-36 YGWC-36 60.00 739.53 729.05 9/14/2018 YGWC-36 60.00 739.53 729.88 2/25/2019 YGWC-36 * 60.00 739.53 729.53 3/25/2019 YGWC-36 60.00 739.53 * 729.53 8/19/2019

- 2. ft MSL = feet mean sea level.
- 3. ft BTOC = feet below top of casing.

^{1. * =} indicates depth to water recorded from transducer.

APPENDIX E NUMERICAL MODELING REPORT



Groundwater Modeling Report

Georgia Power Company

Plant Yates

April 2020



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TRC Environmental Corporation | Georgia Power Company Plant Yates Groundwater Modeling Report

\\GREENVILLE-FP1\WPGVL\PJT2\242211\242211.0002\0001\R2422110002-002 GW MODELING REPORT.DOCX

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Appendix A AQTESOLV Slug Test Data Outputs

Executive Summary

Georgia Power Company (Georgia Power) converted from coal to natural gas at its Plant Yates facility (Site) located near Newnan, Georgia. There are currently several inactive ash-related units at Plant Yates which are being closed in accordance with state and federal regulations, namely the Georgia Environmental Protection Division (GA EPD) Rules of Solid Waste Management 391-3-4-.10 and the United States Environmental Protection Agency (US EPA) Coal Combustion Residuals (CCR) Rule 40 Code of Federal Regulations (CFR) 257 Subpart D. A CCR Unit Solid Waste Handling Permit application for R6 CCR landfill (R6 landfill) was submitted to GA EPD in November 2018 and is currently under review.

A component of the closure approach being developed and implemented for selected impoundments at the Site consists of consolidating ash and placing it within the ash management area (AMA). To better understand the hydrology of the Site, TRC Environmental Corporation (TRC) prepared a groundwater model. The primary objectives of the groundwater model were to:

- Improve the conceptualization of the hydro-stratigraphy, hydrogeology, and groundwater/surface water dynamics at the Site.
- Construct a numerical groundwater flow model that accurately depicts pre-closure groundwater flow conditions.
- Use the calibrated numerical groundwater flow model as a tool to predict groundwater flow conditions following the implementation of closure activities.

Groundwater flow modeling was conducted using Groundwater Vistas. The groundwater model was set up based on the site conceptual hydrogeologic model using six layers to depict the Site lithology. The surficial aquifer includes two modeled layers: a coal residual ash material as Layer 1, where applicable, and the unconsolidated regolith/saprolite as Layer 2. A transition zone consisting of weathered bedrock underlies the surficial aquifer as Layer 3. The competent bedrock beneath the transition zone was segregated as upper bedrock and lower bedrock based on the degree of weathering. The upper bedrock included model Layers 4 and 5. Layer 4 consisted of soft, moderately weathered, fractured biotite gneiss and/or mica schist. Layer 5 was similar to Layer 4 with the exception that the fracture density decreased, and the bedrock was less weathered. The lower bedrock included Layer 6 which represented basement bedrock with little to no weathering or fractures.

The model was calibrated using a combination of trial and error and automated adjustments using Parameter Estimation (PEST) developed by Watermark Numerical Computing (2004), an industry-standard inverse modeling software package. Calibration was accomplished by adjusting various parameters within realistic and expected ranges until simulated groundwater elevations were in reasonable agreement with observed groundwater elevations. The model was calibrated with 49 target groundwater elevations from June 26, 2014 representing pre-closure conditions. The calibration

statistics indicated a residual sum of squares (RSS) value of 1,450 with a root mean square error (RMS error) of 5.44. A small RSS value indicates a tight fit of the model to the Site data. The RSS value observed during the calibration process suggests a reasonable calibration was achieved using PEST. Additionally, the RMS error indicates that groundwater may fluctuate plus/minus 5.44 feet which is within the range of observed seasonal groundwater fluctuations at the Site. Furthermore, the absolute residual mean value (3.94) from the calibration statistics is lower than the 10 percent of the simulated difference in heads, which is generally considered a target value for the acceptance of a model calibration (Anderson and Woessner, 1992).

Following groundwater model calibration, the proposed closure plan was incorporated to build a post-closure groundwater flow model. The post-closure model incorporated the potential future layout of the R6 landfill and the AMA. The post-closure groundwater flow model was used to provide an estimate of changes to Site water levels following completion of closure activities. The overall general groundwater flow direction in the predicted model was to the northwest, which is similar to pre-closure conditions. However, groundwater flow in the R6 landfill area appears to have shifted more to the northeast around the landfill as compared to beneath it, as in previous conditions. The hydraulic gradient in the post-closure model ranged from a minimum of approximately 0.01 ft/ft in the northern portion of the AMA to a maximum of approximately 0.09 ft/ft to the south of AP-2 West. With respect to groundwater elevation, the post-closure model estimated an average decrease of approximately between less than 1 foot to over 4 feet within the R6 landfill and approximately between less than 2 feet to over 8 feet within the AMA as compared to pre-closure conditions.

Section 1 Introduction

Georgia Power (Georgia Power) converted from coal to natural gas at its Plant Yates facility (Site) located near Newnan, Georgia. There are currently several inactive ash-related units at Plant Yates which are being closed in accordance with state and federal regulations, namely the Georgia Environmental Protection Division (GA EPD) Rules of Solid Waste Management 391-3-4-.10 and the United States Environmental Protection Agency (US EPA) Coal Combustion Residuals (CCR) Rule 40 Code of Federal Regulations (CFR) 257 Subpart D. The six original coal ash impoundments subject to the CCR Rule included ash ponds (AP) AP-1, AP-2 (west and east), AP-3, AP-A, AP-B, and AP-B' as shown on **Figure 1-1**. A component of the closure approach being developed and implemented for selected impoundments consists of consolidating ash from AP-2 (including AMAX Cove) and the coves of AP-3, where the ash will be used as subgrade material in the ash management area (AMA) that will encompass AP-A, AP-B', AP-B and the remaining portion of AP-3. AP-2 and the AP-3 coves will be closed by removal of CCR. A CCR Unit Solid Waste Handling Permit application for R6 CCR Landfill (R6 landfill) was submitted to GA EPD in November 2018 and is currently under review.

To better understand the Site hydrology and the potential changes in groundwater flow conditions that may result from CCR unit closure activities, TRC Environmental Corporation (TRC) prepared a groundwater flow model. The groundwater modeling effort consisted of a two-phase approach. The first phase was to create a calibrated groundwater flow model that accurately depicted baseline preclosure groundwater flow conditions. The second phase was to use the calibrated model as a tool to predict post-closure groundwater flow conditions following the implementation of proposed closure activities. This document presents a summary of procedures and results of the groundwater modeling process. Specifically, the report is organized as follows:

- Section 2 summarizes the groundwater model objectives.
- Section 3 presents the development of the conceptual site model.
- Section 4 presents a summary of the calibrated pre-closure groundwater flow model, including a
 description of the program used, domain and computational grid, layers, boundary conditions,
 calibration process and results, and pre-closure groundwater flow conditions.
- Section 5 includes a summary of the post-closure groundwater model which was used to estimate groundwater flow conditions at the completion of closure activities.
- Section 6 includes a summary of this report.
- Section 7 presents the references used for this report.

AP-A

AP-B

R6 CCR LANDFILL

AP-B'

AP-3

AP-3 FINGERS

GEORGIA POWER COMPANY - PLANT YATES NEWNAN, GEORGIA

SHEET TITLE:

1,200 1,600 Feet

400

800

SITE LAYOUT

DRAWN BY:	RAM	SCALE:	PROJ. NO.	242211.0001.0.6
CHECKED BY:	MAH	1: 9,600	FILE NO.	Fig03-01_SiteLayout.mxd
APPROVED BY:	KCS	DATE PRINTED:	FIGURE 1-1	
DATE:	JANUARY 2020		1 '''	OUNE 121



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Section 2 Groundwater Model Objectives

The groundwater flow model was prepared consistent with the Georgia Environmental Protection (EPD) Guidance: Groundwater Contaminant Fate and Transport Modeling (Georgia EPD, 2016). The primary objectives of the groundwater modeling effort were to:

- Improve the conceptualization of the hydro-stratigraphy, hydrogeology, and groundwater/surface water dynamics at the Site.
- Construct a numerical groundwater flow model that accurately depicts pre-closure groundwater flow conditions.
- Use the calibrated numerical groundwater flow model as a tool to predict groundwater flow conditions following the implementation of closure activities.

Section 3 Development of Conceptual Site Model

TRC developed an overall understanding of the hydrogeology, lithology and groundwater quality for the Site using numerous site-specific investigation reports that are referenced within this report and summarized in Section 7.

3.1 Geologic Setting

The geology of the Site is within the Inner Piedmont Physiographic province of western Georgia, immediately southeast of the regional zone of deformation referred to as the Brevard Zone (ACC, 2019). This province is underlain by metamorphic rocks including chlorite schists and quartzofeldspathic gneisses. A thin layer of soil from one to two feet thick overlies a thick layer of saprolite. The saprolite, which extends to typical depths of 20-40 feet below ground surface, was formed in-place by the physical and chemical weathering of the underlying metamorphic rocks. There is typically a zone of variable thickness (approximately 5-20 feet) of transitionally weathered rock between the saprolite and competent bedrock. Localized alluvial soils consisting of generally coarser material (silty-sand, clayey silt, and silty clay with well-rounded gravel and cobbles) than that observed in saprolite may be related to historical river channel migration (ACC, 2019). The groundwater aquifer is unconfined and most of the groundwater flow occurs through the transition zone.

3.2 Hydrogeologic Setting

The Site is in an area of significant groundwater recharge, yet no portion of the Site is within the 100-year floodplain (Southern Company Services (SCS), 2014). The hydraulic gradients observed between piezometer clusters indicate groundwater recharge in upland areas and groundwater discharge near lowland areas with groundwater flow generally from the south, southwestern, and northeastern areas towards the central valley and the Chattahoochee River (SCS, 2014). Based on field values of hydraulic conductivity estimates available at the time of model construction, the hydraulic conductivity values ranged from approximately 0.1 to 5.86 feet/day in the saprolite, from approximately 1.21 to 8.92 feet/day in the transition zone, and from approximately 0.07 to 2.65 feet/day in the upper bedrock. Field estimates of groundwater flow velocities range from approximately 0.05 to 2 feet/day (ACC, 2019). A conceptual site model (CSM) from pre-closure conditions (June 2014) is included as **Figure 3-1**.

3.3 Summary of Supplemental Investigations

3.3.1 Summary of Pre-Closure Groundwater Flow Conditions

A site-wide groundwater gauging event that represented typical pre-closure groundwater conditions occurred in June 2014. The direction of groundwater flow during this event was beneath the R6 landfill area and AMA toward the northwest.

3.3.2 Summary of Supplemental Hydraulic Conductivity Testing within Ash

Aquifer hydraulic conductivities are often determined through the performance of single-well aquifer tests, commonly referred to as slug tests. The tests are conducted by producing a rapid ("instantaneous") change in the water level within the well and recording the recovery of the water level over time. The instantaneous change is produced by submerging a solid plastic rod ("slug") into the groundwater within the well, allowing the water level to return to an equilibrium water level, then rapidly withdrawing the slug from the well. The recovery of the water level is recorded using a down-well pressure transducer connected to a programmable data logger. The data is then downloaded to specialized software and used to calculate hydraulic conductivity.

To refine hydraulic conductivity of the ash, TRC conducted supplemental slug testing on February 1 and 2, 2018 in advance of the initiation of a stability and dewatering investigation conducted by MoreTrench along the AP-3 earthen dam. As part of this investigation, a series of dewatering wells were installed by MoreTrench along the crest of the AP-3 earthen dam. Initially, the plan was to conduct a constant rate pumping test on one of the dewatering wells to ascertain hydrogeological properties of the ash. As such, observation wells were installed to monitor water levels during the pumping test. During the installation of the dewatering wells however, it was discovered that the ash thickness was less than anticipated in the study area and that performance of a successful pumping test was unlikely. As an alternative, slug testing was conducted on the dewatering and observation wells to determine the hydraulic conductivity of the ash.

The slug tests were conducted by displacing a known volume of water within the well and measuring the fluctuation of the groundwater level. This procedure was followed for conducting both a falling-head (slug lowered into a well) and a rising-head (slug removed from the well) slug test. A rising and falling-head slug test was conducted at each well and the data was processed using the aquifer test analysis software AQTESOLV. The hydraulic conductivity estimate from each slug test was evaluated using the Bouwer and Rice (1976) straight-line method for unconfined aquifers. The results of the slug tests are presented in **Table 3-1**.

Observation wells P-1 and P-2 were installed by MoreTrench under the direction of TRC personnel and both wells were screened completely within the ash. The hydraulic conductivity geometric mean for the ash at these two locations was 7.5 x 10⁻⁶ cm/sec or 0.021 feet/day. Slug testing was conducted on additional wells during this study however, review of available field notes for these wells (DW-1, WP-1, and WP-2) indicated that the borings for each well were advanced without sample collection to the base of the ash. As such, the wells were installed partially penetrating or in contact with the underlying higher conductivity natural material (ash/saprolite/transition zone) and therefore were not suitable for hydraulic conductivity testing of the ash given that these wells were screened across multiple units. Since P-1 and P-2 were screened completely within the ash, the hydraulic conductivity value obtained from the slug test results for these wells was incorporated into the groundwater model for the ash hydraulic conductivity. Copies of the AQTESOLV output files are provided in **Appendix A**.

3.3.3 Summary of Lithology Investigations

Bedrock Investigation Along Dyer Road

In June 2017, Georgia Power performed a subsurface boring investigation along Dyer Road, which runs on the northwest side of the R6 landfill as illustrated in **Figure 3-2**. The investigation focused on delineating the ash thickness in this area under the roadway and near the natural gas line located in the right-of-way. TRC requested that four of the borings (YTPR-38, YTPR-46, YTPR-51, and YTPR-55) be advanced 5 feet into the bedrock surface. Drilling was performed by hollow stem auguring through saprolite and NQ™ Diamond Core technology through the transition zone and bedrock. TRC provided oversight of the bedrock borings and it was observed that competent rock ranged from approximately 42 feet to 52 feet below land surface (bls). Upon drilling completion, each borehole was abandoned by grouting from the termination depth to the ground surface and topped off with asphalt patch at the surface. Details from this investigation were incorporated into the groundwater model construction.

Table 3-1
Summary of Supplemental Hydraulic Conductivity Testing within Ash Plant Yates, Georgia

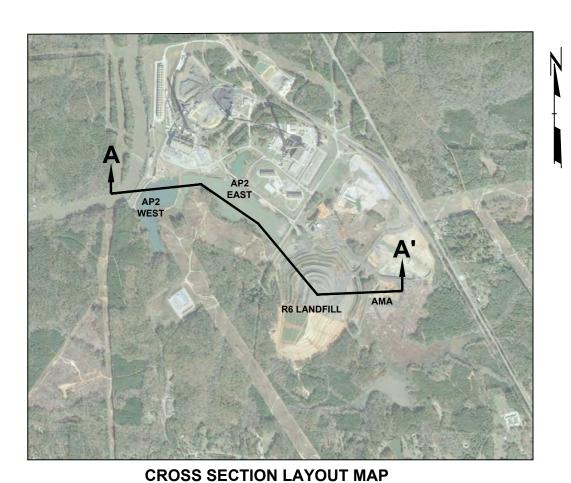
WATER TABLE AQUIFER WELLS							
WELL	GEOLOGIC MATERIAL	ESTIMATED K (ft/day)	PER WELL AVERAGE K (ft/day)	PER UNIT GEOMETRIC MEAN K (ft/day)			
P-1 (FH)	Ash	0.01828	0.01667				
P-1 (RH)	Ash	0.01506	0.01007	0.02124			
P-2 (FH)	Ash	0.04986	0.03235	0.02124			
P-2 (RH)	Ash	0.01484	0.03235				

Hydraulic conductivity testing conducted in February 2018.

(FH) Falling Head Test

(RH) Rising Head Test

^{*} Results for second Falling Head or Rising Head Test



LEGEND

ASH



SAPROLITE

BEDROCK



TRANSITION ZONE



APPROXIMATE WATER SURFACE ELEVATION

NOTE

1. CROSS-SECTION DEPTICS PRE-CLOSURE CONDITIONS. (JUNE 2014)

SOURCE

1. PROFILE IS BASED ON MAY 2014 LIDAR TAKEN FROM METRO ENGINEERING & SURVEYING CO. INC. DRAWING SUPPLIED BY PLANT YATES, 4143YSP.DWG.

GEORGIA POWER COMPANY - PLANT YATES NEWNAN, GEORGIA

CONCEPTUAL SITE MODEL CROSS SECTION A-A'

C. NEWELL PROJ NO.: DRAWN BY: M. HAYS PPROVED BY: K. SAUCIER

242211.0001.0.6

FIGURE 3-1



50 International Drive Patewood Plaza Three, Suite 150 Greenville, SC 29615 Phone: 864.281.0030

242211-CROSS-SECTION.dwg

SCALE: 1=2000'

Soil Boring Locations

Soil Boring Locations that extend to Bedrock

D/(IL.	07.11.07.11.11.2020				
DATE:	JANUARY 2020		FIGURE 3-2		
APPROVED BY:	KCS	DATE PRINTED:	FIGUR	DE 2 2	
CHECKED BY:	MAH	1: 1,200	FILE NO.	FIGURE 3-2	
DRAWN BY:	DJS	SCALE:	PROJ. NO.	242211.0001.0.6	



Patewood Plaza Three, Suite 150 50 International Drive Greenville, SC 29615-3535 Phone: 864-281-0030 FAX: 864-281-0288

Section 4 Pre-Closure Groundwater Flow Modeling

Groundwater modeling efforts were conducted using the numerical modeling software Groundwater Vistas (version 7.24). The following narrative presents an overview of TRC's Groundwater Flow Model (model).

4.1 Groundwater Model Software Selection

Groundwater flow modeling was conducted using Groundwater Vistas. The flexibility of Groundwater Vistas' finite-difference structure makes it easy to conform the model geometry to streams, channels, ponds, and other hydrologic features. The software provides a graphical interface for building, running, and analyzing three-dimensional (3D) groundwater flow models using the MODFLOW suite of codes.

4.2 Model Domain and Computational Grid

The model domain and grid layout are shown on **Figure 4-1**. The model domain has been extended to natural hydraulic boundaries that define the Site watershed. The groundwater and surface water within this watershed drain to the Chattahoochee River located along the western model boundary.

The finite-difference grid consists of 598 rows by 588 columns that was constructed by dividing the model domain into square or rectangular regions called blocks or cells. In plan view, the Site model domain consists of 931,330 active cells. Aquifer hydraulic properties are specified within each cell and layer. The cell size resolution within the model is 20 feet by 20 feet.

4.3 Hydrogeologic Layers and Properties

The groundwater model was set up using six layers to depict the Site lithology. The model layering is illustrated on **Figure 4-2** which represents an east-west cross-sectional view through the central portion of the R6 landfill. The model layers that comprise the surficial aquifer includes two layers: (1) a coal residual ash material (Layer 1 - Ash) where applicable and (2) the unconsolidated regolith/saprolite (Layer 2 - Saprolite). In areas outside of the coal ash footprint, Layer 1 was set to be very thin (0.1-foot thick) and made inactive (no-flow boundary condition). The presence of the thin inactive cells was accommodated in the model by applying the vertical infiltration (recharge) to the underlying Layer 2. This condition in the model was set so that the regolith/saprolite would not be split in half thus creating excessive dry cells within Layer 1 and to allow for better model convergence.

A transition zone (Layer 3 – Transition Zone) consisting of weathered bedrock underlies the surficial aquifer and was modeled as Layer 3. The competent bedrock beneath the transition zone was

segregated into upper bedrock and lower bedrock based on the degree of weathering. The upper bedrock (Layer 4 – Upper Bedrock (some fractures) and Layer 5 – Upper Bedrock (fresh, few fractures)) included Layers 4 and 5. Layer 4 consisted of soft, moderately weathered, fractured biotite gneiss and/or mica schist. Layer 5 was similar to Layer 4 with the exception that the fracture density decreased, and the bedrock was less weathered. The lower bedrock (Layer 6 – Lower Bedrock (fresh, no fractures)) included Layer 6 which represented basement bedrock with little to no weathering or fractures. **Table 4-1** depicts the layer details used in the groundwater model.

The top of the model represents the land surface. The distribution of the land surface elevation was obtained from a May 2014 LIDAR survey which was provided by SCS (SCS, 2014). Information from Site groundwater monitoring well boring logs was used to define the layer elevations where available. The bottom of the saprolite aquifer (Layer 2) was interpolated using mapping software (Surfer®) from boring logs PZ-1S, PZ-2I, PZ-3S through PZ-10S, PZ-13S, PZ-14S, PZ-16S through PZ-20S, PZ-20I, PZ-22S through PZ-28S, PZ-29I, PZ-30I, PZ-31S through PZ-33S, PZ-34 through PZ-49, GYP-1, GYP-4 through GYP-28, and YTPR-36 through YTPR-55.

Ash was included in the groundwater model (Layer 1) at the AP-2 and AP-3 impoundments, the R6 landfill, and the R6 channel. The ash was not modeled as a continuous layer and pinched out outside of these areas. When present, the ash layer exists on top of the saprolite and represents the top of the model. The ash thickness in AP-2 varied between 3.43 to 19.65 feet and was incorporated into the model based on previous investigative activities conducted by TRC. Additionally, the ash thickness of the R6 landfill, R6 channel, and AP-3 was incorporated into the model based on pre-existing ground surface elevations for these areas. This included the November 2000 United States Geological Survey (USGS) digital elevation model (DEM) for the topography of the R6 landfill and R6 channel area, and the 1976 topographic map (digitized in AutoCAD) for the AP-3 pond/coves. Ash thickness in the R6 landfill/ R6 channel ranged between 0.18 to 160.68 feet and in AP-3 between 4.72 to 31.45 feet.

The bottom elevation of the transition zone (Layer 3) was interpolated from boring logs PZ-1I through PZ-10I, PZ-13I, PZ-14I, PZ-16I, PZ-18I, PZ-24I through PZ-30I, PZ-32I, PZ-34I, PZ-35 through PZ-49, GYP-1, GYP-4 through GYP-6, GYP-8, GYP-10, GYP-13, GYP-16, GYP-19, GYP-20, GYP-23, GYP-26, GYP-28, YTPR-38, YTPR-46, YTPR-51, and YTPR-55. Boring logs PZ-1D, PZ-3D, PZ-5D, PZ-6D, PZ-38, PZ-40, PZ-43, and PZ-48 were used to help define the bedrock layers. However, sufficient spatial information on bedrock layers was not available for interpolation, and as such a uniform thickness of 5 feet was assumed for shallow/fractured bedrock (Layer 4) and 15 feet for Layer 5. The thickness of the lower bedrock (Layer 6) varied and the model base was set at an elevation of 500 feet above mean sea level.

Hydraulic conductivity properties were assigned within the model using specific zones. In general, a zone correlated to the model layer and represented the average hydraulic conductivity value for the associated lithology. The hydraulic conductivity values used for each zone were assigned based on a

combination of available slug testing data and through the process of model calibration. Model calibration is described separately in Section 4.5. **Table 4-2** provides a summary of available hydraulic conductivity estimates from Site slug testing results. The hydraulic conductivity values assigned within the model are presented on **Table 4-3**. Additionally, the hydraulic conductivity zones modeled within the ash layer (Layer 1) and saprolite (Layer 2) are depicted on **Figure 4-3**. For visual purposes, the ash from Layer 1 is superimposed onto Layer 2 in **Figure 4-3**.

The horizontal (Kh) hydraulic conductivities shown in **Table 4-3** are generally similar to the average hydraulic conductivity values observed from available slug testing data (**Table 4-2**) with respect to the saprolite. For example, the model Kh for the saprolite was 0.031 feet/day in the uplands (zone 8) and 1.61 feet/day in the valley (zone 1) while the average Kh from slug testing data for 21 wells installed in the saprolite was 1.44 feet/day. The model Kh for the transition zone, however, was 0.01 feet/day which is lower than the average Kh from slug testing for 11 wells installed in the transition zone was 4.72 feet/day. In regard to the upper fractured bedrock layer (Layer 4), the model Kh results were also lower (0.021 feet/day) than the 1.69 feet/day average from slug testing at seven wells. The upper slightly fractured bedrock layer (Layer 5) Kh from the model was 0.01 feet/day and the average from slug testing at 13 wells was 0.28 feet/day. The Kh for the lower bedrock (Layer 6) and the vertical (Kv) hydraulic conductivities were estimated during model calibration.

4.4 Boundary Conditions

Boundary conditions were specified for the model perimeter, model top, and model bottom. These boundary conditions include constant head, lake, drainage channels, recharge, evapotranspiration, and inactive cells (no flow).

4.4.1 Model Perimeter

The watershed drainage basin surrounding the Site was used to establish the model perimeter. Inactive cells were used to represent the edge or topographic ridge surrounding the watershed encompassing the Site. One exception to this was along the western extent of the model perimeter at the Chattahoochee River where a constant head elevation ranging from approximately 688 feet above mean sea level (AMSL) at the upstream portion of the river to 685 feet AMSL at the downstream extent. This elevation was approximated from the May 2014 LIDAR survey data (SCS, 2014).

For the bottom of the model, vertical flow between the upper bedrock (Layer 5) and underlying deep/basement bedrock (Layer 6) was assumed to be very small compared with flow in the overlying fractured bedrock and thus served as the model base consisting of inactive cells. With respect to the model top, the topography of the Site dictates as previously discussed. Recharge and evapotranspiration fluxes were applied to the highest active layer as described below.

4.4.2 Drains

Information was not readily available regarding the geometry, bed conductance, or flow characteristics of the Site tributaries. As such, these features were treated as drains in the groundwater model. The drain feature is a type of head-dependent boundary condition in Groundwater Vistas that will only allow water to be removed from the system. For instance, if the head computed by the model is less than the head in the boundary (drain), the boundary condition is turned off.

The drain boundary condition in the groundwater model was set close to or equal to the ground surface elevation for each cell within a tributary footprint. Using this approach, any water above the ground surface within the model would be removed from the system reducing the likelihood of artificially flooded areas. In general, the drain boundary condition served to keep the water table at or close to the tributary bottom. As with the Site tributaries, the R6 channel was similarly modeled as a drain.

4.4.3 Ash Ponds

The two ash ponds at the Site included in the groundwater model were AP-2 (AP-2 East and AP-2 West) and the AP-3 pond/coves. AP-2 was modeled using the lake boundary condition. This represents a head-dependent boundary condition in which a boundary head and conductance term are assigned to allow for the flux of water into or out of the cell. Since the entire base of AP-2 is covered in ash, this boundary condition was most suitable to allow for water to migrate into or out of the ash layer.

AP-3 was modeled using the constant head boundary condition. This boundary condition differs than that used for AP-2 in that the bottom of AP-3 does not contain a continuous layer of ash and therefore the constant head boundary condition was suitable. Constant head boundary conditions are assigned a head that does not vary throughout the model simulation.

For AP-2, the boundary head was set at 722.88 feet above mean sea level (AMSL). The AP-3 elevation was set at 754 feet AMSL. These elevations were selected based on staff gauge measurements collected by TRC during ash thickness investigation activities conducted in 2016.

4.4.4 Additional Surface Water Features

In review of historical aerial imagery of the Site, several small ponds appear to be historically present and were included in the model using the constant head boundary condition. The water surface elevation of each pond was estimated using the topography of the pond shoreline/edge

from the May 2014 LIDAR survey data (SCS, 2014). The following constant head elevations were used in the model:

- Pond 1 804 feet AMSL
- Pond 2 777 feet AMSL
- Pond 3 778 feet AMSL
- Pond 4 782 feet AMSL
- Pond 5 779 feet AMSL
- Pond 6 725 feet AMSL

4.4.5 Recharge and Evapotranspiration

Recharge is the rate at which infiltrating water reaches the water table while evapotranspiration (ET) is the sum of evaporation from the land surface plus transpiration from plants. Both recharge and ET were expressed in units of feet per day in the model. Recharge and ET are represented in the groundwater model as specified flux boundary cells; however, they are treated as properties because they are distributed to every cell in the top (or highest active) layer of the model.

As reported in the 2014 Hydrogeological Characterization Report (SCS, 2014), the mean annual precipitation observed in the Newnan, Georgia area based on 85 years of data is 51.4 inches per year. In general, approximately 70 percent of precipitation in Georgia is lost to evapotranspiration (Rasmussen, 2016). An initial conservative estimate of 15.4 inches per year (0.0035 feet/day) for recharge was used based on ET loss alone (precipitation minus 70 percent loss to ET). This initial value did not however, account for surface runoff. In using this initial value, the groundwater model was inundated with excessive surface water thus flooding most of the model area. Information regarding the Site surface water features/tributaries was not readily available to provide an estimate of surface runoff and groundwater recharge. Recharge in the model was assigned using zones that were based on simulations conducted during model calibration. A recharge rate of 3.5 inches per year (0.0008 feet/day) was used for the lowland/valley portions of the site while the upland areas were assigned a recharge rate of 2.2 inches per year (0.0005 feet/day). Model calibration is discussed in Section 4.5.

A recharge rate of 1 inch per year (0.00023 feet/day) was used within the R6 landfill footprint to account for the clay liner that serves as the cap for the landfill. The recharge zones modeled within Layer 1 is depicted on **Figure 4-4**.

Evapotranspiration from the water table can be significant in this region of Georgia, as in general, there is a relatively shallow water table at many locations, including Plant Yates. The

computed ET decreases linearly with depth of the water table below the land surface to a value of zero at a specified root extinction depth. The extinction depth is a function of vegetative cover, soil type, and land use. Similar to recharge, ET and extinction depth in the model was assigned using zones that were based on simulations conducted during model calibration. An ET rate of 175 inches per year (0.04 feet/day) with an extinction depth of 1 foot was used for the lowland/valley portions of the site while the upland areas were assigned an ET rate of 191 inches per year (0.0437 feet/day) with an extinction depth of 10 feet. These ET rates are higher than what you would anticipate from actual site conditions however, ET does not remove water from the model if groundwater is below the extinction depth. Additionally, a shallow extinction depth as used in the lowland areas of the model (1 foot) may represent removal of water from the ground surface via overland flow. Review of the volumetric budget of the model indicated that the amount of recharge was similar to the amount of ET. The evapotranspiration zones modeled within Layer 1 are depicted on Figure 4-5.

4.5 Groundwater Flow Model Calibration

The groundwater flow model was calibrated using a comparison of the observed heads in monitoring wells to model computed heads for steady-state simulations representing average hydrologic conditions. The observed heads from Site monitoring wells were selected from the June 26, 2014 groundwater measurement event which represents data that was closest to the time interval of the May 2014 LIDAR topographic survey data. This provided the best spatial coverage closest to the time of the topographic survey which was used as the ground surface for the model. The observed heads used in the calibration process are depicted on **Table 4-4**.

The groundwater model was calibrated using a combination of trial and error and automated adjustments using Parameter Estimation (PEST), an industry-standard inverse modeling software package developed by Watermark Numerical Computing (2004). PEST uses calibration targets to compute statistics that guide the selection of aquifer property values. The calibration targets used in the model calibration were observed groundwater elevations as presented on **Table 4-4**. An upper and lower boundary was assigned in PEST to refine the model hydrogeological parameters. The objective of PEST was to find the best set of parameter estimates that minimized the sum of weighted squared deviations between certain model outcomes and the corresponding field-measured counterparts.

The PEST parameters estimated during model calibration were recharge, ET, Kh, and Kv. A total of 49 groundwater head measurements (targets) were used during the calibration process. The results of the observed versus simulated target values following use of the PEST calibration tool is presented on **Figure 4-6**. For a perfect fit of modeled results to observed values, all points would plot on a line extending at a 45-degree angle through the origin of the modeled versus observed value chart. As

depicted in **Figure 4-6**, the majority of the points fall on or close to the line indicating a good fit between the observed and modeled results.

A statistical summary of the minimum residual, maximum residual, and the absolute residual mean are presented in **Table 4-5**. The residual is the difference between observed groundwater elevations and simulated groundwater elevations. If the residual is positive, the observed groundwater elevation is higher than the simulated groundwater elevation. If the residual is negative, the observed groundwater elevation is lower than the simulated groundwater elevation. The standard deviation of the residuals and the range of observed groundwater elevations are also provided in **Table 4-5**. A common statistical test to examine calibration is the standard deviation of the residuals (the difference between observed and simulated values) divided by the range of observed values. Rumbaugh (2004) suggests that a good calibration yields a value less than 10 to 15 percent or (0.10 to 0.15). The standard deviation of the residuals divided by the range of observed groundwater elevations for the calibrated model is 0.039.

The summary also includes the value of the sum of squared residuals, which was used as the objective function during parameter estimation. Finally, the summary includes the frequency of residuals within 1-foot, 2.5-foot, and 5-foot intervals. A graphical summary showing the match between the observed and simulated groundwater elevations is shown on **Figure 4-6**. Sixteen percent of the simulated groundwater elevations are within \pm 1 foot of the observed groundwater elevations, 49 percent are within \pm 2.5 feet, while 71 percent are within \pm 5 feet. A model output of the pre-closure calibrated head contours is presented as **Figure 4-7**.

A mass balance conducted on the calibrated model concluded that the model reflects a good conservation of mass (inputs and outputs) with a total calculated error of only 0.002 percent.

4.6 Groundwater Flow Model Sensitivity Analysis

A sensitivity analysis evaluated how changes in selected model parameters affect the model. One parameter at a time was varied while all the other parameters were held constant. The tested model parameters were varied by 50 percent of the calibrated values. The sensitivity analysis was conducted on horizontal and vertical conductivity, recharge, and evapotranspiration. A sensitivity analysis was not conducted on the constant head boundary conditions as these values were designated by SCS as operational requirements for the facility. Results of the sensitivity analysis are presented in **Table 4-6**. These results were analyzed to identify the parameters causing the greatest sensitivity when changed, and to confirm that final calibrated values fall within the range of expected values according to the ranges set during conceptual model formulation.

Model results are most sensitive to changes in recharge rate and horizontal conductivity in the saprolite zone. Calibrated values for these parameters are consistent with observed values in the study area, as identified during the conceptual model formulation stage. No significant improvement in the model

calibration was noted to by adjusting these parameters. Thus, changes to the model are unwarranted, and the model was accepted for post-closure use, according to the objectives set forth at the beginning of the modeling process and the model-calibration criteria established during numerical model development.

4.7 Groundwater Flow Model Verification

The groundwater flow model was evaluated to determine the suitability for post-closure simulations. This was accomplished by testing the calibrated model to determine if the simulated groundwater elevations would match those from a separate historic data set of observed groundwater elevations, a process commonly referred to as model verification. The data set used for the verification process was the February 20, 2017 observed groundwater elevations. The observed groundwater elevations from February 20, 2017 are listed on **Table 4-7**, along with the simulated groundwater elevations from the model. **Table 4-7** also lists the residuals which is the difference between observed groundwater elevations and simulated groundwater elevations. As previously discussed, if the residual is positive, the observed groundwater elevation is higher than the simulated groundwater elevation. If the residual is negative, the observed groundwater elevation is lower than the simulated groundwater elevation. The results of the observed versus simulated target values for the February 20, 2017 event are also graphically depicted on **Figure 4-8**.

A statistical summary of the fit of the February 20, 2017 data set using the calibrated model is presented on **Table 4-8**. The standard deviation of the residuals divided by the range of observed groundwater elevations using the February 20, 2017 verification data set is 0.036. As previously discussed, a good fit yields a value less than 0.10 to 0.15 (Rumbaugh, 2004) suggesting the independent data set is a good match to the calibrated model. Additionally, the statistical summary includes the frequency of residuals within 1-foot, 2.5-foot, and 5-foot intervals. Twelve percent of the simulated groundwater elevations are within \pm 1-foot of the observed groundwater elevations, 27 percent are within \pm 2.5 feet, while 58 percent are within \pm 5 feet.

Of the approximately 42 percent (25 water level measurements) of the simulated groundwater elevations that were outside of the ± 5-foot range of observed groundwater elevations, 40 percent or 10 of the 15 measurements were in the northwestern area of the site near the Chattahoochee River. The USGS maintains a stream gauge approximately a half mile upstream of the site. In reviewing the historical gage height data, the river was approximately 5.5 feet lower during the February 20, 2017 event compared to that used for the calibrated model. Additionally, 28 percent or seven of the 25 simulated groundwater elevations that were outside of the ± 5-foot range of observed groundwater elevations were located within bedrock which has the least amount of information available for the site.

Although there is slightly more variation in the simulated groundwater elevations using the February 20, 2017 data set, the overall fit of the data seemed to match the observed values and thus the calibrated model was accepted for post-closure use.

Table 4-1 Groundwater Model Layer Description

LAYER NUMBER	DESCRIPTION	LAYER THICKNESS (ft)	TOP LAYER ELEVATION (ft msl)
1	CCR Ash ⁽¹⁾	Between 0.18 to 160.68	700 to 878
2	Saprolite/Regolith/Alluvium	Between 5 to 88	685 to 885
3	Transition Zone	Between 8 to 45	653 to 855
4	Upper Bedrock (some fractures)	Assumed 5	633 to 808
5	Upper Bedrock (fresh, few fractures)	Assumed 15	628 to 803
6	Lower Bedrock (fresh, no fractures)	Varies, greater than 20	613 to 788

⁽¹⁾ The CCR Ash does not cover the entire layer and Layer 1 is inactive outside of the ash footprint.

Table 4-2
Summary of Available Hydraulic Conductivity Testing
Plant Yates, Georgia

					PER UNIT
WELL	DESCRIPTION/LAYER	LAYER	ESTIMATED K	AVERAGE K	GEOMETRIC
			(ft/day)	(ft/day)	MEAN K (ft/day)
PZ-4S	Saprolite	2	1.3		
PZ-5S	Saprolite	2	1.2		
PZ-7S	Saprolite	2	0.1		
PZ-8S	Saprolite	2	0.7		
PZ-9S	Saprolite	2	0.6		
PZ-10S	Saprolite	2	0.4		
PZ-14S	Saprolite	2	1.4		
PZ-17S	Saprolite	2	0.98		
PZ-18S	Saprolite	2	0.25		
PZ-19S	Saprolite	2	0.92		
PZ-20S	Saprolite	2	0.83	1.44	0.87
PZ-22S	Saprolite	2	0.86		
PZ-23S	Saprolite	2	0.64		
PZ-24S	Saprolite	2	0.81		
PZ-25S	Saprolite	2	0.47		
PZ-26S	Saprolite	2	0.22		
PZ-28S	Saprolite	2	0.90		
PZ-31S	Saprolite	2	5.86		
PZ-32S	Saprolite	2	5.45		
PZ-33S	Saprolite	2	5.62		
PZ-36	Saprolite	2	0.69		
PZ-18I	Transition Zone	3	1.82		
PZ-26I	Transition Zone	3	1.21		
PZ-30I	Transition Zone	3	6.19		
PZ-32I	Transition Zone	3	6.06		
PZ-35	Transition Zone	3	7.35		
PZ-37	Transition Zone	3	5.36	4.72	3.98
PZ-39	Transition Zone	3	5.25		
PZ-41	Transition Zone	3	5.35		
PZ-43	Transition Zone	3	8.92		
PZ-47	Transition Zone	3	2.28		
PZ-49	Transition Zone	3	2.12		
PZ-5I	Fractured Bedrock	4	1.10		
PZ-3D	Fractured Bedrock	4	1.60		
PZ-40	Fractured Bedrock	4	1.85		
PZ-41	Fractured Bedrock	4	2.65	1.69	1.63
PZ-45	Fractured Bedrock	4	1.15		
PZ-46	Fractured Bedrock	4	1.78		
PZ-48	Fractured Bedrock	4	1.73		
PZ-1D	Slightly Fractured Bedrock	5	0.20		
PZ-31	Slightly Fractured Bedrock	5	0.30		
PZ-41	Slightly Fractured Bedrock	5	0.20		
PZ-5D	Slightly Fractured Bedrock	5	0.30		
PZ-6D	Slightly Fractured Bedrock Slightly Fractured Bedrock	5	0.30		
PZ-21I	- · ·	5	0.07	0.20	0.22
PZ-25 I	Slightly Fractured Bedrock	5 5	0.09	0.28	0.23
PZ-27I PZ-28I	Slightly Fractured Bedrock Slightly Fractured Bedrock	5	0.51		
		5	0.10		
PZ-29I	Slightly Fractured Bedrock Slightly Fractured Bedrock	5	0.13		
PZ-38 PZ-42	Slightly Fractured Bedrock	5	0.54 0.53		
PZ-42 PZ-44		5			
rz-44	Slightly Fractured Bedrock	Э	0.31		

Data from 2014 Draft Hydrogeological Characterization Report and March 2017 Slug Test Summary

Table 4-3
Groundwater Model Hydraulic Conductivity Parameters

LAYER NUMBER	LAYER/ MATERIAL DESCRIPTION	AQUIFER TYPE	MODEL ZONE NUMBER	Kh (feet/day)	Kv (feet/day)
1	CCR Ash	Unconfined	6	0.0028	0.0011
	Saprolite/Regolith - Valley	Unconfined	1	1.61	0.2
2	Saprolite/Regolith - Uplands	Unconfined	8	0.031	0.002
	Alluvial/Fluvial Deposits ⁽¹⁾	Unconfined	7	10	4
3	Transition Zone	Unconfined	2	0.01	0.001
4	Upper Bedrock (some fractures)	Unconfined	3	0.021	0.0025
5	Upper Bedrock (fresh, few fractures)	Unconfined	5	0.01	0.005
6	Lower Bedrock (fresh, no fractures)	Unconfined	4	0.013	0.0013

⁽¹⁾ A narrow band of alluvial/fluvial deposits adjacent to the Chattahoochee River are included in the model within the saprolite layer.

Table 4-4 Groundwater Model Calibration Targets June 26, 2014

PIEZOMETER / WELL ID	GROUNDWATER ELEVATION ⁽¹⁾	MODEL LAYER	PIEZOMETER / WELL ID	GROUNDWATER ELEVATION ⁽¹⁾	MODEL LAYER
PZ-1S	805.88	2	PZ-14S/YGWA-14S	733.37	2
PZ-1I/YGWA-1I	800.95	3	PZ-14I	732.46	3
PZ-1D/YGWA-1D	788.77	4	PZ-16S	766.66	2
PZ-2I/YGWA-2I	822.65	3	PZ-16I	766.31	3
PZ-3S	762.40	2	GYP-1	775.87	3
PZ-3I/YGWA-3I	747.03	3	GYP-4	765.27	3
PZ-3D/YGWA-3D	765.96	4	GYP-5	823.69	3
PZ-4S	762.25	2	GYP-6	756.68	3
PZ-4I/YGWA-4I	763.20	3	GYP-8	787.51	3
PZ-5S	767.73	2	GYP-9	732.78	2
PZ-5I/YGWA-5I	767.66	3	GYP-11	731.66	2
PZ-5D/YGWA-5D	774.51	4	GYP-13	749.86	3
PZ-6S/YGWA-6S	764.38	2	GYP-14	770.40	2
PZ-6I/YGWA-6I	764.46	3	GYP-15	754.04	2
PZ-6D	760.81	4	GYP-16	787.62	3
PZ-7S	723.32	2	GYP-17	757.09	2
PZ-7I	723.56	3	GYP-18	758.95	3
PZ-8S	733.93	2	GYP-19	759.80	3
PZ-8I	734.68	3	GYP-21	769.00	2
PZ-9S	694.76	2	GYP-22	758.69	2
PZ-9I	694.58	3	GYP-23	757.82	3
PZ-10S	693.39	2	GYP-24	770.40	2
PZ-10I	687.06	3	GYP-25	766.24	2
PZ-13S	772.05	2	GYP-28	755.93	3
PZ-13I	769.32	3			

⁽¹⁾ Groundwater elevation measured in feet above mean sea level.

Groundwater elevations for PZ 1 through PZ-16 from Yate-wlupdate-2017 file provided by Southern Company Services, Inc.

Groundwater elevations for GYP-1 through GYP-28 from GYP water levels file provided by Southern Company Services, Inc.

Table 4-5
Statistical Summary of Simulated Groundwater Elevations in Calibrated Model

CALIBRATION STATISTIC	CALIBRATED MODEL VALUE
Minimum Residual (feet)	-19.50
Maximum Residual (feet)	6.64
Absolute Residual Mean (feet)	3.94
Average Residual (feet)	-1.26
Standard Deviation of Residuals	5.29
Range of Observed Groundwater Elevations (feet)	136.63
Standard Deviation/Range	0.039
Absolute Residual Mean/Range*100	2.88
Sum of Squared Residuals	1450
Percent of residuals within:	
±1ft	16
± 2.5 ft	49
± 5 ft	71

Table 4-6 Summary of Groundwater Model Sensitivity Analysis

MODEL PARAMETER	MULTIPLIER	NORMALIZED RSS ⁽¹⁾
	0.5	1.05
Recharge Rate (Lowlands – Zone 1)	1.5	1.15
B 4 (B04 15115 4 : 1)	0.5	1.00
Recharge Rate (R6 landfill Footprint)	1.5	1.00
Decharge Date (Unlands 7575 2)	0.5	1.21
Recharge Rate (Uplands – Zone 3)	1.5	1.26
Evapotranspiration Rate (Lowlands – Zone 1)	0.5	1.00
	1.5	1.00
Evapotranspiration Rate (Uplands – Zone 3)	0.5	1.01
Evapotralispiration Rate (Opiands – Zone 3)	1.5	1.00
Evapotranspiration Extinction Depth (Lowlands – Zone 1.5	0.5	1.00
	1.00	
Evapotranspiration Extinction Depth (Uplands – Zone 3)	0.5	1.45
Evapotrarispiration Extinction Deptir (Opiands – 2011e 3)	1.5	0.95
Horizontal Conductivity of Ash (Layer 1)	0.5	1.00
Tionzontal Conductivity of Asir (Layer 1)	1.5	1.00
Horizontal Conductivity of Saprolite/Regolith in Lowlands	0.5	1.24
(Layer 2 – Zone 1)	1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5	0.99
Horizontal Conductivity of Saprolite/Regolith in Uplands	0.5	1.23
(Layer 2 – Zone 8)	0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5	0.92
Horizontal Conductivity of Alluvial/Fluvial Deposits	0.5	1.00
(Layer 2 – Zone 7)	1.5	1.00
Horizontal Conductivity of Transition Zone (Layer 3)	0.5	1.00
Tionzontal conductivity of Transition Zone (Layer 3)	1.5	0.98
Horizontal Conductivity of Upper Bedrock (Layer 4)	0.5	1.03
Fibrizontal Conductivity of Opper Dedrock (Layer 4)	1.5	0.98
Horizontal Conductivity of Upper Bedrock (Layer 5)	0.5	1.04
Tionzontal Conductivity of Opper Dedition (Layer 3)	1.5	0.97
Horizontal Conductivity of Lower Bedrock (Layer 6)	0.5	1.01
Tionzontal Conductivity of Lower Bedfock (Layer 6)	1.5	1.00
Vertical Conductivity of Ash (Layer 1)	0.5	1.00
vertical Conductivity of Asir (Layer 1)	1.5	1.00

Table 4-6 Summary of Groundwater Model Sensitivity Analysis

MODEL PARAMETER	MULTIPLIER	NORMALIZED RSS ⁽¹⁾
Vertical Conductivity of Saprolite/Regolith in Lowlands	0.5	1.02
(Layer 2 – Zone 1)	1.5	0.99
Vertical Conductivity of Saprolite/Regolith in Uplands	0.5	0.98
(Layer 2 – Zone 8)	1.5 0.5 1.5 0.5 1.5 0.5 1.5 0.5 1.5	1.01
Vertical Conductivity of Alluvial/Fluvial Deposits	0.5	1.00
(Layer 2 – Zone 7)	-	1.00
V :: 10 1 :: 17 :: 7 / 10	0.5	1.00
Vertical Conductivity of Transition Zone (Layer 3)		1.00
Vertical Conductivity of Upper Pedrack (Layer 4)	0.5	1.00
Vertical Conductivity of Upper Bedrock (Layer 4)	1.5	1.00
Vertical Conductivity of Llaner Bodrock (Layer F)	0.5	1.00
Vertical Conductivity of Upper Bedrock (Layer 5)	1.5	1.00
Vertical Conductivity of Layer Bodrock (Layer 6)	0.5	1.03
Vertical Conductivity of Lower Bedrock (Layer 6)	1.5	0.99

⁽¹⁾ Normalized RSS (residual sum of squares) is the new RSS from the sensitivity analysis divided by calibrated model RSS.

Table 4-7 Groundwater Model Verification – February 20, 2017 Observed Water Levels

WELL ID	OBSERVED	SIMULATED	RESIDUAL	MODEL LAYER	WELL ID	OBSERVED	SIMULATED	RESIDUAL	MODEL LAYER
PZ-1S	801.24	803.84	-2.60	2	PZ-23S	749.71	745.61	4.10	2
PZ-1I	796.93	799.09	-2.16	3	PZ-24S	735.53	741.17	-5.64	2
PZ-2I	819.21	821.52	-2.31	3	PZ-24I	735.01	740.53	-5.52	3
PZ-3S	758.89	765.84	-6.95	2	PZ-25S	730.40	735.52	-5.12	2
PZ-4S	758.44	757.51	0.93	2	PZ-25I	729.02	735.62	-6.60	3
PZ-4I	759.46	758.87	0.59	3	PZ-26S	695.70	700.35	-4.65	2
PZ-5S	764.44	762.61	1.83	2	PZ-26I	691.49	699.68	-8.19	3
PZ-5I	764.35	764.15	0.20	3	PZ-27S	689.24	696.53	-7.29	2
PZ-5D	770.39	765.57	4.82	4	PZ-27I	688.15	696.88	-8.73	3
PZ-6S	760.30	765.54	-5.24	2	PZ-28S	695.37	697.01	-1.64	2
PZ-6I	760.27	764.97	-4.70	3	PZ-28I	693.78	698.08	-4.30	3
PZ-6D	757.21	764.53	-7.32	4	PZ-29I	690.86	695.11	-4.25	3
PZ-7S	720.86	733.97	-13.11	2	PZ-30I	724.29	727.55	-3.26	3
PZ-7I	720.78	731.91	-11.13	3	PZ-31S	724.29	722.62	1.67	2
PZ-8S	733.21	732.28	0.93	2	PZ-35	731.04	734.43	-3.39	3
PZ-8I	733.37	738.56	-5.19	3	PZ-36	729.54	732.39	-2.85	2
PZ-9S	693.57	702.62	-9.05	2	PZ-37	748.80	746.49	2.31	3
PZ-9I	693.46	704.65	-11.19	3	PZ-38	768.15	763.01	5.14	3
PZ-10S	692.98	696.06	3.08	2	PZ-39	791.61	790.81	0.80	4
PZ-10I	686.57	696.07	-9.50	3	PZ-40	787.10	792.32	-5.22	3
PZ-14S	730.30	735.08	-4.78	2	PZ-41	774.68	772.13	2.55	4
PZ-14I	730.29	733.08	-2.79	3	PZ-42	769.10	765.41	3.69	4
PZ-16S	765.58	765.99	-0.41	2	PZ-43	730.63	734.30	-3.67	4
PZ-17S	770.33	769.01	1.32	2	PZ-44	708.89	719.45	-10.56	4
PZ-18S	768.39	767.28	1.11	2	PZ-45	696.77	709.75	-12.98	3
PZ-18I	765.76	767.39	-1.63	3	PZ-46	700.36	715.20	-14.84	3
PZ-19S	748.46	754.04	-5.58	2	PZ-47	726.93	737.93	-11.00	4
PZ-20S	757.56	755.87	1.69	2	PZ-48	755.06	762.34	-7.28	4
PZ-21I	752.68	756.30	-3.62	3	PZ-49	749.11	754.68	-5.57	3
PZ-22S	742.19	742.02	0.17	2					

⁽¹⁾ Groundwater elevations listed in feet above mean sea level.

Table 4-8 Statistical Summary of Model Verification Evaluation Using February 20, 2017 Observed Groundwater Elevations

CALIBRATION STATISTIC	CALIBRATED MODEL VALUE
Minimum Residual (feet)	-14.84
Maximum Residual (feet)	5.14
Absolute Residual Mean (feet)	4.89
Average Residual (feet)	-3.75
Standard Deviation of Residuals	4.73
Range of Observed Groundwater Elevations (feet)	132.64
Standard Deviation/Range	0.036
Absolute Residual Mean/Range*100	3.69
Sum of Squared Residuals	2150
Percent of residuals within:	
±1ft	12
± 2.5 ft	27
± 5 ft	76

LEGEND

EXISTING SHALLOW (SAPROLITE) PIEZOMETER



EXISTING INTERMEDIATE (TRANSITION ZONE) PIEZOMETER



EXISTING DEEP (BEDROCK) PIEZOMETER

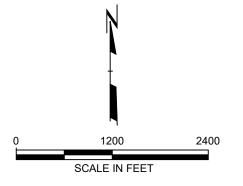
GROUNDWATER MODEL DOMAIN



GRID LAYOUT (EACH CELL IS 20'X20')

SOURCE

NOVEMBER 2015 AERIAL WAS OBTAINED FROM GOOGLE EARTH AT https://www.google.com/



GEORGIA POWER COMPANY - PLANT YATES NEWNAN, GEORGIA

GROUNDWATER MODEL DOMAIN AND GRID LAYOUT

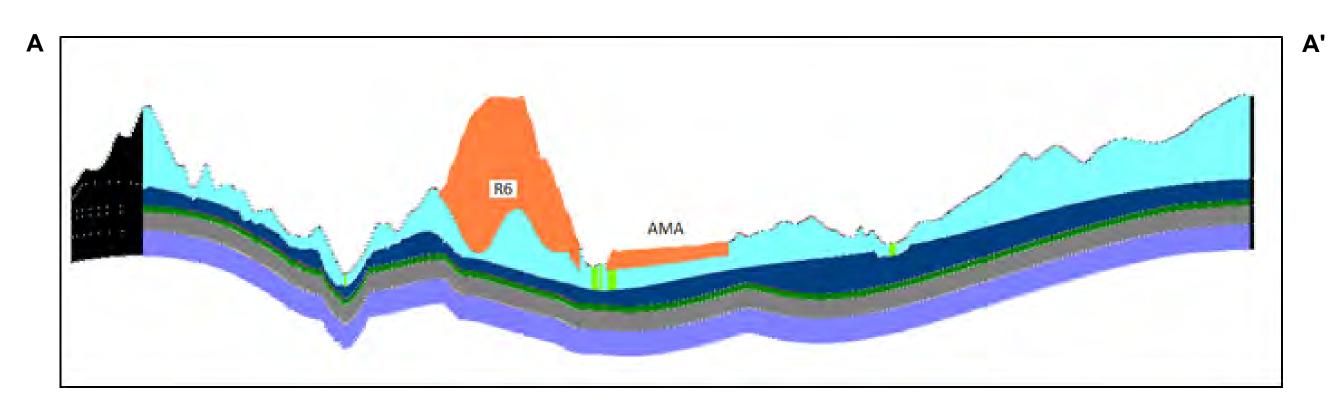
DRAWN BY:	A. PEEBLES	PROJ NO.:	242211.0
CHECKED BY:	M. HAYS		
APPROVED BY:	K. SAUCIER		FIGURE 4-1
DATE:	VDDII 3030		



242211.0001.0.6

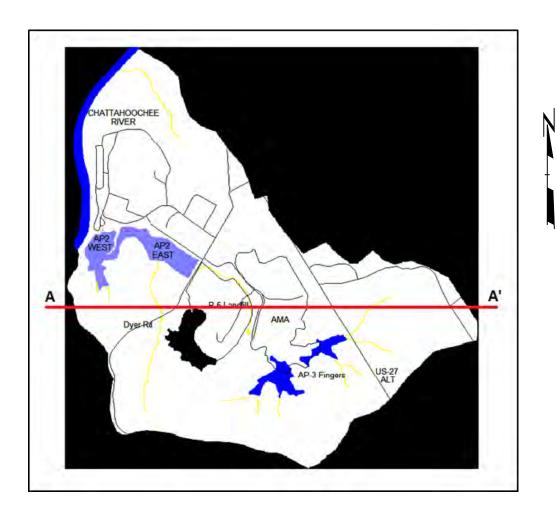
242211-DATA_GAP-GRIDBOUND.dwg





CROSS SECTION A-A'

NOT TO SCALE



LEGEND Ash Saprolite Transition Zone Upper Bedrock (some fractures) Upper Bedrock (fresh, few fractures) Lower Bedrock (fresh, no fractures)

SOURCE

1. PROFILE IS BASED ON MAY 2014 LIDAR TAKEN FROM METRO ENGINEERING & SURVEYING CO. INC. DRAWING SUPPLIED BY PLANT YATES, 4143YSP.DWG.

GEORGIA POWER COMPANY - PLANT YATES NEWNAN, GEORGIA

GROUNDWATER MODEL LAYER CROSS SECTION A-A'

DRAWN BY: R. MAYER PROJ NO.: 242211.0001.0.6 M. HAYS CHECKED BY: FIGURE 4-2 APPROVED BY: K. SAUCIER



50 International Drive Patewood Plaza Three, Suite 150 Greenville, SC 29615 Phone: 864.281.0030

242211-CROSS-SECTION.pdf

JANUARY 2020

CROSS SECTION LAYOUT MAP

NOT TO SCALE

Figure 4-3 Horizontal Hydraulic Conductivity Zones – Ash/Saprolite (Layers 1 and 2)

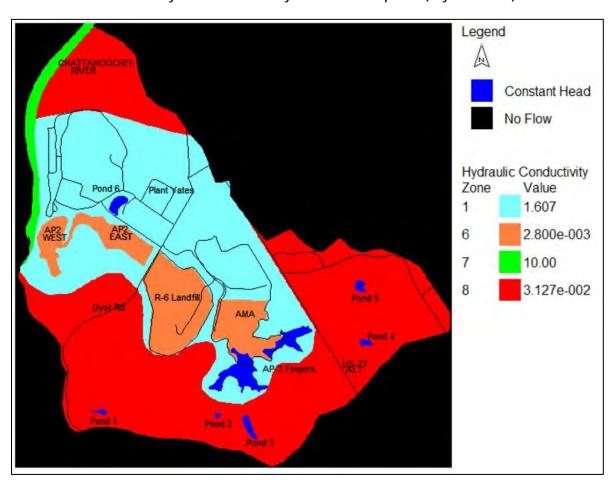


Figure 4-4 Recharge Zones – Layer 1

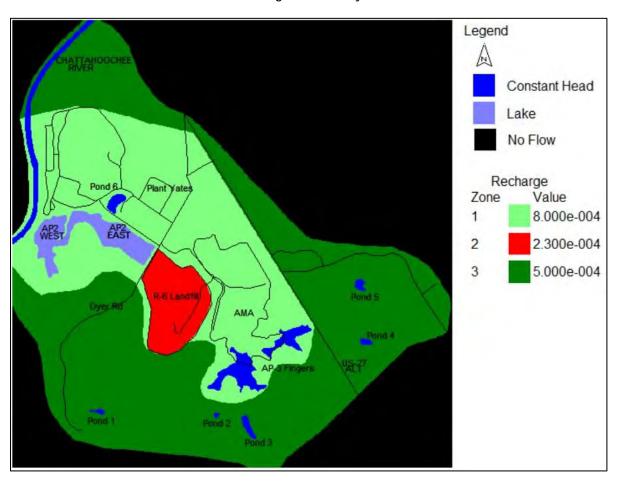


Figure 4-5 Evapotranspiration Zones – Layer 1

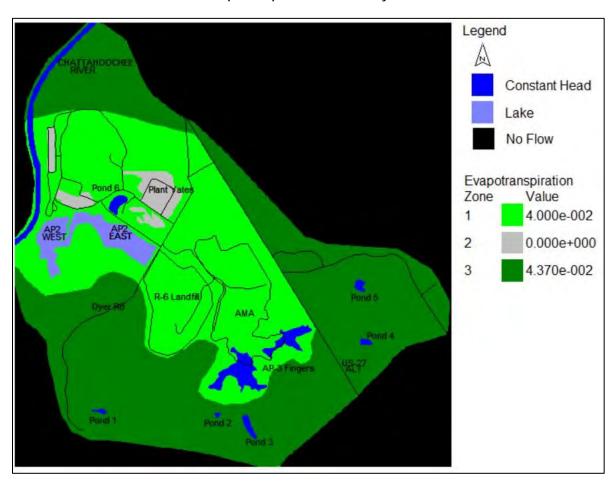
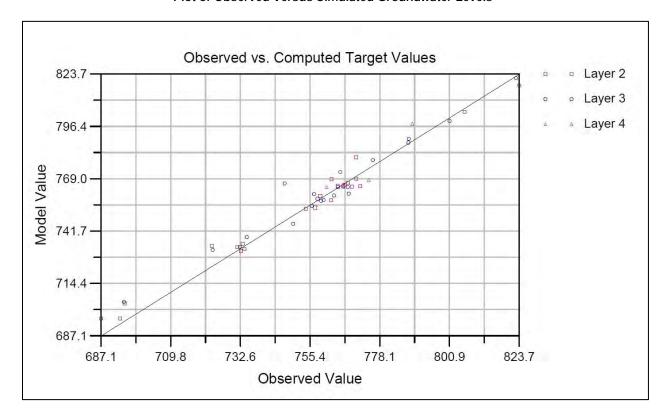


Figure 4-6
Plot of Observed Versus Simulated Groundwater Levels



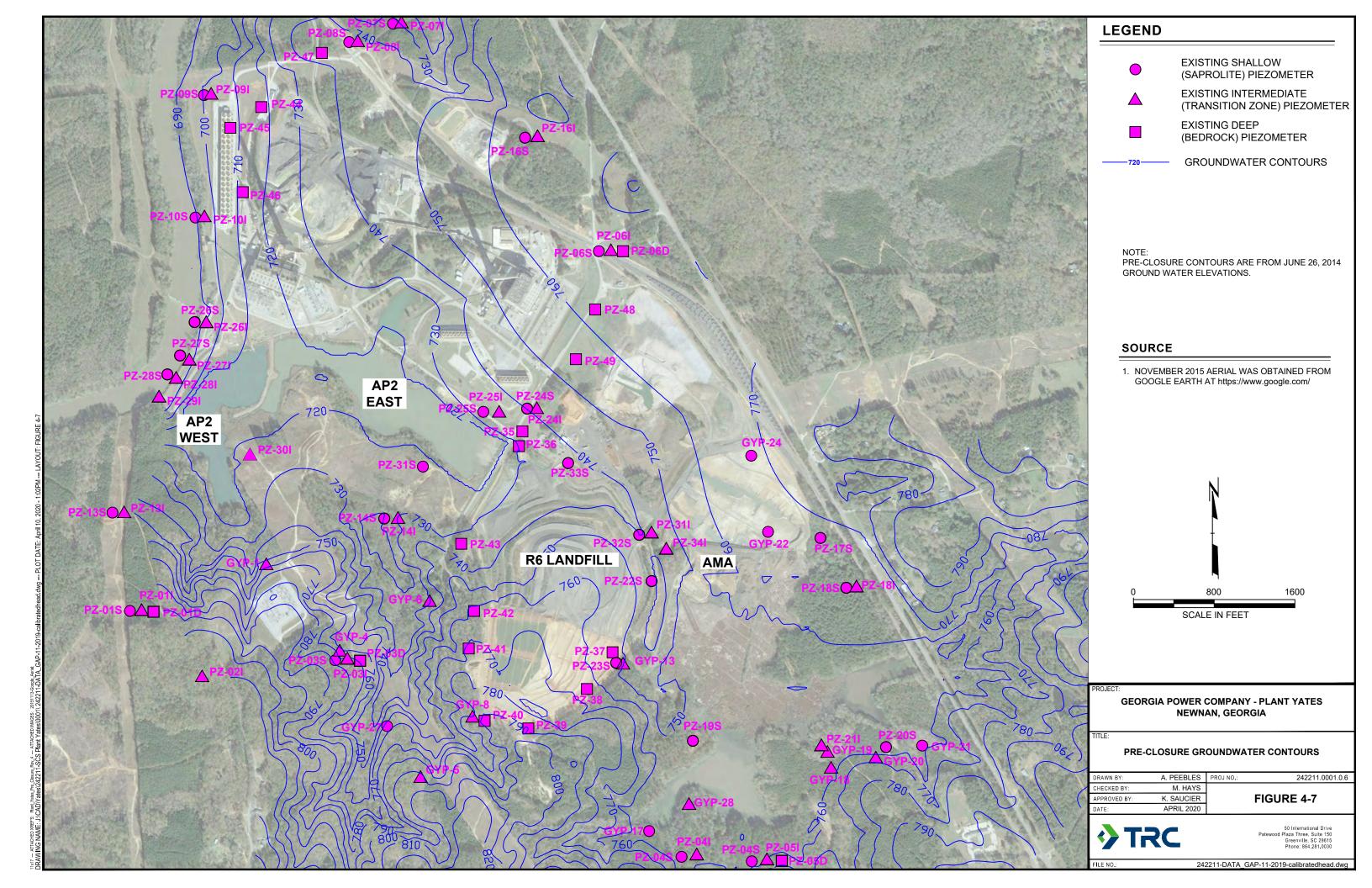
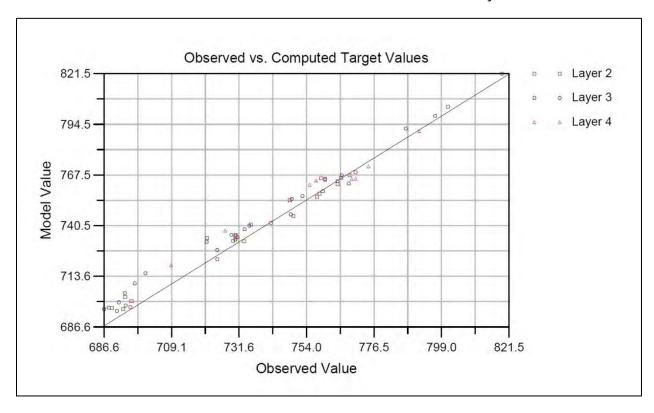


Figure 4-8
Plot of Observed Versus Simulated Groundwater Levels – February 20, 2017



Section 5 Post-Closure Groundwater Flow Modeling

Following groundwater modeling calibration, the calibrated parameter values such as hydraulic conductivity, recharge, and ET were used to create a post-closure groundwater flow model. The post-closure model represented the potential future Site closure plan and was used as a predictive tool for evaluating the impact of the closure plan to Site groundwater flow conditions.

The post-closure model incorporated the potential future layout of the R6 landfill and the AMA. This included the following changes:

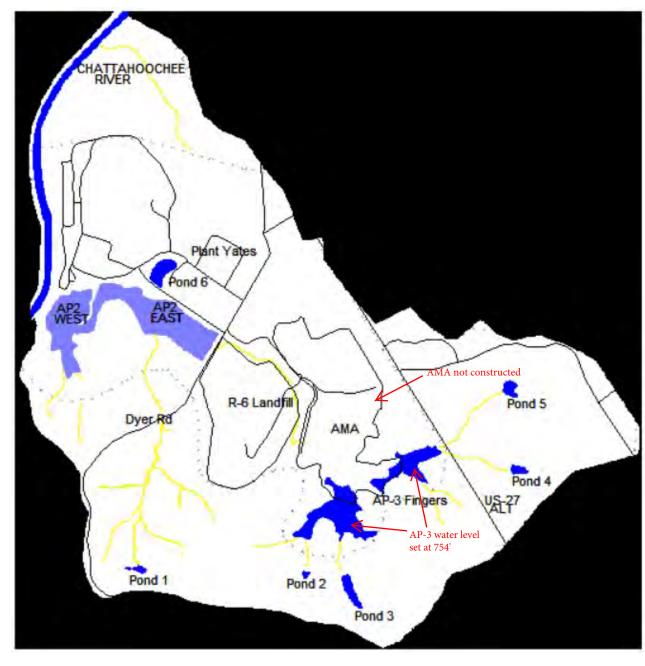
- Consolidation of on-site ash within the AMA footprint. This change to the post-closure model was created by removing the ash layer from AP-2 and reflecting its placement within a revised AMA footprint.
- The ash consolidated within the AMA is covered with a synthetic liner. To account for the synthetic liner over the AMA, a recharge rate of zero inches per year was assigned to within the AMA footprint.
- The layout of the AMA and corresponding berm contour details was provided by SCS in an AutoCAD file titled AMA with AP-B, dated August 14, 2017. The features depicted in the AutoCAD file in addition to an updated topographic survey from April 2017 (UAV drone survey) were incorporated into the post-closure model to update the revised topographic surface.
- The AP-3 area was modified to reflect the separation into the East Cove and South Cove. Based on the anticipated operating levels provided by SCS, a constant head boundary condition of 748.50 feet AMSL was applied to the East Cove and a constant head elevation of 744.50 feet AMSL was applied to the South Cove.
- The addition of perimeter surface drains designed to convey surface water from the synthetic liner covering the AMA was included in the post-closure model. This was accomplished by setting the drain elevation within the post-closure model equal to the ground surface elevation.
- A storm water retention basin was added to the post-closure model north of the R6 landfill. Based on the anticipated operating level provided by SCS, a constant head boundary condition of 730 feet AMSL was applied to the basin.

Figure 5-1 depicts a comparison of the pre-closure model configuration to the post-closure model configuration.

The post-closure groundwater flow model was used to provide an estimate of changes to Site water levels following completion of closure activities. The post-closure model was run to achieve steady-state using the hydraulic properties obtained from the calibrated model. The predicted groundwater contours from

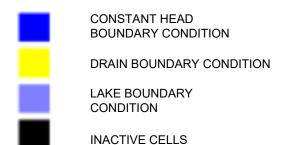
the model are depicted on **Figure 5-2**. Additionally, a side by side comparison of the pre-closure groundwater contour output to the post-closure groundwater contour output is depicted on **Figure 5-3**. The overall general groundwater flow direction in the predicted model was to the northwest, which is similar to pre-closure conditions. However, groundwater flow in the R6 landfill area appears to have shifted more to the northeast around the landfill as compared to through it, as in previous conditions. The hydraulic gradient in the predictive model ranged from approximately 0.01 ft/ft in the northern portion of the AMA to approximately 0.09 ft/ft to the south of AP-2 West.

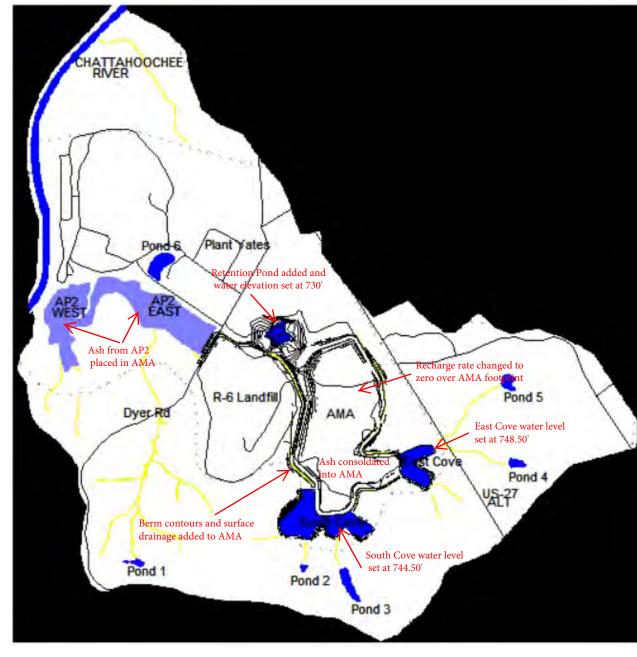
The change in post-closure groundwater elevation is depicted on **Figure 5-4**. With respect to the change in groundwater elevations, the predicted change in groundwater elevations in the post-closure model appears to decrease from approximately less than 1 foot to over 4 feet within the R6 landfill and approximately less than 2 feet to over 8 feet within the AMA compared to pre-closure conditions.



PRE-CLOSURE MODEL CONFIGURATION

LEGEND





POST-CLOSURE MODEL CONFIGURATION



NOT TO SCALE

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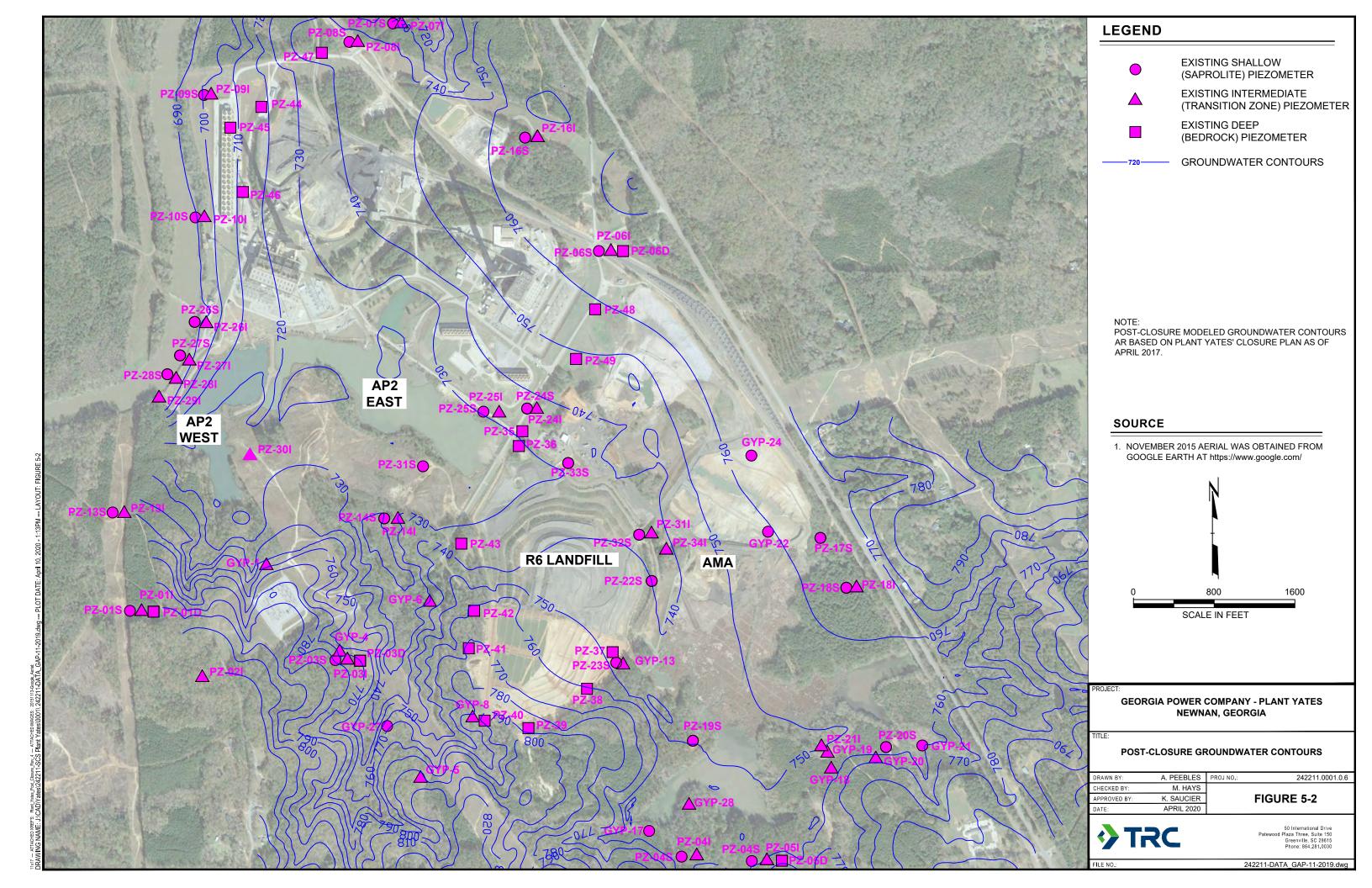
COMPARISON OF PRE-CLOSURE VERSUS POST-CLOSURE **GROUNDWATER MODEL CONFIGURATIONS**

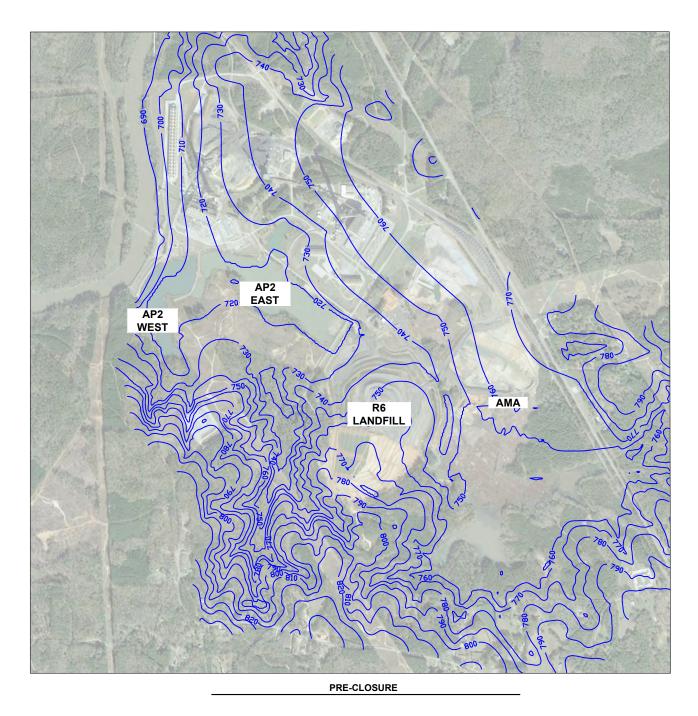
R. MAYER PROJ NO.: DRAWN BY: CHECKED BY: M. HAYS APPROVED BY: K. SAUCIER APRIL 2020

242211.0001.0.6 FIGURE 5-1



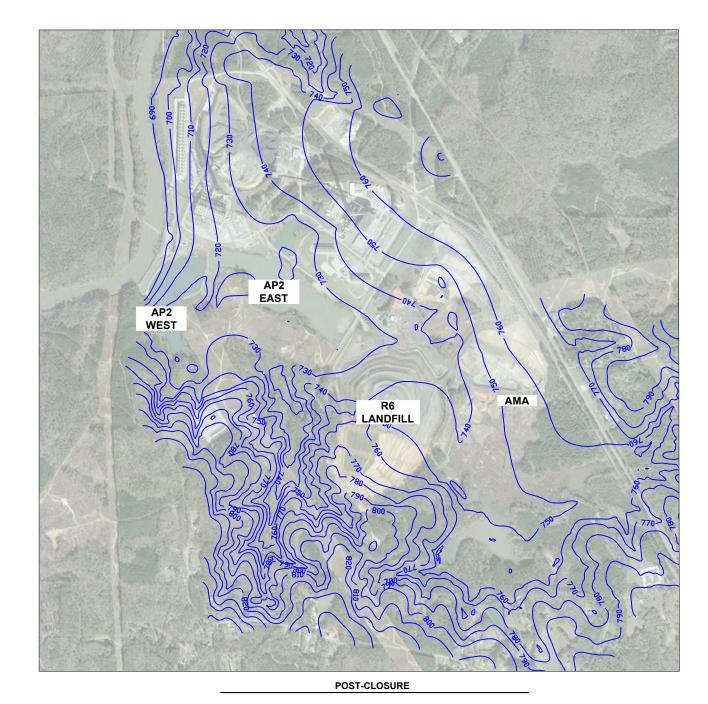
242211-DATA_GAP-11-2019-FIGURE_5-1.dwg





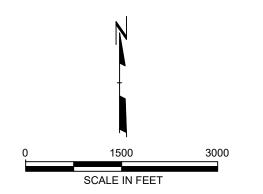
LEGEND

MODEL GROUNDWATER CONTOURS



SOURCE

 NOVEMBER 2015 AERIAL WAS OBTAINED FROM GOOGLE EARTH AT https://www.google.com/



GEORGIA POWER COMPANY - PLANT YATES
NEWNAN, GEORGIA

TITLE:

COMPARISON OF PRE-CLOSURE VERSUS POST-CLOSURE GROUNDWATER CONTOURS

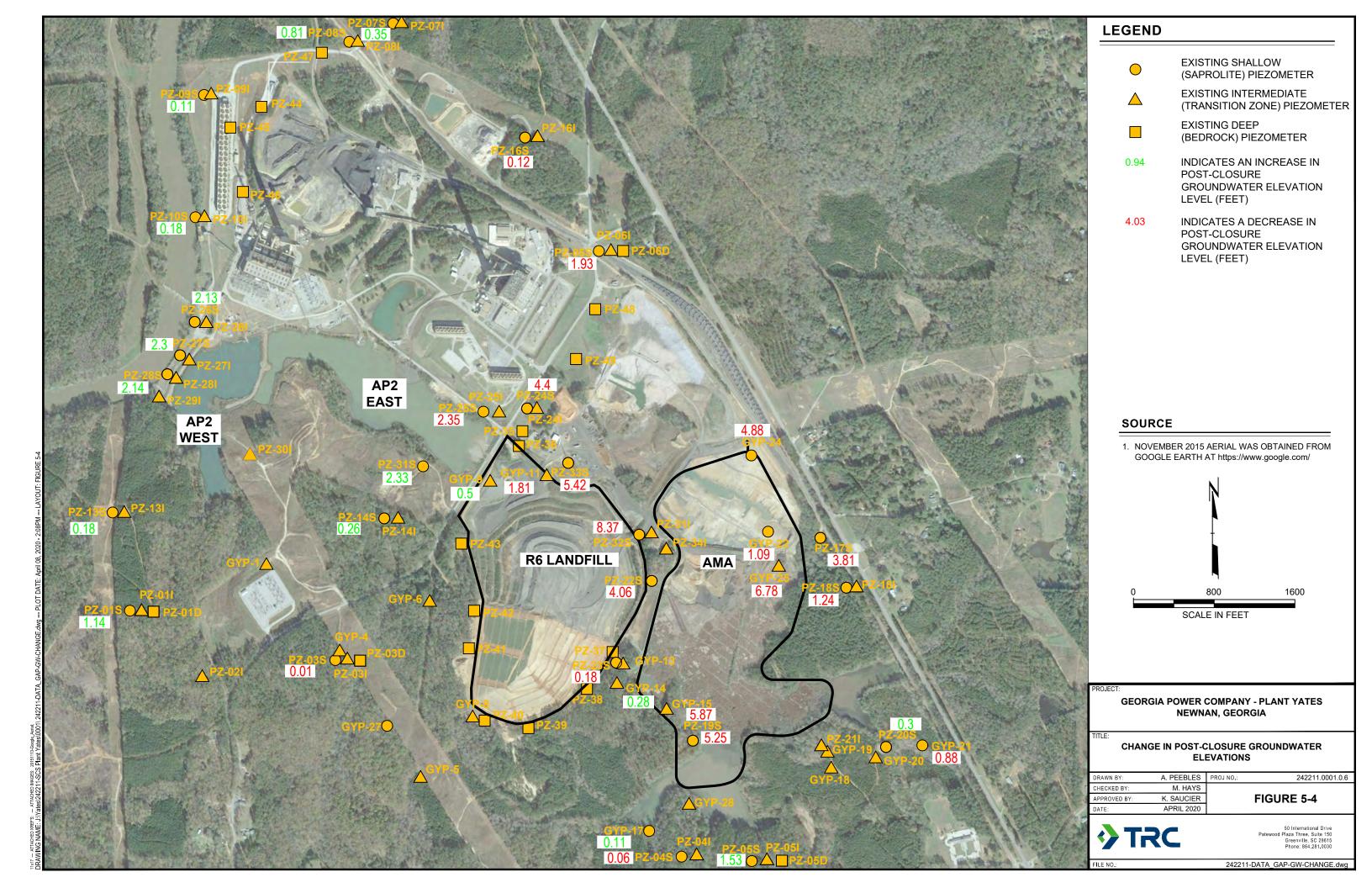
DRAWN BY:	A. PEEBLES	PRO
CHECKED BY:	M. HAYS	
APPROVED BY:	K. SAUCIER	
DATE:	APRIL 2020	

FIGURE 5-3



50 International Drive wood Plaza Three, Suite 150 Greenville, SC 29615

242211-DATA_GAP-11-2019-FIGURE_5-3.dwg



Section 6 Summary

This report documents the development and application of groundwater flow modeling for the Georgia Power Plant Yates Facility located near Newnan, Georgia. Groundwater modeling was performed at the Site to help predict changes in groundwater levels following the completion of closure activities. Groundwater Vistas was used to create a calibrated model that accurately depicted pre-closure groundwater levels for the Site. Following groundwater modeling calibration, the proposed closure plan was incorporated to build a post-closure groundwater flow model. The post-closure model incorporated the potential future layout of the R6 landfill and the AMA. The post-closure groundwater flow model was used to provide an estimate of changes to Site water levels and groundwater flow dynamics following completion of closure activities.

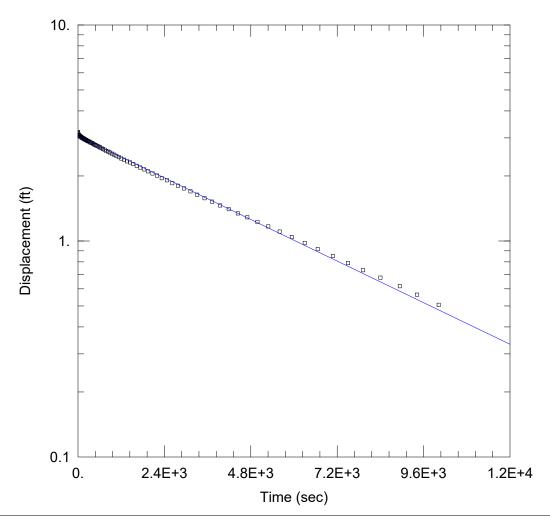
Based on the results of the modeling efforts, the following conditions are predicted to occur:

- The overall general groundwater flow direction in the predicted model is anticipated to remain similar to pre-closure conditions which is flow to the northwest;
- Groundwater flow in the R6 landfill area appears to have shifted more to the northeast around the landfill as compared to through it, as in previous conditions;
- The hydraulic gradient is predicted to range from a minimum of approximately 0.01 ft/ft in the northern portion of the AMA to a maximum of approximately 0.09 ft/ft to the south of AP-2 West; and
- The predicted change in groundwater elevations appears to decrease from approximately less than 1 foot to over 4 feet within the R6 landfill and approximately less than 2 feet to over 8 feet within the AMA.

Section 7 References

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Appendix A AQTESOLV Slug Test Data Outputs



P-1 FALLING HEAD TEST

Data Set: P:\...\P-1 Falling Head.aqt

Date: 02/15/18 Time: 08:49:11

PROJECT INFORMATION

Company: TRC Client: SCS Project: 242211

Location: Plant Yates, GA

Test Well: <u>P-1</u> Test Date: <u>2/1/2018</u>

AQUIFER DATA

Saturated Thickness: 12. ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (P-1)

Initial Displacement: 3.19 ft

Total Well Penetration Depth: 22. ft

Casing Radius: 0.08333 ft

Static Water Column Height: 8. ft

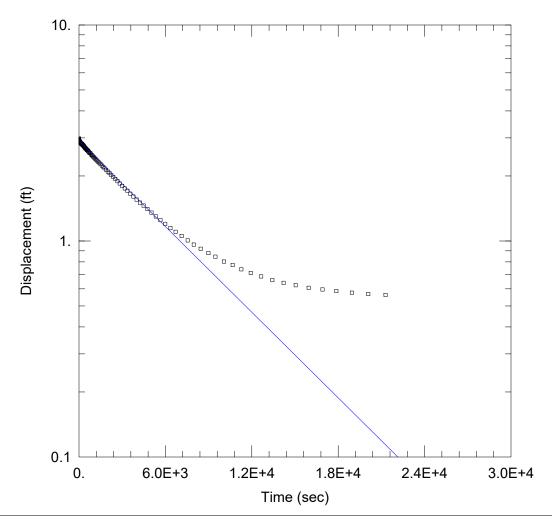
Screen Length: 10. ft Well Radius: 0.25 ft

SOLUTION

Aquifer Model: <u>Unconfined</u>

Solution Method: Bouwer-Rice

K = 0.01828 ft/day y0 = 3.051 ft



P-1 RISING HEAD TEST

Data Set: P:\...\P-1 Rising Head.aqt

Date: 02/15/18 Time: 08:49:42

PROJECT INFORMATION

Company: TRC
Client: SCS
Project: 242211

Location: Plant Yates, GA

Test Well: <u>P-1</u> Test Date: <u>2/1/2018</u>

AQUIFER DATA

Saturated Thickness: 12. ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (P-1)

Initial Displacement: 2.98 ft

Total Well Penetration Depth: 22. ft

Casing Radius: 0.08333 ft

Static Water Column Height: 8. ft

Screen Length: 10. ft Well Radius: 0.25 ft

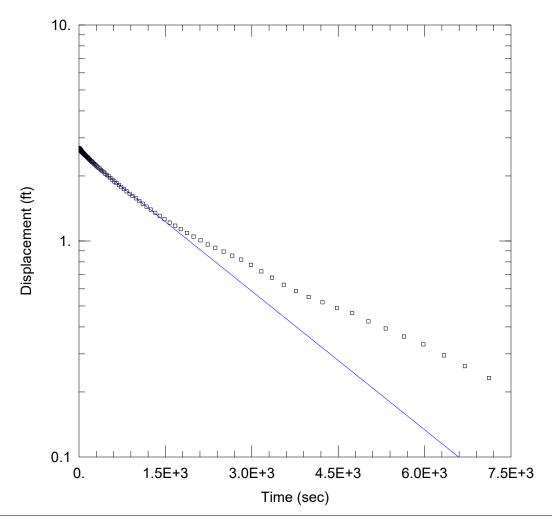
SOLUTION

Aquifer Model: Unconfined

K = 0.01506 ft/day

Solution Method: Bouwer-Rice

y0 = 2.903 ft



P-2 FALLING HEAD TEST

Data Set: P:\...\P-2 Falling Head.aqt

Date: 02/15/18 Time: 08:50:25

PROJECT INFORMATION

Company: TRC Client: SCS Project: 242211

Location: Plant Yates, GA

Test Well: <u>P-2</u> Test Date: <u>2/2/2018</u>

<u>AQUIFER</u> <u>DAT</u>A

Saturated Thickness: 18. ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (P-2)

Initial Displacement: 2.7 ft

Total Well Penetration Depth: 25. ft

Casing Radius: 0.08333 ft

Static Water Column Height: 8. ft

Screen Length: 10. ft Well Radius: 0.25 ft

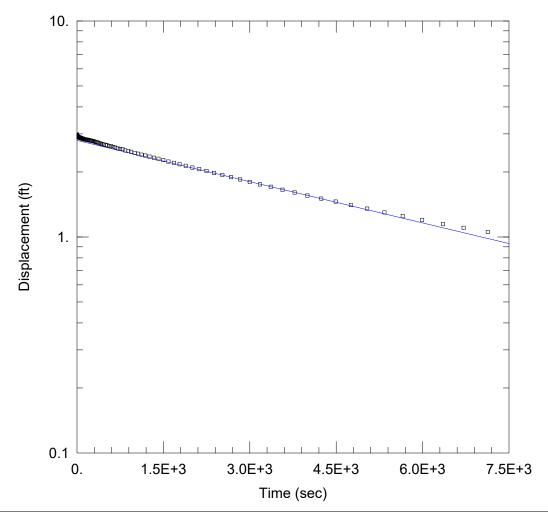
SOLUTION

Aquifer Model: Unconfined

K = 0.04986 ft/day

Solution Method: Bouwer-Rice

y0 = 2.566 ft



P-2 RISING HEAD TEST

Data Set: P:\...\P-2 Rising Head.aqt

Date: 02/15/18 Time: 08:43:54

PROJECT INFORMATION

Company: TRC Client: SCS Project: 242211

Location: Plant Yates, GA

Test Well: P-2 Test Date: 2/2/2018

AQUIFER DATA

Saturated Thickness: 18. ft Anisotropy Ratio (Kz/Kr): 1.

WELL DATA (P-2)

Initial Displacement: 2.98 ft

Total Well Penetration Depth: 25. ft

Casing Radius: 0.08333 ft

Static Water Column Height: 8. ft

Solution Method: Bouwer-Rice

Screen Length: 10. ft Well Radius: 0.25 ft

SOLUTION

Aquifer Model: Unconfined

K = 0.01484 ft/day

y0 = 2.799 ft